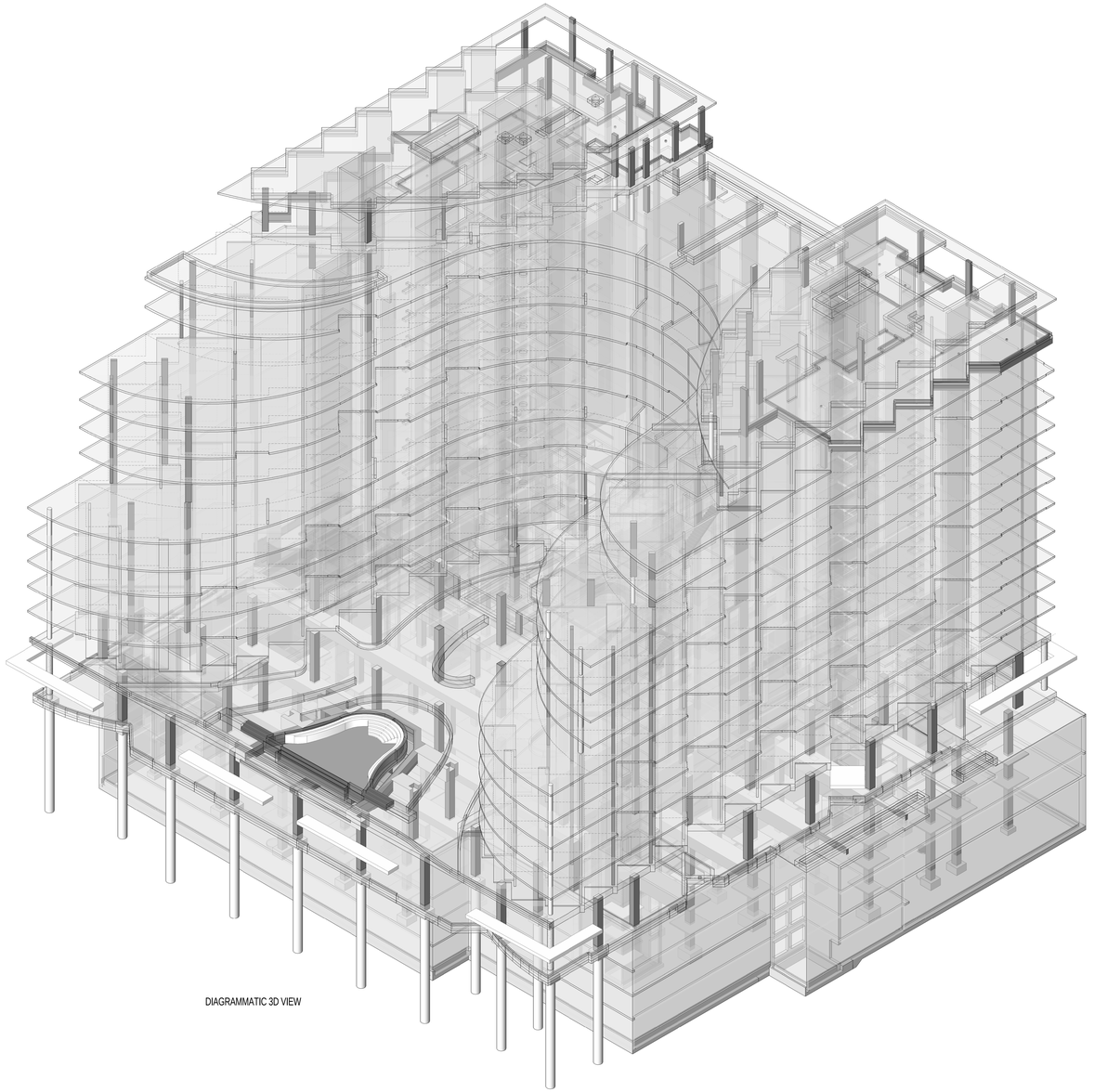

DIAGRAMMATIC 3D VIEW

DRAWING LIST

Sheet Number	Sheet Name
S0 SERIES - COVER SHEET	
S0-01	COVER SHEET
S1 SERIES - PLANS	
S1-01	FOUNDATION PLAN
S1-02	LEVEL P3 FLOOR FRAMING PLAN
S1-03	LEVEL P2 FLOOR FRAMING PLAN
S1-04	LEVEL P1 FLOOR FRAMING PLAN
S1-05	LEVEL P1 MEZZANINE FRAMING PLAN
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S1-07	LEVEL 1-GROUND FLOOR FRAMING PLAN WITH BOTTOM REINFORCEMENT AND SCHEDULES
S1-08	LEVEL L1 MEZZANINE FRAMING PLAN
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S1-10	LEVEL 3 FLOOR FRAMING PLAN
S1-11	LEVEL 4 FLOOR FRAMING PLAN
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S1-14	LEVEL 7 FLOOR FRAMING PLAN
S1-15	LEVEL 8 FLOOR FRAMING PLAN
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S1-17	LEVEL 10 FLOOR FRAMING PLAN
S1-18	LEVEL 11 FLOOR FRAMING PLAN
S1-19	LEVEL 12 / PH1 FLOOR FRAMING PLAN
S1-20	LEVEL 13/PH2/ MECH. FLOOR FRAMING PLAN
S1-21	ROOF FRAMING PLANS
S2 SERIES - SCHEDULES	
S2-01	COLUMN SCHEDULES - LEVEL 2 AND BELOW
S2-02	COLUMN SCHEDULE - LEVEL 2 ABOVE AND CONNECTION PLATE SCHEDULE
S3 SERIES - WALL ELEVATIONS	
S3-01	WALL ELEVATIONS AND NOTES
S3-02	WALL ELEVATIONS
S3-03	WALL ELEVATIONS
S3-04	WALL ELEVATIONS
S3-05	WALL ELEVATIONS
S3-06	WALL ELEVATIONS
S3-07	WALL ELEVATIONS
S3-08	WALL ELEVATIONS
S3-09	WALL ELEVATIONS
S3-10	WALL ELEVATIONS
S4 SERIES - FOUNDATION ELEVATION AND SECTIONS	
S4-01	FOUNDATION ELEVATIONS
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S4-03	FOUNDATION SECTIONS
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S5 SERIES - PODIUM FLOOR SECTIONS	
S5-01	PODIUM FLOOR SECTIONS
S8 SERIES - TYPICAL NOTES & DETAILS	
S8-01	GENERAL NOTES & TYPICAL DETAILS
S8-02	TYPICAL DETAILS

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
COVER SHEET

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale

Drawn by
DC

Checked by
BBS

Project No.
2014/0031

Date
JUNE 12,2015

Drawing No.
S0-01

DESIGN CRITERIA NOTES

1. GENERAL
   - 1.1. THE PROJECT HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2012 OBC (O. REG. 332/12 AS AMENDED) INCLUDING CLAUSES 4.1.6.1(1), 4.1.6.4(3), 4.1.7 AND 4.1.8
   - 1.2. THE PARKING GARAGE HAS BEEN DESIGNED IN ACCORDANCE TO THE STRUCTURAL REQUIREMENTS OF CSA-S413-07 PARKING STRUCTURES. SEE ARCHITECTURAL DRAWINGS FOR WATERPROOFING, SEALERS, ETC... DETAILED IN ACCORDANCE WITH CSA-S413-07. SEE ALSO MECHANICAL DRAWINGS FOR COMPLIANCE WITH THE MECHANICAL REQUIREMENTS OF CSA-S413-07.
   - 1.3. THE AREAS OF THE STRUCTURE ACCESSIBLE TO VEHICULAR TRAFFIC HAVE BEEN DESIGNED FOR CLAUSES 4.1.5.5, 4.1.5.10, APPENDIX A OF THE OBC AND WHERE APPLICABLE GARAGE ROOF SLABS HAVE BEEN DESIGNED FOR THE REQUIREMENTS OF THE CITY OF TORONTO GUIDELINES FOR GARBAGE TRUCK AND FIRE TRUCK WHEEL LOADING.
   - 1.4. ALL REINFORCED CONCRETE SLABS, BEAMS, COLUMNS, AND SHEAR WALLS HAVE BEEN DESIGNED TO ACHIEVE A MINIMUM 2 HOUR FIRE RESISTANCE RATING.
   - 1.5. MISCELLANEOUS METAL, PRECAST AND STAIR FABRICATORS SHALL:
     - 1.5.1. PROVIDE SHOP DRAWINGS TO THE ARCHITECT PRIOR TO FABRICATION; STAMPED, SIGNED AND DATED BY A PROFESSIONAL ENGINEER.
     - 1.5.2. DESIGN ALL GUARDS TO MEET LATERAL LOADS DESCRIBED IN OBC 4.1.5.15.
     - 1.5.3. DESIGN ALL HANDRAILS TO MEET LOADS DESCRIBED IN OBC 3.4.6.4(9).
     - 1.5.4. DESIGN ALL STAIRS TO SUPPORT A MINIMUM LIVE LOAD OF 4.8kPa.
   - 1.6. ARCHITECTURAL PRECAST FABRICATOR SHALL:
     - 1.6.1. PROVIDE SHOP DRAWINGS TO THE ARCHITECT PRIOR TO FABRICATION, STAMPED, SIGNED AND DATED BY A PROFESSIONAL ENGINEER.
     - 1.6.2. WHERE PRECAST IS USED AS A GUARD DESIGN THE PRECAST AND CONNECTIONS TO MEET LATERAL LOADS DESCRIBED IN OBC 4.1.5.15.
   - 1.7. IT IS THE RESPONSIBILITY OF THE CONTRACTOR WHO IS SUPPLYING AND INSTALLING EQUIPMENT, THAT ALL ELEMENTS OF STRUCTURES LISTED IN TABLE 4.1.8.17 OF THE OBC 2012 ARE DESIGNED IN ACCORDANCE WITH CLAUSE 4.1.8.17.
   - 1.8. BUILDING IMPORTANCE CATEGORY (SNOW, WIND, AND EARTHQUAKE) IS NORMAL (Ie=1.0)
   - 1.9. STIFF ELEMENTS NOT PART OF SFRS SHALL BE SEPARATED FROM THE STRUCTURE AS PER OBC CLAUSE 4.1.8.3 (6a). EXAMPLES INCLUDE, BUT NOT LIMITED TO MASONRY PARTITIONS, BRICK VENEER, PRECAST CLADDING ETC. IT IS THE RESPONSIBILITY OF THE SUBCONTRACTOR TO PROVIDE SHOP DRAWINGS, STAMPED, SIGNED AND DATED BY A PROFESSIONAL ENGINEER DEMONSTRATING COMPLIANCE. PROVIDE MINIMUM 15mm SEPARATION UNLESS NOTED OTHERWISE ON DRAWINGS AND SPECIFICATION.

2. LATERAL LOADS ON STRUCTURE
   - 2.1. WIND
     - q(1/50) = 0.44 kPa
     - Ce = (h/10)^0.28 NOT LESS THAN 1.0 AND NOT GREATER THAN 2.5(DYNAMIC APPROACH).
     - Cg = 1.8
     - Cp = AS PER FIGURE I-15 OF USER'S GUIDE - NBC 2005 STRUCTURAL COMMENTARIES (PART 4 OF DIVISION B).
   - 2.2. EARTHQUAKE
     - Sa(0.2) = 0.22 PGA = 0.12 Fa = 1.0
     - Sa(0.5) = 0.13 SITE CLASS =C Fv = 1.0
     - Sa(1.0) = 0.067 Rd = 1.5 Ie = 1.0
     - Sa(2.0) = 0.021 Ro = 1.3
     - SFRS CONSISTS OF CONCRETE SHEAR WALLS.
     - STRUCTURAL IRREGULARITIES CONSIDERED IN THE DESIGN PER CLAUSE 4.1.8.6. CONSIST OF: TYPES 1, 2, 3, 4, 5,8
     - METHOD OF ANALYSIS :- DYNAMIC

3. FOUNDATION WALLS
   - 3.1. WALLS RETAINING EARTH ARE DESIGNED TO SAFELY WITHSTAND HORIZONTAL EARTH AND HYDROSTATIC PRESSURE
     (P=K (Yh1+Y'h2+q)+Ywh2),

     WHERE
     K= 0.40
     Wt = 22kN/m³
     q = 12kPa (LIVE LOAD SURCHARGE)
     h1 = DEPTH TO GROUND WATER TABE, IN METRES
     h2 = DEPTH BELOW GROUND WATER TABLE, IN METRES
     Y = 22 kN/m³ (SOIL UNIT WEIGHT)
     Y' = 12 kN/m³ (SUBMERGED SOIL UNIT WEIGHT)
     Yw = 10kN/m³ (WATER UNIT HEIGHT)

	STRENGTH AT 28 DAYS (Mpa)	SLUMP AT DELIVERY (mm)	AIR ENTRAINMENT	MAXIMUM W/C RATIO	EXPOSURE CLASSIFICATION
GENERAL (NON-EXPOSED) CONCRETE INTERIOR SLABS, WALLS, COLUMNS, BEAMS AND FOUNDATIONS (SEE PLAN & SCHEDULE FOR STRENGTH)	25 30 35 40 45	80 ± 20	- - - -	TO SUIT	N
EXTERIOR (EXPOSED) CONCRETE WALLS AND OR COLUMNS, EXCEPT PARKING STRUCTURE	25 30 35 40 45	80 ± 20	4 - 7 %	0.55	F - 2
(1) GENERAL INTERIOR SLAB - ON - GRADE	25	80 ± 20	- - - -	TO SUIT	N
EXTERIOR SLABS-ON-GRADE. SIDEWALKS, CURBS AND GUTTERS (UNREINFORCED)	32	80 ± 20	5 - 8 %	0.45	C - 2
(2) PARKING STRUCTURE. (SLAB, COLUMNS, BEAMS WALLS GRADE BEAMS/CAPS AND FOOTINGS)	35 40 45	80 ± 20	5 - 8 %	0.40	C - 1
(2) GARAGE ROOF (PROVIDE DCI AT A DOSAGE OF 10L/m³)	35	80 ± 20	5 - 8 %	0.40	C - 1
(1) PARKING SLABS-ON-GRADE	32	80 ± 20	5 - 8 %	0.45	C - 2
NON WATER PROOFED EXPOSED BALCONY SLAB, POOL	30	80 ± 20	5 - 8 %	0.50	F - 1
LEAN MIX CONCRETE	5	80 ± 20	- - - -	TO SUIT	N

1) REINFORCED WITH SYNTHETIC FIBRES ADDED AT BATCHING PLANT - SEE SPECIFICATION.

2) CHLORIDE ION PENETRABILITY REQUIREMENTS < 1500 COULOMBS WITHIN 56 DAYS.

FOUNDATION PLAN NOTES:

1. TOP OF SLAB VARIES IN PARKING LEVEL AND IS TO BE SLOPED TO SUIT DRAINAGE AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
2. SLAB-ON-GRADE TO BE 100 mm THICK WITH SAW CUTS @3000 O/C EACH WAY, SEE SPECIFICATIONS.
3. PROVIDE SLAB DEPRESSIONS, OTHER THAN THOSE SHOWN ON STRUCTURAL DRAWINGS, AS REQUIRED BY ARCHITECTURAL AND MECHANICAL DRAWINGS AND SPECIFICATIONS.
4. SLAB-ON-GRADE TO BE PLACED ON A 200 mm MINIMUM LAYER OF COMPACTED 19mm MAXIMUM SIZE CLEAR CRUSHED STONE.
5. FOOTINGS SHALL BE CARRIED DOWN TO SOUND BEDROCK WITH A FACTORED GEOTECHNICAL RESISTANCE (ØULS) OF 5000 kPa.
   ALL SPREAD AND STRIP FOOTINGS TO BE CAST AGAINST SIDES OF EXCAVATION.
6. FRICTION CAISSONS HAVE BEEN DESIGNED FOR ULS BEND STRENGTH OF 700kPa BETWEEN CONCRETE AND SOUND SHALE.
7. BEDROCK OF THIS VALUE IS ASSUMED TO BE FOUND AT THE ELEVATIONS SHOWN, ANY VARIATIONS MUST BE REPORTED TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK.
8. REFER TO SOIL REPORT BRM-00017976-J0 DATED APRIL 24, 2015 PREPARED BY EXP. AND MEMORANDUM DATED APRIL 24,2015.
9. BEDROCK AT THE UNDERSIDE OF ALL FOOTINGS IS TO BE INSPECTED AND APPROVED BY A REPRESENTATIVE OF THE SOILS CONSULTANT BEFORE PLACING CONCRETE. A 50mm LEAN MIX CONCRETE SKIM SLAB IS TO BE PLACED ON BEDROCK IMMEDIATELY AFTER EXCAVATION AND INSPECTION IS COMPLETED.
10. TOP OF FOOTINGS TO BE AT ELEVATIONS AS NOTED ON PLAN (TOF =).
11. SDF = STEP DOWN FOOTING. STEPS IN ANY FOOTING SHALL BE 600mm MAX. AND 10 HORIZ. TO 7 VERT.
12. UNLESS OTHERWISE NOTED, ALL WALL FOOTINGS TO BE 350 mm DEEP WITH 150 mm PROJECTION EACH SIDE. ALL PERIMETER FOUNDATION WALL FOOTINGS TO BE WITH 150 mm PROJECTION.
    PROTECT FOOTING BASE BY 50mm CONCRETE IF CONCRETE PLACEMENT NOT OCCUR ON THE SAME DAY.
13. DO NOT PLACE BACKFILL AGAINST WALLS RETAINING EARTH UNTIL FLOORS AT TOP AND BOTTOM OF WALLS ARE PLACED AND HAVE ATTAINED SPECIFIED DESIGN STRENGTH U/N.
14. FILL REQUIRED ON BOTH SIDES OF FOUNDATION WALLS SHALL BE PLACED AND COMPACTED SIMULTANEOUSLY ON BOTH SIDES TO EQUALIZE SOIL PRESSURE.
15. CO-ORDINATE ALL DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCIES TO THE ENGINEER AND ARCHITECT PRIOR TO PROCEEDING WITH ANY WORK.
16. CENTRE LINES OF COLUMNS, WALLS, CAPS, CAISSONS AND FOOTINGS ARE COINCIDENT UNLESS OTHERWISE SHOWN. (SEE ARCHITECTURAL)
17. CONCRETE FOR FOUNDATION WALLS AND ALL FOOTINGS SHALL BE CLASS C-1 WITH A MINIMUM CONCRETE STRENGTH AT 28 DAYS OF 35 MPa U/N.
18. ALL CONCRETE WALLS POURED MONOLITHICALLY WITH COLUMNS AND/OR SHEAR WALLS SHALL HAVE THE SAME CONCRETE STRENGTH AS INDICATED FOR THE COLUMNS AND/OR SHEAR WALLS WHICHEVER IS HIGHER.
19. THE FORMING CONTRACTOR IS TO SUBMIT FOR REVIEW PROPOSED SLAB-ON-GRADE SAWCUT LAYOUT SEE TYPICAL DETAILS.
20. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO POURING STRUCTURAL CONCRETE.
21. SEE COLUMN SCHEDULES AND SHEAR WALL ELEVATIONS.
22. SEE ALSO GENERAL NOTES AND TYPICAL DETAILS.
23. SEE TYPICAL CONCRETE MIX CHART DRAWING S1-01.
24. SEE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR UNDER SLAB-ON-GRADE WEEPING TILE SYSTEM.
25. PROVIDE DCI AT A DOSAGE OF 10L/m³ CONCRETE IN THE FRAMED RAMP SLAB AND ALL TURNING AREAS.
26. C - ON PLAN DENOTES CAISSON. SEE SCHEDULE ON THIS DRAWING.
27. CP - ON PLAN DENOTES CAISSON CAP. SEE SCHEDULE ON THIS DRAWING.
28. REFER TO SOIL REPORT RECOMMENDATIONS REGARDING RELIEF WELLS, BOND STRENGTH FOR CAISSONS.

SITE PREPARATION NOTES FOR SLAB-ON-GRADE (WITHIN BUILDING ENVELOPE)

1. THE AREA WITHIN THE BUILDING SHALL BE STRIPPED OF THE UPPER LAYER SOIL, FILL, ORGANICALLY CONTAMINATED MATERIAL AND RUBBLE AND TO A MINIMUM OF 200mm (8") BELOW THE UNDERSIDE OF THE SLAB ON GRADE.
2. THE EXPOSED SUB-GRADE SHALL BE EXAMINED AND APPROVED BY THE SOIL CONSULTANT.
3. THE ENTIRE AREA SHALL BE PROOF ROLLED WITH A HEAVY COMPACTOR TO A MINIMUM OF 95% STANDARD PROCTOR MAX. DRY DENSITY AND TO THE APPROVAL OF THE SOIL CONSULTANT.
4. ANY LOOSE OR SOFT SPOTS ENCOUNTERED SHALL BE SUB-EXCAVATED AND BACKFILLED WITH COMPACTED APPROVED MATERIAL.
5. FILL REQUIRED TO RAISE THE GRADES SHALL BE COMPRISED OF APPROVED GRANULAR 'B' TYPE 1 CONFORMING TO OPSS 1010 PLACED IN SUCCESSIVE LOOSE 200mm(8") LAYERS EACH COMPACTED TO AT LEAST 95% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY.
6. THE LAYER IMMEDIATELY BELOW THE SLAB-ON-GRADE SHALL BE 200mm (8") OF GRANULAR 'A' COMPACTED TO MIN. 98% STANDARD PROCTOR MAX. DRY DENSITY.
7. ALL PROCEDURES, EQUIPMENT AND MATERIALS SHALL BE APPROVED BY THE SOIL CONSULTANT WHO SHALL CONDUCT SUFFICIENT TESTS TO ENSURE THAT THE SPECIFIED MATERIALS AND DENSITIES ARE ACHIEVED.
8. THE CONTRACTOR SHALL CO-ORDINATE WITH THE SOIL CONSULTANT AND ARRANGE A SUITABLE PROGRAM FOR SAMPLING AND INSPECTIONS, ETC. AND NOTIFY THE ARCHITECT ACCORDINGLY.
9. EXISTING ON-SITE MATERIAL SHALL NOT BE USED WITHIN THE BUILDING AREA FOR BACKFILLING IN TRENCHES AGAINST FOUNDATION WALLS OR UNDER SLABS-ON-GRADE.
10. REFER TO THE SPECIFICATION AND THE SOIL REPORT FOR PREPARATION OF AREAS OUTSIDE THE BUILDING ENVELOPE.

LEGEND

(dotted outline)	WALLS AND COLUMN SHADED AS THUS INDICATE WALL AND COLUMN BELOW SLAB
(shaded)	WALLS AND COLUMN SHADED AS THUS INDICATE WALL AND COLUMN ABOVE SLAB
-	INDICATES COLUMN NUMBER OF BELOW SLAB. REFER TO COLUMN SCHEDULE
-	INDICATES COLUMN NUMBER OF ABOVE SLAB. REFER TO COLUMN SCHEDULE

CAISSON SCHEDULE

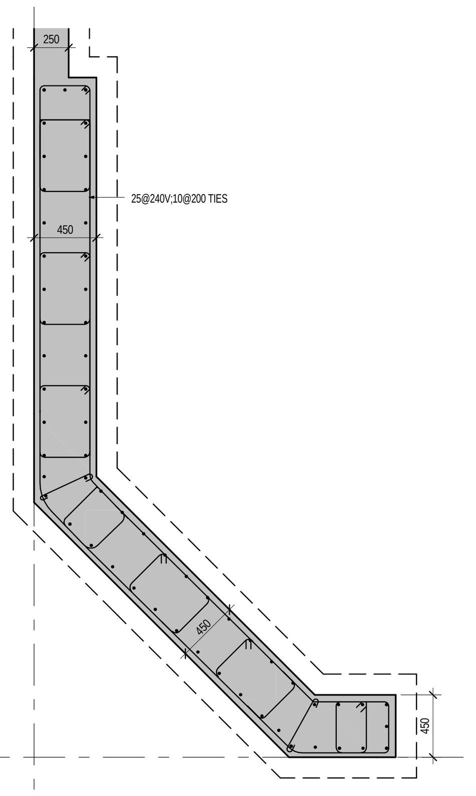
CAISSON NUMBER	CAISSON DIA.	UPPER SHAFT REINFORCEMENT	UPPER SHAFT STIRRUPS ZONE	REMAINING STIRRUPS	EMBEDMENT INTO SOUND BEDROCK, m	CONCRETE STRENGTH	LOAD ULS kN
C1	915Ø	11-20 V + 10@150 TIES	2750	10@300	2.0	25MPa	3700
C2	915Ø	16-25 V + 10@150 TIES	2750	10@200	4.0	35MPa	7400

FOOTING SCHEDULE

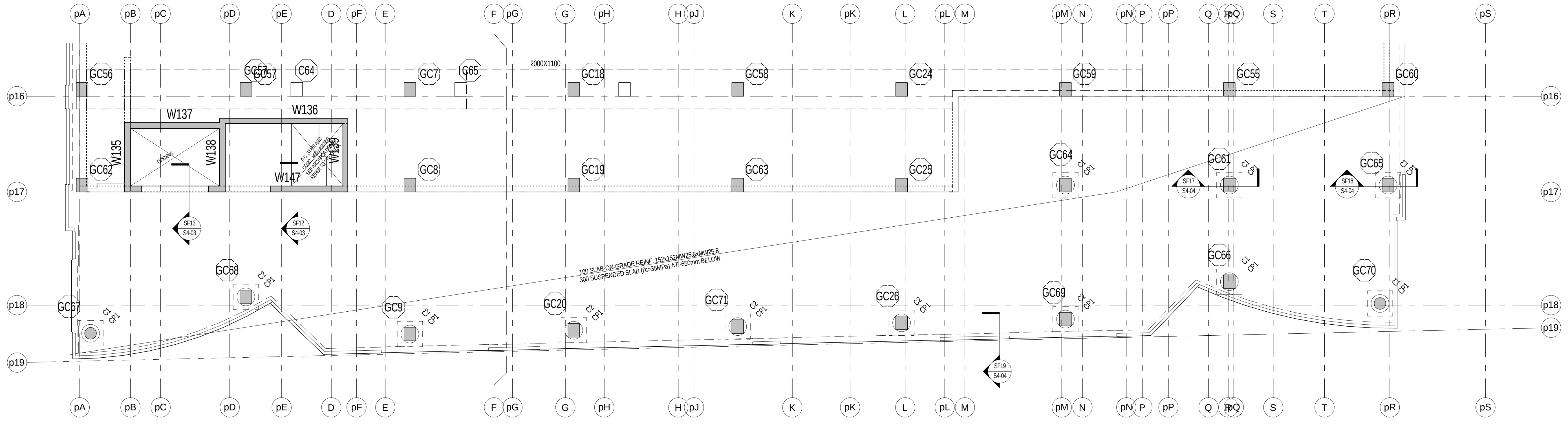
FOOTING NUMBER	FOOTING NUMBER AND SIZE	FOOTING REINF. LG. (HOOKED EE)	FOOTING REINF. TR. (HOOKED EE)	LOAD ULS KN
F1	1000x1100x500 DP.	10-20B	11-20B	4400
F2	1300x1400x800 DP.	13-20B	14-20B	7300
F3	1600x1700x1100 DP.	12-25B	13-25B	1000
F4	1800x2000x1300 DP.	18-25B	20-25B	14400

CAISSON CAP SCHEDULE

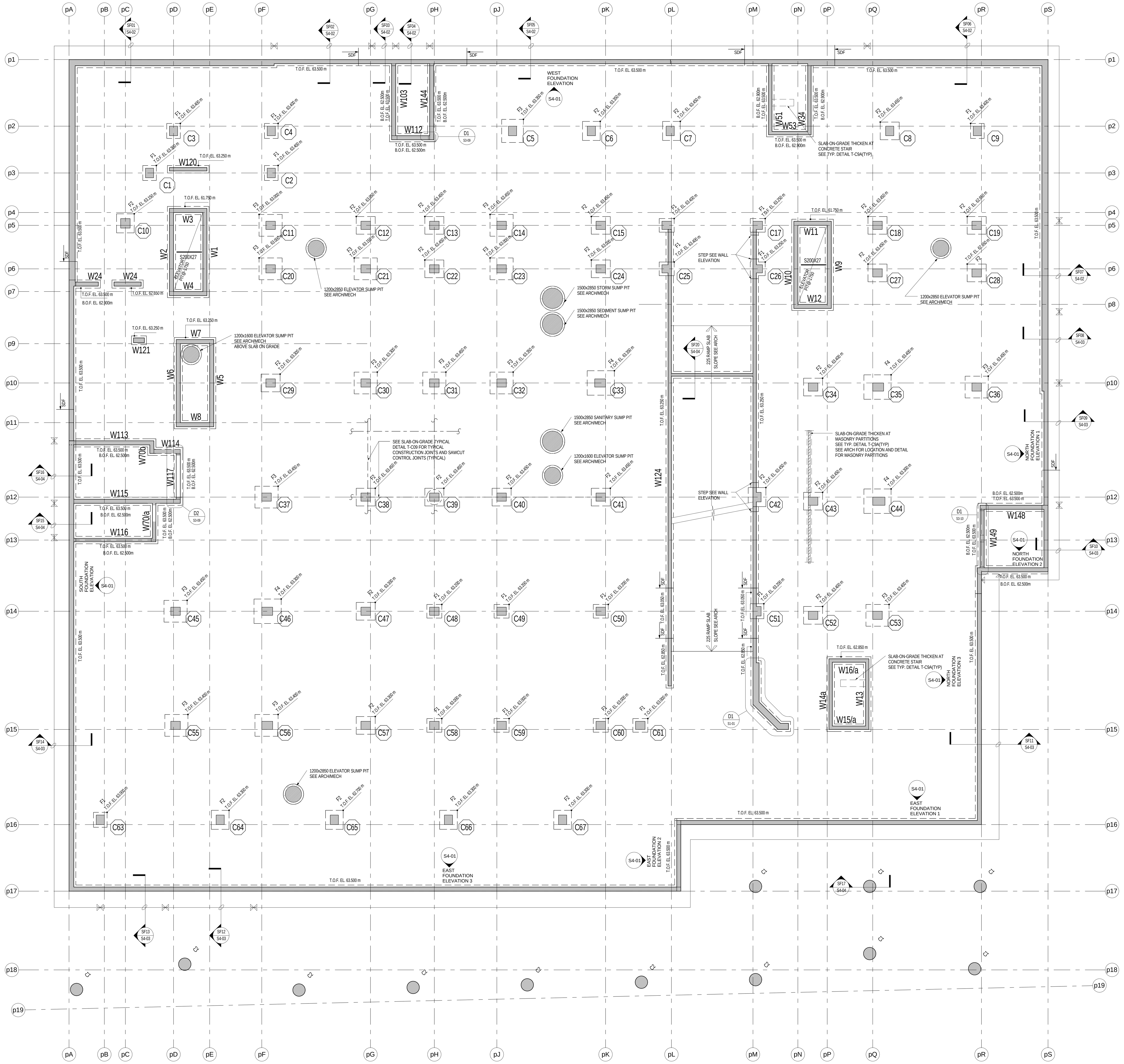
CAP NUMBER	CAP SIZE	CAP REINFORCE MENT	CONCRETE STRENGTH
CP1	1300x1300x700	6-20M BEW	35MPa



D1 DETAIL
S1-01
1 : 25



FOUNDATION PLAN AT GROUND LEVEL
1 : 100



FOUNDATION PLAN
1 : 100

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LICENSED PROFESSIONAL ENGINEER JUNE 12,2015 B. BARADARAN-SEYED PROVINCE OF ONTARIO

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No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR WT REVIEW	May 01,2015

AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
FOUNDATION PLAN

Project
Hines TRIDEL®
AQUAVISTA
Block 3 - R3 & R4
Merchant's Whart
Toronto, ON

Scale
As indicated

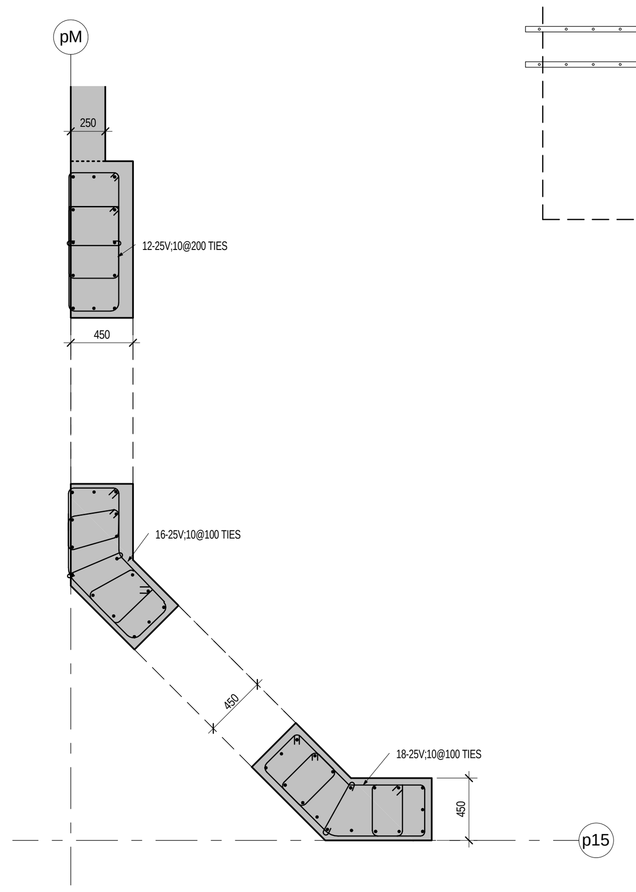
Drawn by
DC

Checked by
BBS

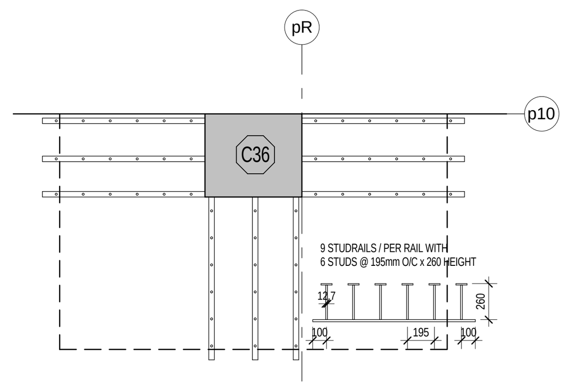
Project No.
20140031

Date
JUNE 12,2015

Drawing No.
S1-01



D1 PLAN DETAIL
S1-02
1 : 25



D2 PLAN DETAIL
S1-02
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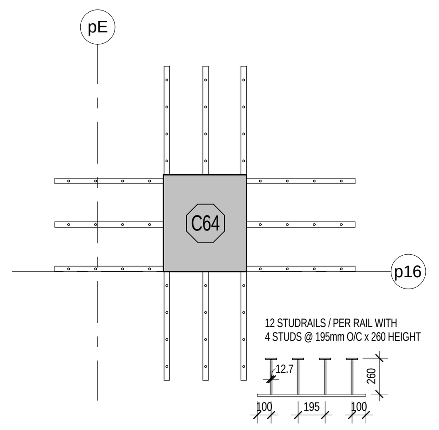

D3 PLAN DETAIL
S1-02
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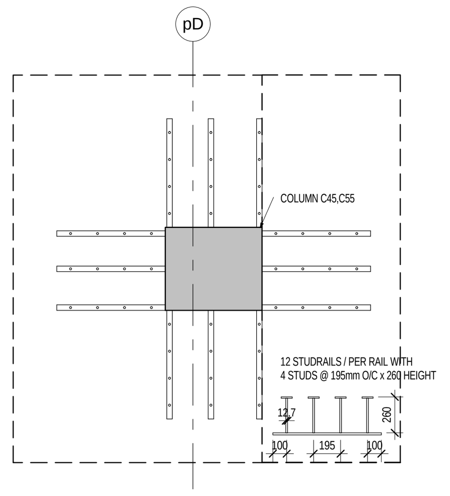
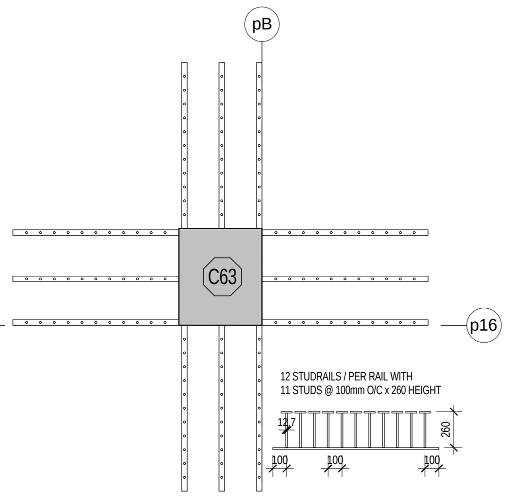
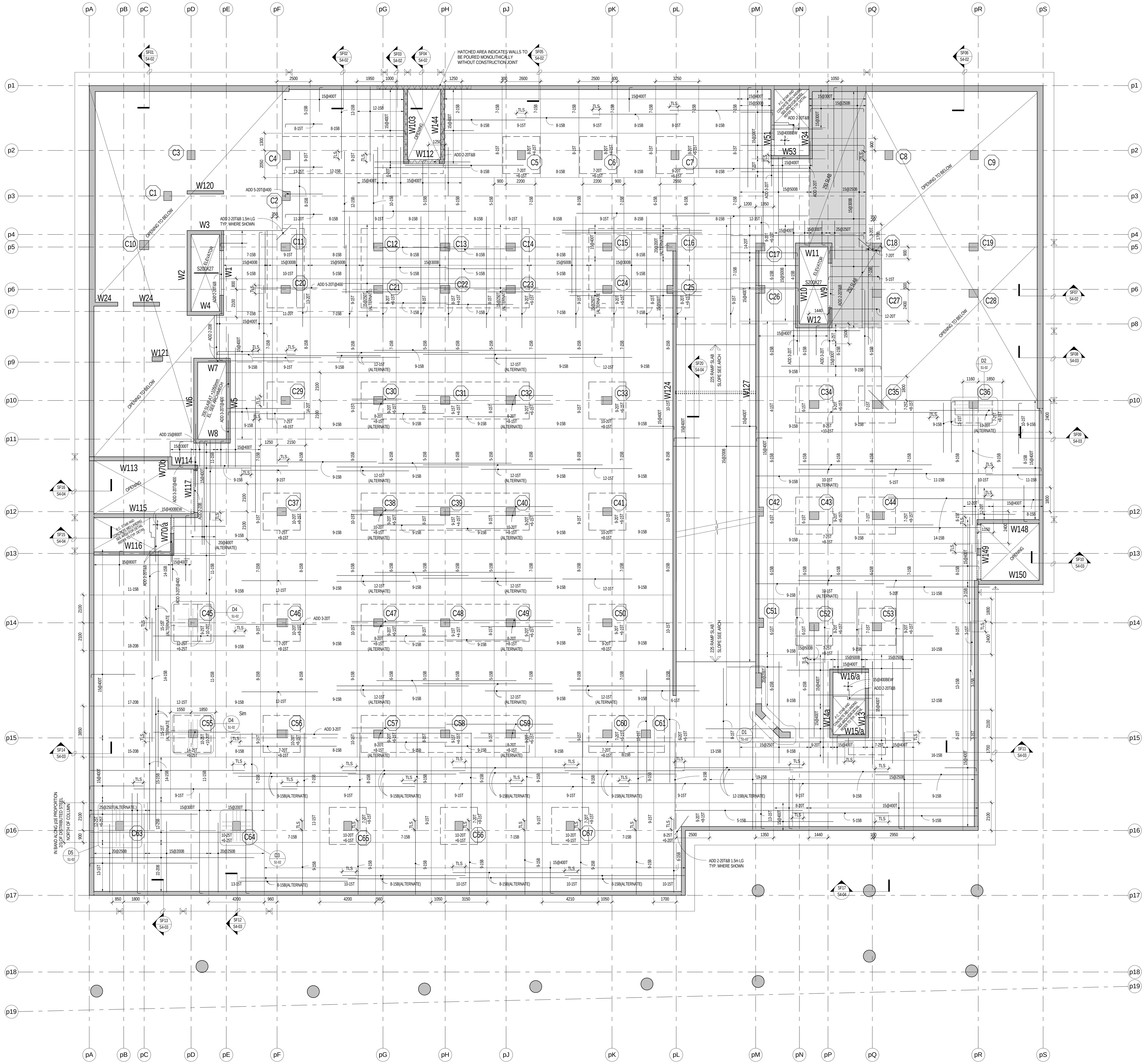


D4 PLAN DETAIL
S1-02
1 : 25



D5 PLAN DETAIL
S1-02
1 : 25



LEVEL P3 FRAMING PLAN
1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED PARKING LEVEL DATUM ELEVATION 66.60m AT CENTRAL ELEVATOR CORE. TOP OF SLAB ELEVATION VARIES AND IS TO BE SLOPED TO SUIT DRAINAGE AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
PARKING AND BIKE	2.4 kPa	0.5 kPa
STORAGE	7.2 kPa	0.5 kPa
STAIRS AND ELEVATOR LOBBIES	4.8 kPa	0.5 kPa

3. CONCRETE SHALL MEET THE REQUIREMENTS FOR CLASS CI EXPOSURE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 35 MPa AT 28 DAYS. REFER ALSO TO COLUMN AND WALL SCHEDULES. SEE ALSO CONCRETE MIX SCHEDULE ON S0-02.
4. CONCRETE COVER FOR TOP BARS IN SLABS TO BE 40 mm, CONCRETE COVER FOR BOTTOM BARS IN SLABS TO BE 30mm.
5. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
6. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
7. SEE TYPICAL DETAIL FOR LINTELS IN NON-LOAD BEARING MASONRY WALLS.
8. SEE ARCHITECTURAL DRAWINGS AND MECHANICAL DRAWINGS FOR CURBS, REINFORCE AS PER TYPICAL DETAIL.
9. SEE COLUMN AND WALL SCHEDULES.
10. SEE ALSO TYPICAL NOTE AND DETAIL DRAWINGS.
11. REFER TO BEAM SCHEDULES.
12. AT SLEEVES THROUGH THE SLAB, HOOK BARS AT OPENING AND ADD BARS OF SAME SIZE ON EACH SIDE OF THE OPENING, EQUAL TO 1/2 THE NUMBER OF BARS HOOKED.
13. PROVIDE 25mm CHAMFER ON ALL EXPOSED EDGES OF COLUMNS, WALLS, BEAMS AND DROPS.
14. AREAS NOTED THUS ▭ ON PLAN, INDICATE THICKENED SLAB: SEE PLAN FOR THICKNESS.
    TRANSFER SLABS TO BE SHORED UNTIL REACH SPECIFIED 28 DAYS CONCRETE STRENGTH.
    SHORING SHOULD NOT BE SUPPORTED ON BUILDING SLABS.
15. PROVIDE DCI AT A DOSAGE OF 10L/m³ CONCRETE IN THE FRAMED RAMP SLAB AND ALL TURNING AREAS.
16. SLAB SOFFIT TO FOLLOW TOP OF SLAB SLOPES TO MAINTAIN SLAB THICKNESS AS NOTED.
17. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.

TUL = TOP UPPER LAYER
TLL = TOP LOWER LAYER
BUL = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB:	200 MIN. u/n
INTEGRITY BARS:	3-20M BEW
TEMP. STEEL:	15@500
DROP:	2800x2800x110DP. u/n
CONCRETE STRENGTH:	35 MPa CLASS C1
SLAB:	225 RAMP SLAB
INTEGRITY BARS:	3-20M BEW
TEMP. STEEL:	15@400
CONCRETE STRENGTH:	35 MPa CLASS C1
SLAB:	250 MIN.
INTEGRITY BARS:	3-20M BEW
TEMP. STEEL:	15@400
CONCRETE STRENGTH:	35 MPa CLASS C1
SLAB:	310 MIN.
INTEGRITY BARS:	3-20M BEW
TEMP. STEEL:	15@300
CONCRETE STRENGTH:	35 MPa CLASS C1

Contractor Must Check And Verify All Dimensions On The Job.

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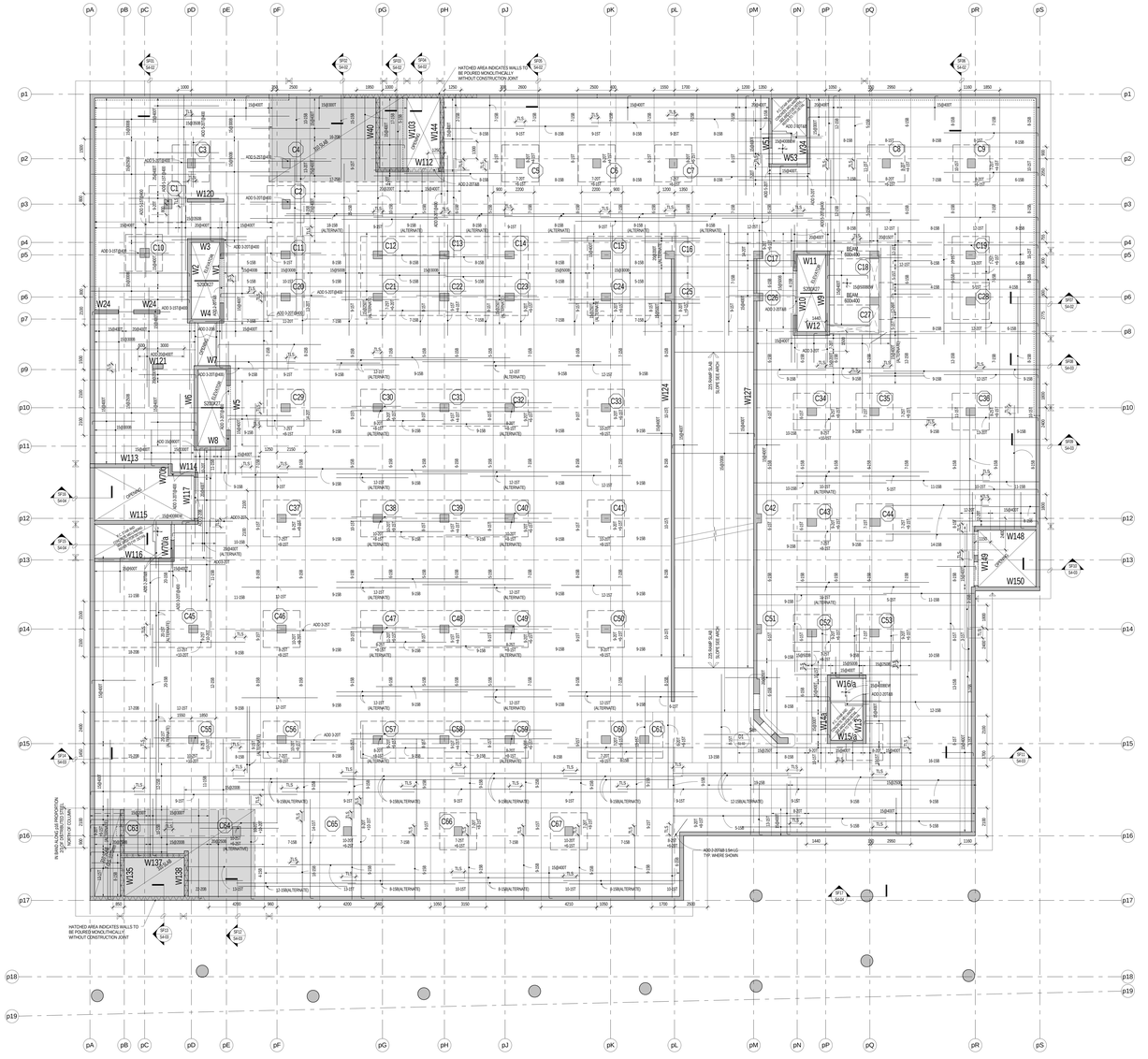
No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR WT REVIEW	May 01,2015

AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL P3 FLOOR FRAMING PLAN

Project
Hines TRIDEL®
AQUAVISTA
Block 3 - R3 & R4
Merchant's Whart
Toronto, ON

Scale: As indicated
Drawn by: DC
Checked by: BK/BBS
Project No.: 20140031
Date: JUNE 12,2015
Drawing No.: S1-02



LEVEL P2 FRAMING PLAN

1 : 100

1. TOP OF STRUCTURAL SLAB VARIES FROM FINISHED PARKING LEVEL DATUM ELEVATION 69.450m AT ELEVATOR CORE. TOP OF SLAB ELEVATION VARIES AND IS TO BE SLOPED TO SUIT DRAINAGE AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
PARKING	2.4 kPa	0.5 kPa
STORAGE AND LOCKERS	7.2 kPa	0.5 kPa
STAIRS AND ELEVATOR LOBBIES	4.8 kPa	2.5 kPa
MECHANICAL AND ELECTRICAL	3.6 kPa	6.0 kPa
LOBBY	4.8 kPa	2.5 kPa
WATER TANK	12.0 kPa	1.5 kPa
WHIRLPOOL / SPA AREA	10kPa	1.5 kPa

3. CONCRETE SHALL MEET THE REQUIREMENTS FOR CLASS C EXPOSURE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 35 MPa AT 28 DAYS. REFER ALSO TO COLUMN AND WALL SCHEDULES. SEE ALSO CONCRETE MIX SCHEDULE ON FOUNDATION PLAN.
4. CONCRETE COVER FOR TOP BARS IN SLABS TO BE 40 mm. CONCRETE COVER FOR BOTTOM BARS IN SLABS TO BE 25 mm.
5. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
6. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
7. SEE TYPICAL DETAIL FOR LINTELS IN NON-LOAD BEARING MASONRY WALLS.
8. REFER TO ARCHITECTURAL, MECHANICAL AND MECHANICAL DRAWINGS FOR CURBS. REINFORCE AS PER TYPICAL DETAIL.
9. SEE COLUMN AND WALL SCHEDULES.
10. SEE ALSO TYPICAL NOTE AND DETAIL DRAWINGS.
11. REFER TO BEAM SCHEDULES.
12. AT SLEEVES THROUGH THE SLAB, HOOK BARS AT OPENING AND ADD BARS OF SAME SIZE ON EACH SIDE OF OPENING. EXTEND BARS 600 mm BEYOND TO CLEAR SPAN.
13. PROVIDE 20mm CHAMFER ON ALL EXPOSED EDGES OF COLUMNS, WALLS, BEAMS AND DROPS.
14. AREAS NOTED THUS [ ] ON PLAN, INDICATE THICKENED SLAB. SEE PLAN FOR THICKNESS.
    TRANSFER SLABS TO BE SHORED UNTIL REACH SPECIFIED 28 DAYS CONCRETE STRENGTH.
    SHORING SHOULD NOT BE SUPPORTED ON BUILDING SLABS.
15. PROVIDE DC/ AT A DOSAGE OF 10L/M³ CONCRETE IN THE FRAMED RAMP SLAB AND ALL TURNING AREAS.
16. SLAB SOFFIT TO FOLLOW TOP OF SLAB SLOPES TO MAINTAIN SLAB THICKNESS AS NOTED.
17. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL.

TLL = TOP UPPER LAYER
TLL = TOP LOWER LAYER
BUL = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB:	200 MIN. slr
INTEGRITY BARS:	3-20M BEW
TEMP. STEEL:	15@400
DROP:	2800x2800x1100F slr
CONCRETE STRENGTH:	35 MPa CLASS C1
SLAB:	225 RAMP SLAB
INTEGRITY BARS:	3-20M BEW
TEMP. STEEL:	15@400
CONCRETE STRENGTH:	35 MPa CLASS C1
SLAB:	310 MIN.
INTEGRITY BARS:	3-20M BEW
TEMP. STEEL:	15@300
CONCRETE STRENGTH:	35 MPa CLASS C1

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

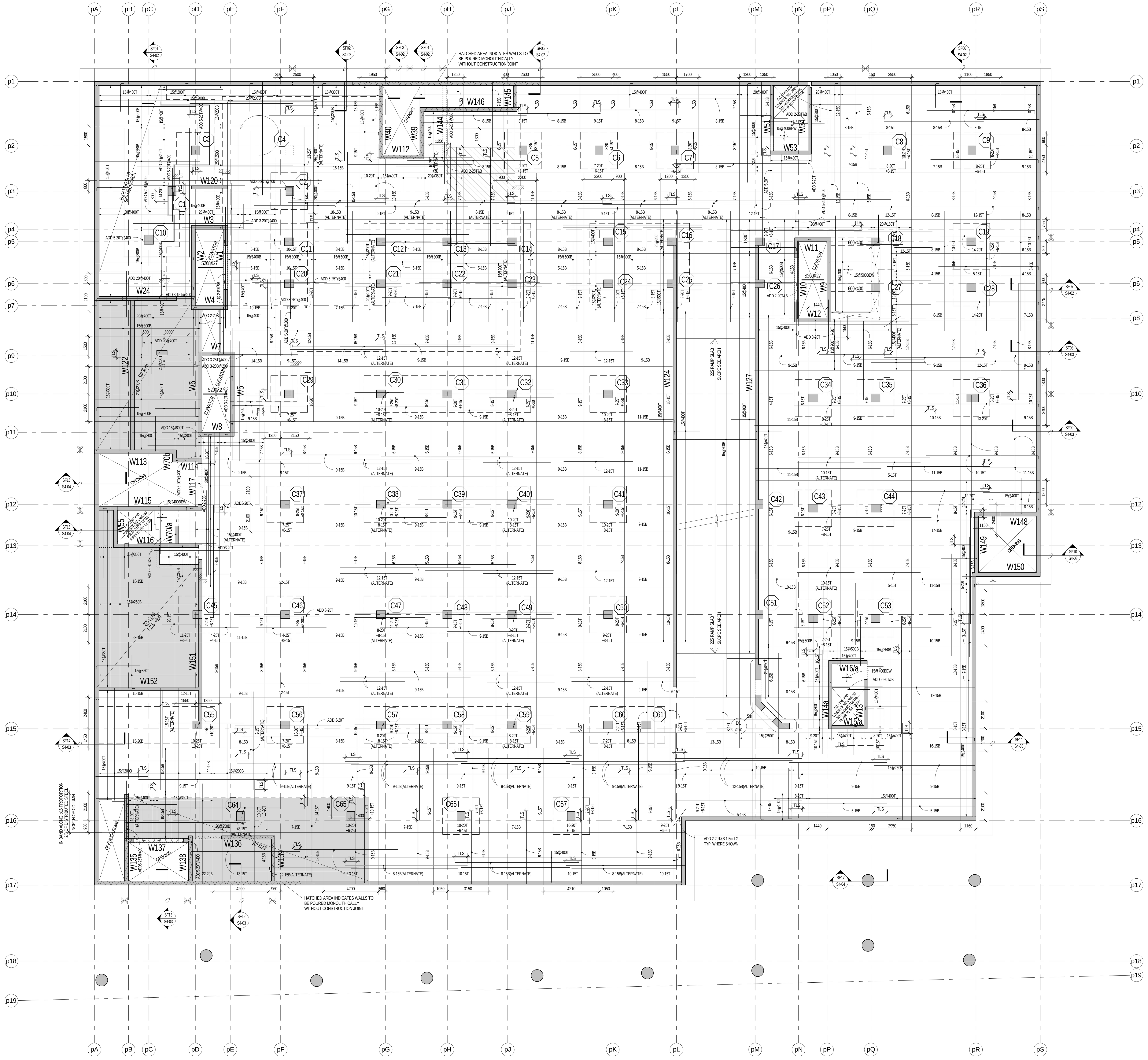
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL P2 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	DC
Checked by	BK/BBS
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S1-03

Plot Date: 6/11/2015 12:03:34 PM



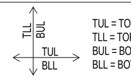
LEVEL P1 FRAMING PLAN

1 : 100

1. TOP OF STRUCTURAL SLAB VARIES FROM FINISHED PARKING LEVEL DATUM ELEVATION 72.200m AT ELEVATOR CORE. TOP OF SLAB ELEVATION VARIES AND IS TO BE SLOPED TO SUIT DRAINAGE AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
PARKING	2.4 kPa	0.5 kPa
STORAGE AND LOCKERS	7.2 kPa	0.5 kPa
STAIRS AND ELEVATOR LOBBIES	4.8 kPa	0.5 kPa
MECANICAL AND ELECTRICAL	3.6 kPa	6.0 kPa
AMENITY	4.8 kPa	1.5 kPa
WATER TANK	10 kN/m³	1.0 kPa
WHIRLPOOL / SPA AREA	10kPa	1.5 kPa

3. CONCRETE SHALL MEET THE REQUIREMENTS FOR CLASS CI EXPOSURE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 35 MPa AT 28 DAYS. REFER ALSO TO COLUMN AND WALL SCHEDULES. SEE ALSO CONCRETE MIX SCHEDULE ON FOUNDATION PLAN.
4. CONCRETE COVER FOR TOP BARS IN SLABS TO BE 40 mm, CONCRETE COVER FOR BOTTOM BARS IN SLABS TO BE 30mm.
5. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
6. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
7. SEE TYPICAL DETAIL FOR LINTELS IN NON-LOAD BEARING MASONRY WALLS.
8. SEE ARCHITECTURAL DRAWINGS AND MECHANICAL DRAWINGS FOR CURBS, REINFORCE AS PER TYPICAL DETAIL.
9. SEE COLUMN AND WALL SCHEDULES.
10. SEE ALSO TYPICAL NOTE AND DETAIL DRAWINGS.
11. REFER TO BEAM SCHEDULES.
12. AT SLEEVES THROUGH THE SLAB, HOOK BARS AT OPENING AND ADD BARS OF SAME SIZE ON EACH SIDE OF THE OPENING, EQUAL TO 1/2 THE NUMBER OF BARS HOOKED.
13. PROVIDE 25mm CHAMFER ON ALL EXPOSED EDGES OF COLUMNS, WALLS, BEAMS AND DROPS.
14. AREAS NOTED THUS [shaded] ON PLAN, INDICATE THICKENED SLAB: SEE PLAN FOR THICKNESS.
    AREAS NOTED THUS [hatched] ON PLAN, ARE BUILT-UP CONCRETE. SEE TYPICAL DETAILS.
    TRANSFER SLABS TO BE SHORED UNTIL REACH SPECIFIED 28 DAYS CONCRETE STRENGTH.
    SHORING SHOULD NOT BE SUPPORTED ON BUILDING SLABS.
15. PROVIDE DCI AT A DOSAGE OF 10L/m³ CONCRETE IN THE FRAMED RAMP SLAB AND ALL TURNING AREAS.
16. SLAB SOFFIT TO FOLLOW TOP OF SLAB SLOPES TO MAINTAIN SLAB THICKNESS AS NOTED.
17. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL.



TUL = TOP UPPER LAYER
TLL = TOP LOWER LAYER
BUL = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB:	200 MIN. u/n
INTEGRITY BARS:	3-20M BEW
TEMP. STEEL:	15@500
DROP:	2800x2800x110DP. u/n
CONCRETE STRENGTH:	35 MPa CLASS C1
SLAB:	225 RAMP SLAB
INTEGRITY BARS:	3-20M BEW
TEMP. STEEL:	15@400
CONCRETE STRENGTH:	35 MPa CLASS C1
SLAB:	275 MIN.
INTEGRITY BARS:	3-20M BEW
TEMP. STEEL:	15@400
CONCRETE STRENGTH:	35 MPa CLASS C1
SLAB:	310 MIN.
INTEGRITY BARS:	3-20M BEW
TEMP. STEEL:	15@300
CONCRETE STRENGTH:	35 MPa CLASS C1

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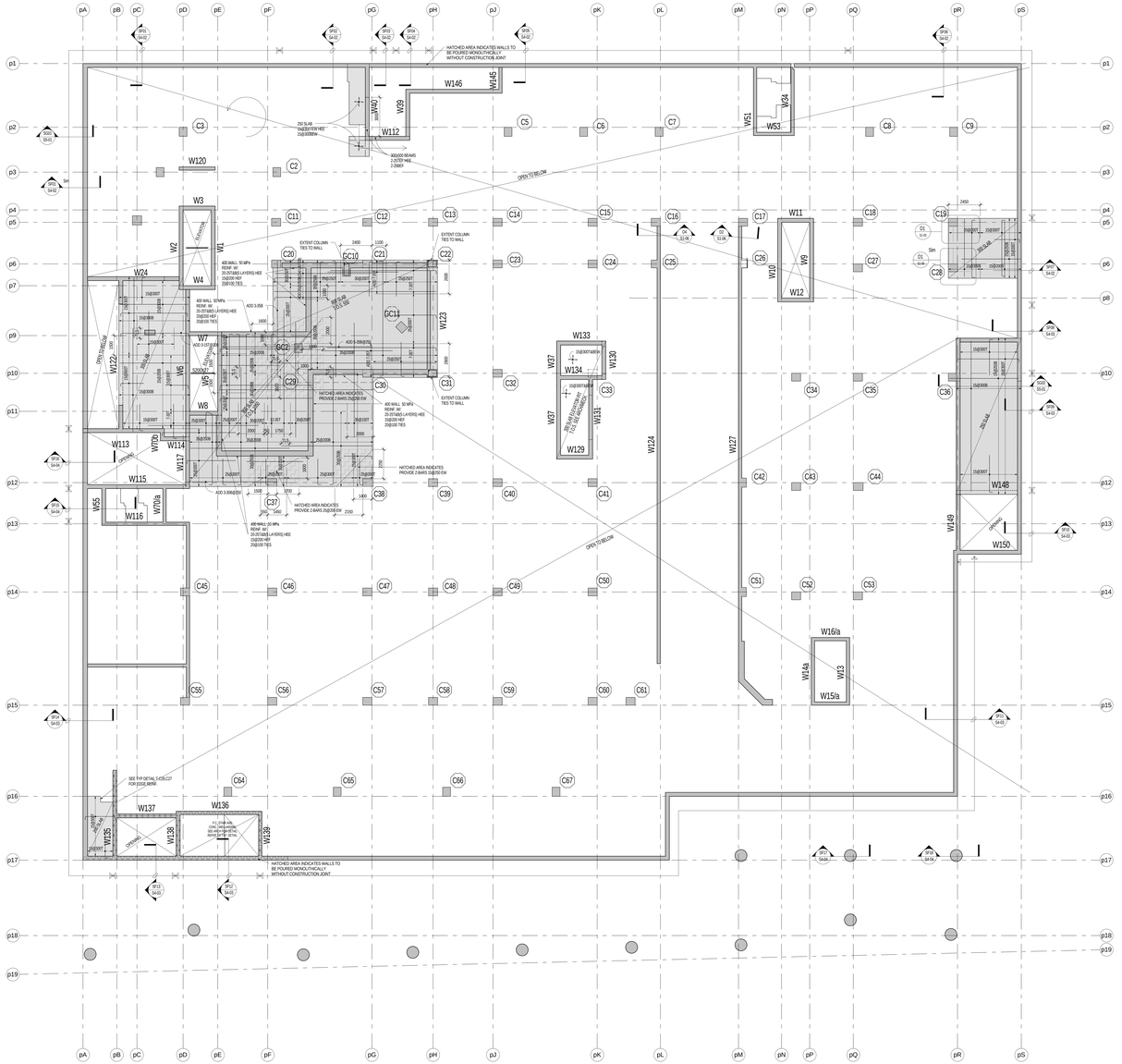
No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR WT REVIEW	May 01,2015

AQUAVISTA AT BAYSIDE TORONTO

Drawing Title
LEVEL P1 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale: As indicated
Drawn by: DC
Checked by: BK/BBS
Project No.: 20140031
Date: JUNE 12,2015
Drawing No.: S1-04



LEVEL P1 MEZZ. FRAMING PLAN

1 : 100

1. TOP OF LEVEL 1 MEZZANINE FLOOR STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED GROUND FLOOR DATUM ELEVATION TO DR. THE STRUCTURAL SLAB STEPS AT THE EDGE OF GROUND FLOOR. SEE PLAN AND DETAILS FOR ELEVATION CHANGES. SEE ALSO ARCHITECTURAL DRAWINGS AND LANDSCAPE DRAWINGS FOR GARAGE ROOF ELEVATIONS AND SLOPES TO DRAINS.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL), IN ADDITION TO SELF WEIGHT:

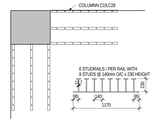
	LL	SDL
STAIRS / MEZZANINE	4.8 kPa	0.5 kPa

3. CONCRETE SHALL MEET THE REQUIREMENTS FOR CLASS C1 EXPOSURE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 35 MPa AT 28 DAYS. REFER ALSO TO COLUMN AND WALL SCHEDULES. SEE ALSO CONCRETE MIX SCHEDULE ON FOUNDATION PLAN.
4. CONCRETE COVER FOR TOP BARS IN SLABS TO BE 25 mm. CONCRETE COVER FOR BOTTOM BARS IN SLABS TO BE 30 MM.
5. ALL OPENINGS, MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
6. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 48 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
7. SEE TYPICAL DETAIL FOR LINTELS IN NON-LOAD BEARING MASONRY WALLS.
8. SEE ARCHITECTURAL DRAWINGS AND MECHANICAL DRAWINGS FOR CURBS. REINFORCE AS PER TYPICAL DETAIL.
9. SEE COLUMN AND WALL SCHEDULES.
10. SEE ALSO TYPICAL NOTE AND DETAIL DRAWINGS.
11. REFER TO BEAM SCHEDULES.
12. PROVIDE CHAMFER STRIPS TO ALL EXPOSED EDGES OF COLUMNS, WALLS, BEAMS & DROPS.
13. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.

TLL = TOP LAYER LAYER
TUL = TOP UPPER LAYER
BLL = BOTTOM LAYER
BUL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	250 MIN. 3.0M BEW 35 MPa 15@400 ulh
SLAB INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	250 MIN. 3.0M BEW 35 MPa 15@400 ulh
SLAB INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	300 MIN. 3.0M BEW 35 MPa 15@300 ulh
SLAB INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	800 MIN. 5.0M BEW 45 MPa 25@300 ulh



D1 / S1-05 DETAIL
1 : 25

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

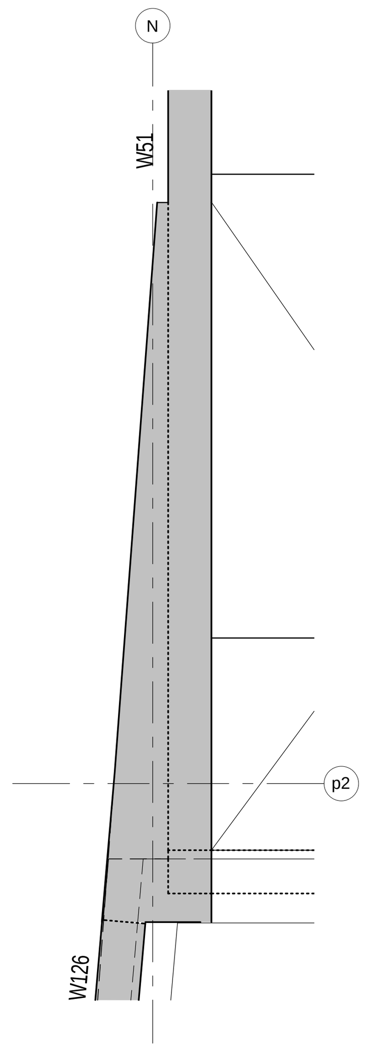
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL P1 MEZZANINE FRAMING PLAN

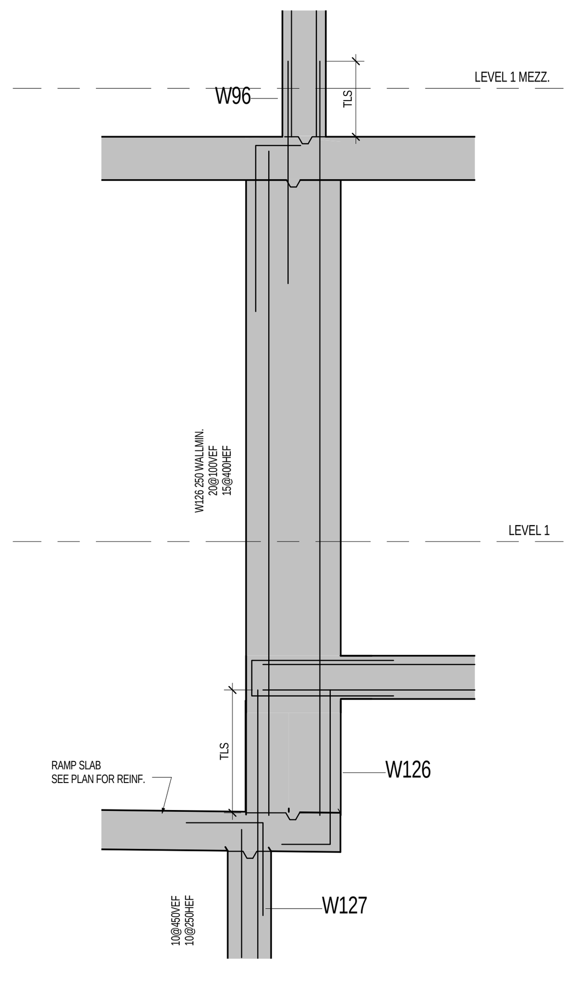
Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	DC
Checked by	BK/BBS
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S1-05

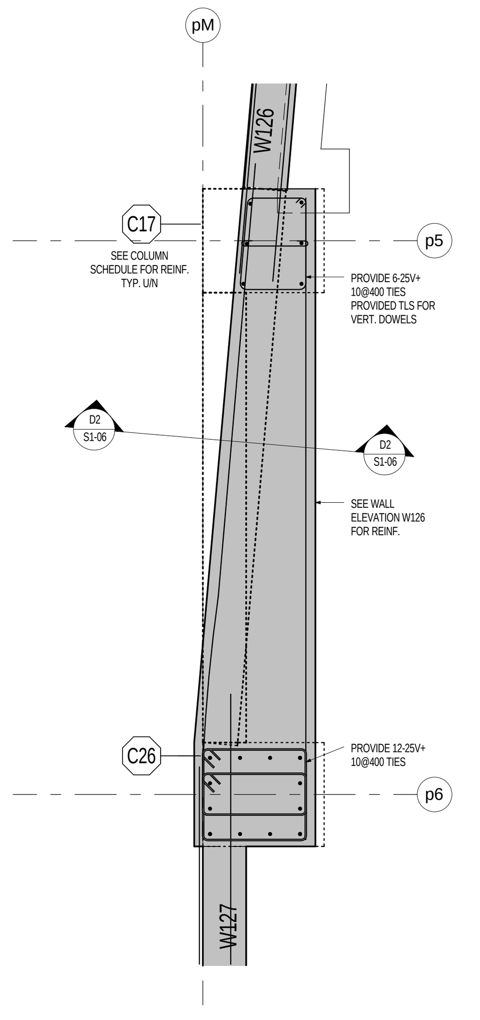
Plot Date: 6/11/2015 12:03:41 PM



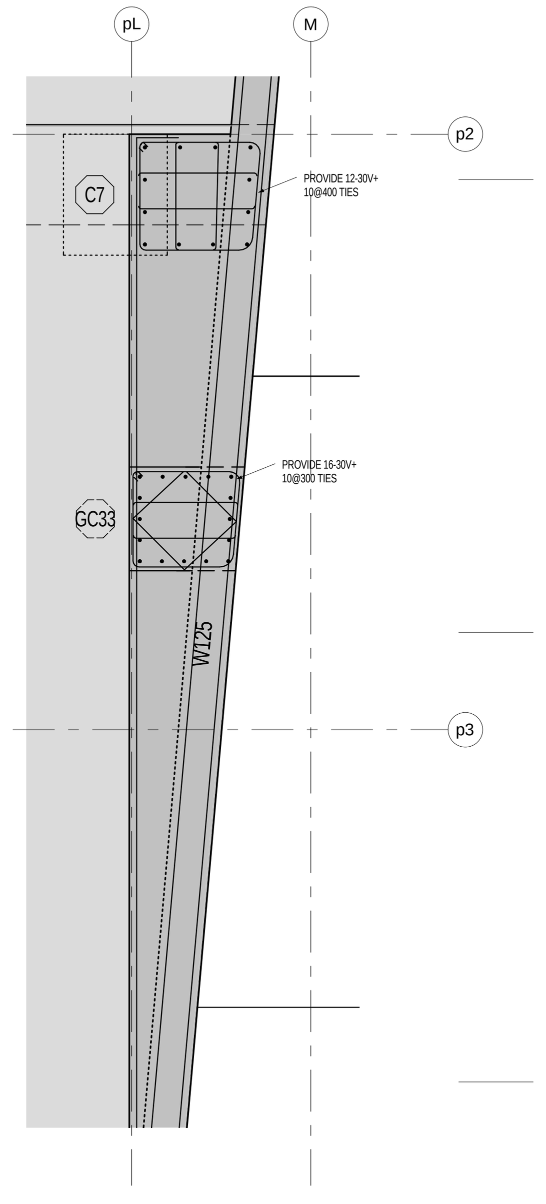
D5 PLAN DETAIL
S1-06 1 : 20



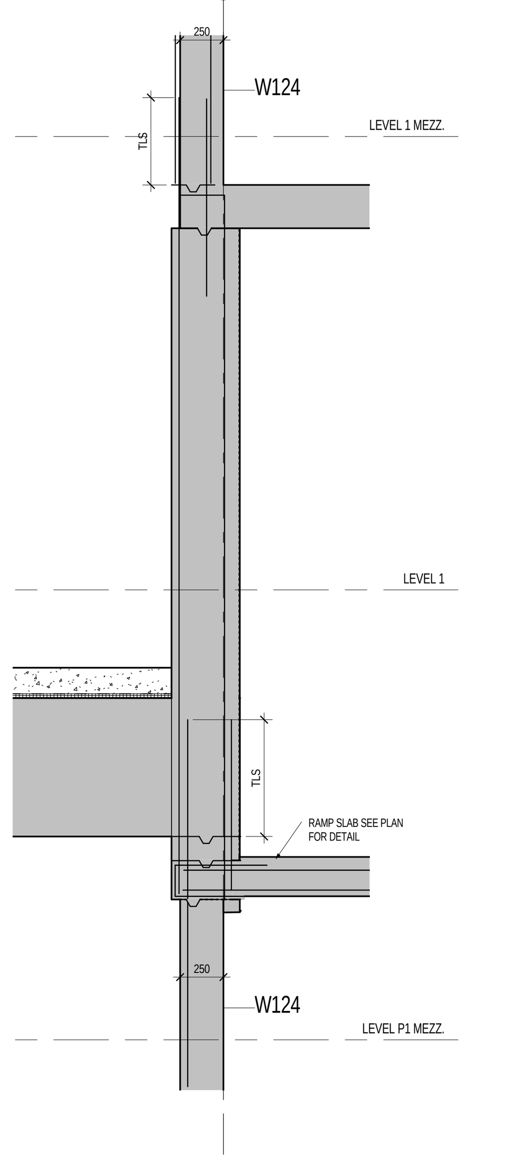
D2 ELEVATION DETAIL
S1-06 1 : 20



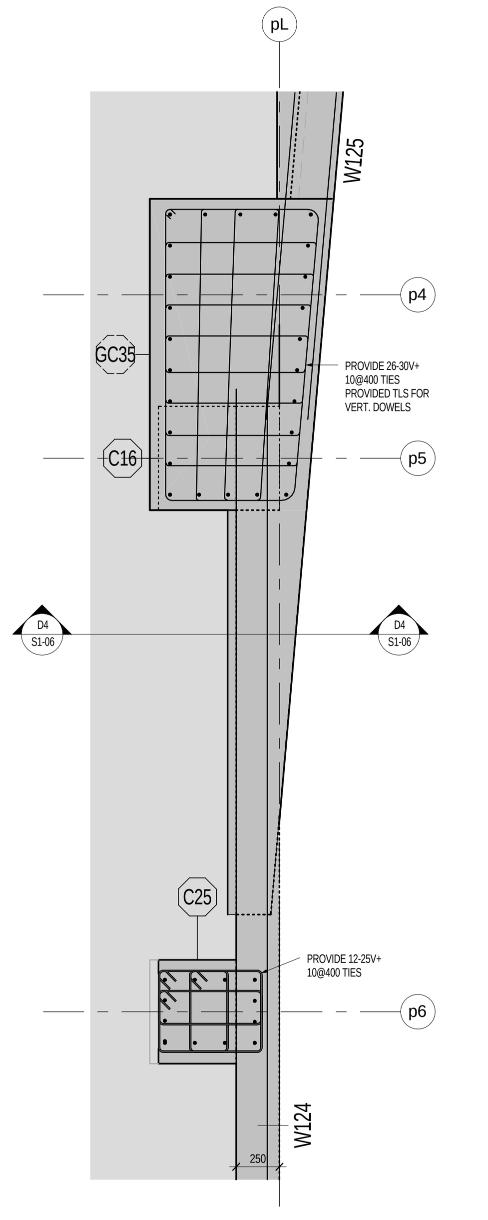
D1 PLAN DETAIL
S1-06 1 : 20



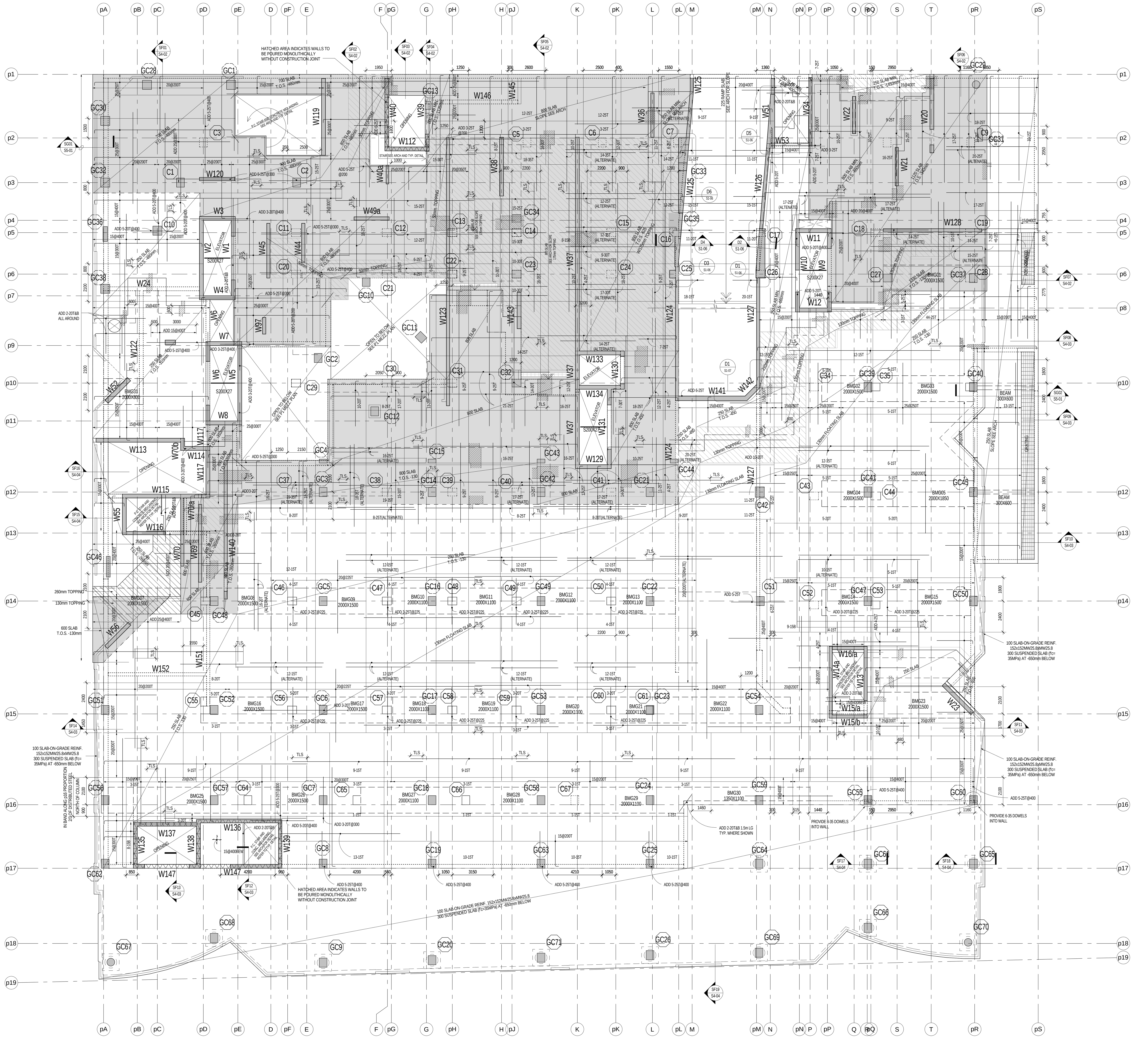
D6 PLAN DETAIL
S1-06 1 : 20



D4 PLAN DETAIL
S1-06 1 : 20



D3 PLAN DETAIL
S1-06 1 : 20



LEVEL 1 FRAMING PLAN WITH TOP REINFORCEMENT

1 : 100

1. TOP OF GROUND FLOOR STRUCTURAL SLAB VARIES FROM FINISHED GROUND FLOOR DATUM ELEVATION 77.600m. THE STRUCTURAL SLAB STEPS AT THE EDGE OF GROUND FLOOR. SEE PLAN AND DETAILS FOR ELEVATION CHANGES. SEE ALSO ARCHITECTURAL DRAWINGS AND LANDSCAPE DRAWINGS FOR GARAGE ROOF ELEVATIONS AND SLOPES TO DRAINS.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO SELF WEIGHT. AND TOPPING LOADS AS SHOWN ON PLAN.

	LL	SDL
RETAIL SPACE	4.8 kPa	1.5 kPa
LOBBY,LOUNGE,AMENITY SPACES	4.8 kPa	1.5 kPa
STAIRS	4.8 kPa	0.5 kPa
LOCKERS / STORAGE	4.8 kPa	0.5 kPa
LOADING DOCK AND MOVING ROOM	4.8 kPa	0.5 kPa
PARKING RAMP	2.4kPa	0.5kPa
RETAIL CHILLER/GARBAGE ROOM	6.0kPa	3.6kPa
LOADING RAMP	12.0kPa	0.5kPa

3. CONCRETE SHALL MEET THE REQUIREMENTS FOR CLASS C1 EXPOSURE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 35 MPa AT 28 DAYS. REFER ALSO TO COLUMN AND WALL SCHEDULES. SEE ALSO CONCRETE MIX SCHEDULE ON FOUNDATION PLAN.
4. CONCRETE COVER FOR TOP BARS IN SLABS TO BE 25 mm, CONCRETE COVER FOR BOTTOM BARS IN SLABS TO BE 30 mm.
5. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
6. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 48 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
7. SEE TYPICAL DETAIL FOR LINTELS IN NON-LOAD BEARING MASONRY WALLS.
8. SEE ARCHITECTURAL DRAWINGS AND MECHANICAL DRAWINGS FOR CURBS, REINFORCE AS PER TYPICAL DETAIL.
9. SEE COLUMN AND WALL SCHEDULES.
10. SEE ALSO TYPICAL NOTE AND DETAIL DRAWINGS.
11. REFER TO BEAM SCHEDULES.
12. PROVIDE 25mm CHAMFER ON ALL EXPOSED EDGES OF COLUMNS, WALLS, BEAMS & DROPS.
13. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.
14. PROVIDE TLS FOR ALL TRANSFER SLAB REINFORCEMENT.

TUL = TOP UPPER LAYER
TLL = TOP LOWER LAYER
BUL = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	250 MIN. u/n 3-20M BEW 35 MPa CLASS C1 15@400 u/n	SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	225 RAMP 3-20M BEW 35 MPa CLASS C1 15@400 u/n
SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	300 MIN. 3-20M BEW 35 MPa CLASS C1 15@300 u/n	SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	600 MIN. 3-20M BEW 45 MPa CLASS C1 20@250 u/n
SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	700 MIN. 3-30M BEW 45 MPa CLASS C1 20@200 u/n	SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	800 MIN. 5-30M BEW 45 MPa CLASS C1 25@300 u/n
SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	1120 MIN. 5-30M BEW 45 MPa CLASS C1 25@200 u/n	SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	1330 MIN. 5-30M BEW 45 MPa CLASS C1 25@200 u/n

GARAGE ROOF PLAN NOTES

1. TOP OF GARAGE ROOF SLAB ELEVATION VARIES AND IS TO BE SLOPED TO SUIT DRAINAGE AND RAMPING AS SHOWN ON THE ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL DRAWINGS.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
LANSCAPED EMERGENCY VEHICLE ACCESSIBLE AREAS	12.0 kPa	21 kPa

3. CONCRETE SHALL MEET THE REQUIREMENTS FOR CLASS C1 EXPOSURE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 35 MPa AT 28 DAYS. REFER ALSO TO COLUMN AND WALL SCHEDULES. SEE TYPICAL CONCRETE MIX SCHEDULE ON FOUNDATION PLAN.
4. CONCRETE COVER FOR TOP BARS IN SLABS TO BE 40 mm, CONCRETE COVER FOR BOTTOM BARS IN SLABS TO BE 30mm.
5. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
6. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 48 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
7. SEE TYPICAL DETAIL FOR LINTELS IN NON-LOAD BEARING MASONRY WALLS.
8. SEE ARCHITECTURAL DRAWINGS AND MECHANICAL DRAWINGS FOR CURBS, REINFORCE AS PER TYPICAL DETAIL.
9. SEE COLUMN AND WALL SCHEDULES.
10. SEE ALSO TYPICAL NOTE AND DETAIL DRAWINGS.
11. REFER TO BEAM SCHEDULES.
12. AT SLEEVES THROUGH THE SLAB, HOOK BARS AT OPENING AND ADD BARS OF SAME SIZE ON EACH SIDE OF THE OPENING, EQUAL TO 1/2 THE NUMBER OF BARS HOOKED.
13. PROVIDE 25mm CHAMFER ON ALL EXPOSED EDGES OF COLUMNS, WALLS, BEAMS, AND DROPS.
14. AREAS NOTED THUS ON PLAN, ARE BUILT-UP CONCRETE. SEE TYPICAL DETAILS.
15. AREAS NOTED THUS ON PLAN, INDICATE THICKENED SLAB: SEE PLAN FOR THICKNESS.
TRANSFER SLABS TO BE SHORED UNTIL REACH SPECIFIED 28 DAYS CONCRETE STRENGTH.
SHORING SHOULD NOT BE SUPPORTED ON BUILDING SLABS.
16. PROVIDE DCI AT A DOSAGE OF 10L/m³ CONCRETE IN THE VIA VELO, BICYCLE PARKING AREA AND RAMPS (ALL EXTERIOR AREAS).
17. SEE LANDSCAPE DRAWINGS FOR PLANTER WALLS AND PILLARS. REINFORCE AS PER TYPICAL DETAILS.
18. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.
19. PROVIDE TLS FOR ALL TRANSFER SLAB REINFORCEMENT.
20. IN TRANFER SLABS, HOOK BOTTOM BARS AND EXTEND ALL TOP BARS INTO ADJACENT SLABS BY 2000mm (TYP)

Contractor Must Check And Verify All Dimensions On The Job.

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No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR WT REVIEW	May 01,2015

AQUAVISTA AT BAYSIDE TORONTO

Drawing Title
LEVEL 1-GROUND FLOOR FRAMING PLAN WITH TOP REINFORCEMENT AND DETAILS

Project
Hines TRIDEL®
AQUAVISTA
Block 3 - R3 & R4
Merchant's Whart
Toronto, ON

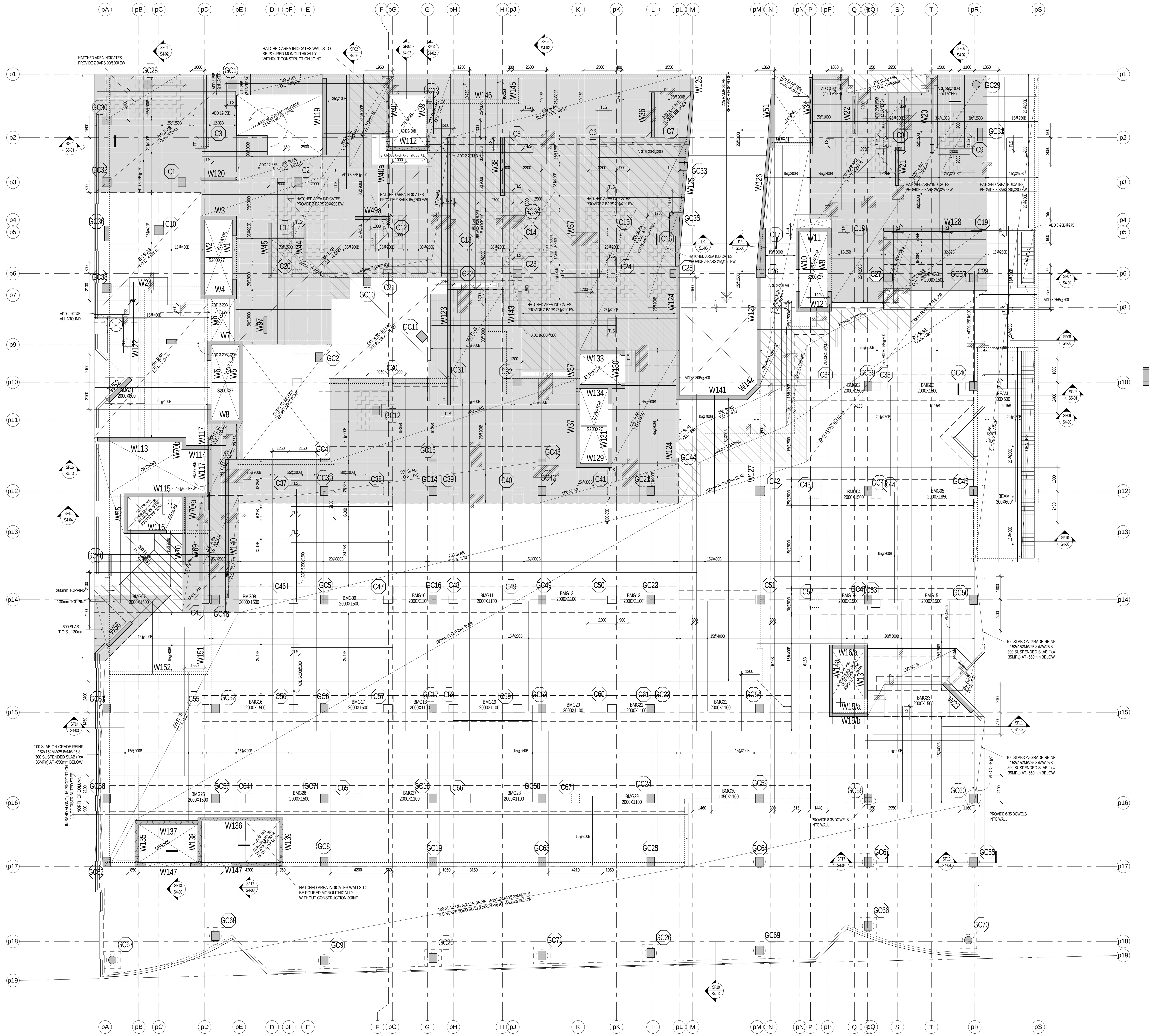
Scale	As indicated
Drawn by	DC
Checked by	BK/BBS
Project No.	20140031
Date	JUNE 12,2015
Drawing No.	S1-06

LEVEL 1 BEAM SCHEDULE

BEAM NUMBER	SIZE	BOTTOM BARS No.	BOTTOM BARS SIZE	BOTTOM BARS	TOP BARS No.	TOP BARS SIZE	TOP BARS	STIRRUPS SIZE	STIRRUPS	REMARKS
BMG01	2000X1500	12	35	BLL	6	30	TUL	15	10@150 RE	TRIPLES
		10	15	5HEF	5	35	TUL 2000	15	@300 REM	DOUBLES
					5	35	2000 TUL			
BMG02	2000X1500	22	35	BLL	9	30	2600 TUL	15	@125	FIVE SETS
		22	35	BUL	6	35	1400 TUL			
		10	15	5HEF	12	30	TUL 3600 4900			
					8	35	TUL 2000 2000			
					6	30	TLL 2000 2000			
BMG03	2000X1500	12	35	BLL	12	30	FROM BMG02 TUL	15	5@125 RE	TRIPLES
		12	35	BUL	8	35	FROM BMG02 TUL	15	6@200 LE	TRIPLES
		10	15	5HEF	6	30	FROM BMG02 TLL	15	@300 REM	DOUBLES
					8	30	TUL 3600			
					9	35	TUL 2000			
BMG04	2000X1500	22	35	BLL	6	35	2600 TUL	15	@100	FIVE SETS
		12	35	BUL	7	35	1400 TUL			
		12	15	6HEF	12	35	TUL 3400 4300			
					5	35	TUL 2000 2000			
					5	35	TLL 2000 2000			
BMG05	2000X1850	14	35	BLL	12	35	FROM BMG04 TUL	15	10@100 RE	FIVE SETS
		14	35	BUL	5	35	FROM BMG04 TUL	15	@200 REM	FIVE SETS
		16	15	8HEF	5	35	FROM BMG04 TLL			
					5	35	TUL FROM BMG06			
					5	35	TLL 2700 2700			
BMG06 (DELETED)	1475x900	6	30	BLL	5	35	FROM BMG05 TUL	10	@200	TRIPLES
		6	15	3HEF	8	35	4900 TUL			
BMG07	2000X1500	7	30	BLL	6	30	3700 TUL	10	@300	TRIPLES
		12	15	6HEF	8	30	TUL 5300 4000			
					7	30	TLL 2100 2100			
BMG08	2000X1500	16	30	BLL	8	30	FROM BMG07 TUL	10	5@125 LE	TRIPLES
		12	15	6HEF	7	30	FROM BMG07 TLL	10	@300 REM	TRIPLES
					17	35	TUL 4000 4300			
					17	35	TLL 2000 2000			
BMG09	2000X1500	19	35	BLL	17	35	FROM BMG08 TUL	15	@100	FIVE SETS
		14	35	BUL	17	35	FROM BMG08 TLL			
		12	15	6HEF	11	35	TUL 4300 3100			
					8	35	TLL 2000 2000			
BMG10	2000X1100	15	35	BLL	11	35	FROM BMG09 TUL	10	@300	TRIPLES
		8	15	4HEF	8	35	FROM BMG09 TLL			
					6	35	TUL 3100 3100			
					5	35	TLL 1400 1400			
BMG11	2000X1100	6	25	BLL	6	35	FROM BMG10 TUL	10	@300	TRIPLES
		8	15	4HEF	5	35	FROM BMG10 TLL			
					7	35	TUL 3100 4300			
					7	30	TLL 2000 2000			
BMG12	2000X1100	10	30	BLL	7	35	FROM BMG11 TUL	10	8@200 RE	TRIPLES
		10	30	BUL	7	30	FROM BMG11 TLL	10	10@150 LE	TRIPLES
		8	15	4HEF	8	35	TUL 4300 3700	10	@300 REM	TRIPLES
					7	30	TLL 2000 2000			
BMG13	2000X1100	10	30	BLL	8	35	FROM BMG12 TUL	10	@300	TRIPLES
		10	30	BUL	7	30	FROM BMG12 TLL			
		8	15	4HEF	9	35	TUL 2600			
BMG14	2000X1500	18	35	BLL	6	35	2600 TUL	15	5@100 RE	FIVE SETS
		14	35	BUL	5	35	1400 TUL	15	@200 REM	TRIPLES
		10	15	5HEF	15	35	TUL 3400 4900			
					9	35	TLL 2000 2000			
BMG15	2000X1500	16	35	BLL	15	35	FROM BMG14 TUL	15	10@150 RE	FIVE SETS
		12	35	BUL	9	35	FROM BMG14 TLL	15	@300 REM	TRIPLES
		12	15	6HEF	12	35	TUL 3600			
					8	35	TLL 2000			
BMG16	2000X1500	15	35	BLL	12	30	4100 TUL	15	@150	FOUR SETS
		11	30	BUL	10	25	2000 TUL			
		12	15	6HEF	15	35	TUL 4100 4400			
					12	35	TLL 2100 2100			

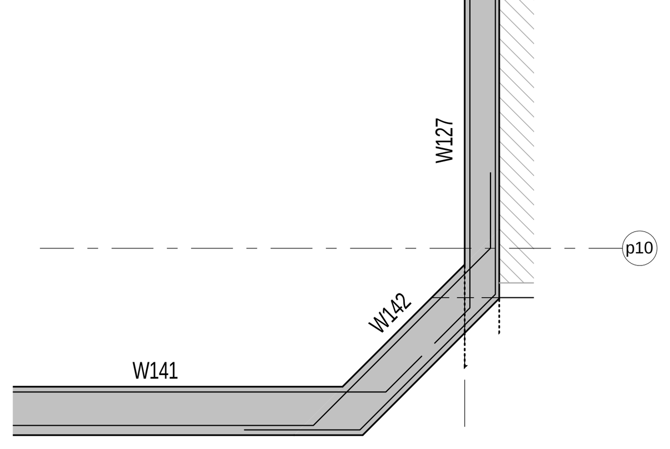
LEVEL 1 BEAM SCHEDULE

BEAM NUMBER	SIZE	BOTTOM BARS No.	BOTTOM BARS SIZE	BOTTOM BARS	TOP BARS No.	TOP BARS SIZE	TOP BARS	STIRRUPS SIZE	STIRRUPS	REMARKS
BMG17	2000X1500	15	35	BLL	15	35	FROM BMG16 TUL	15	50@100 LE	FOUR SETS
		11	30	BUL	12	35	FROM BMG16 TLL	15	@150 REM	TRIPLES
		12	15	6HEF	15	35	TUL 4400 3100			
					12	35	TLL 2100 2100			
BMG18	2000X1100	13	30	BLL	15	35	FROM BMG17 TUL	10	@200	DOUBLES
		8	15	4HEF	12	35	FROM BMG17 TLL			
					8	30	TUL 3000 3100			
					7	30	TLL 1400 1400			
BMG19	2000X1100	10	30	BLL	8	30	FROM BMG18 TUL	10	@200	DOUBLES
		8	15	4HEF	7	30	FROM BMG18 TLL			
					9	35	TUL 3100 4300			
					8	30	TLL 1400 1400			
BMG20	2000X1100	10	35	BLL	9	35	FROM BMG19 TUL	10	25@150 LE	TRIPLES
		10	30	BUL	8	30	FROM BMG19 TLL	10	@200 REM	TRIPLES
		8	15	4HEF	10	35	TUL FROM BMG21			
					6	30	TLL FROM BMG21			
BMG21	2000X1100	6	25	BLL	10	35	4200 TUL 6850	10	@200	DOUBLES
		8	15	4HEF	6	30	2000 TLL 2800			
BMG22	2000X1500	18	35	BLL	10	35	FROM BMG21 TUL	15	28@125 RE	TRIPLES
		10	30	BUL	6	30	FROM BMG21 TLL	15	20@125 LE	TRIPLES
		8	15	4HEF	10	35	TUL 4800	15	@200 REM	TRIPLES
					4	30	TLL 2700			
BMG23	2000X1500	20	35	BLL	7	35	4400 TUL	15	20@125 RE	DOUBLES
		17	35	BUL	5	30	2500 TLL	15	8@200 LE	DOUBLES
		10	15	5HEF	12	35	TUL 6400 5000	15	@300 REM	DOUBLES
					4	30	TLL 2800 2800			
BMG24 (DELETED)	2000X900	9	25	BLL	12	35	FROM BMG23	15	5@200 RE	DOUBLES
		6	15	3HEF				15	5@200 LE	DOUBLES
								15	@300 REM	DOUBLES
BMG25	2000X1500	12	35	BLL	9	30	5000 TUL	10	15@200 RE	TRIPLES
		12	15	6HEF	12	35	TUL 4800 7250	10	10@200 LE	TRIPLES
					10	30	TLL 3000 3000	10	@300 REM	TRIPLES
BMG26	2000X1500	14	35	BLL	12	35	FROM BMG25 TUL	10	30@150 RE	TRIPLES
		10	30	BUL	10	30	FROM BMG25 TLL	10	7@200 LE	TRIPLES
		10	15	5HEF	12	35	TUL 4800 4800	10	@300 REM	TRIPLES
					10	30	TLL 2300 2300			
BMG27	2000X1100	13	35	BLL	12	35	FROM BMG26 TUL	10	7@200 RE	TRIPLES
		8	15	4HEF	10	30	FROM BMG26 TLL	10	7@200 LE	TRIPLES
					8	35	TUL 4800 4800	10	@300 REM	TRIPLES
					8	30	TLL 2300 2300			
BMG28	2000X1100	15	35	BLL	8	35	FROM BMG27 TUL	10	40@100 RE	TRIPLES
		8	15	4HEF	8	30	FROM BMG27 TLL	10	7@200 LE	TRIPLES
					8	35	TUL 4800 4800	10	@300 REM	TRIPLES
					8	30	TLL 2300 2300			
BMG29	2000X1100	13	30	BLL	8	35	FROM BMG28 TUL	10	@200	TRIPLES
		8	15	4HEF	8	30	FROM BMG28 TLL			
					10	35	TUL 4800 5800			
					10	30	TLL 2400 2400			
BMG30	2000X1100	17	35	BLL	10	35	FROM BMG29 TUL	10	7@200 RE	TRIPLES
		6	35	BUL	10	30	FROM BMG29 TLL	10	18@200 LE	TRIPLES
		8	15	4HEF	8	35	TUL 4200	10	@300 REM	TRIPLES
BMG31	2000X800	18	35	BLL	6	35	TUL	15	@125	FOUR SETS
		16	30	BUL	3	30	TUL			
		4	15	2HEF						



LEVEL 1 FRAMING PLAN WITH BOTTOM REINFORCEMENT
1 : 100

REFER TO S1-06 FOR PLAN NOTES:



D1 S1-07 PLAN DETAIL
1 : 25

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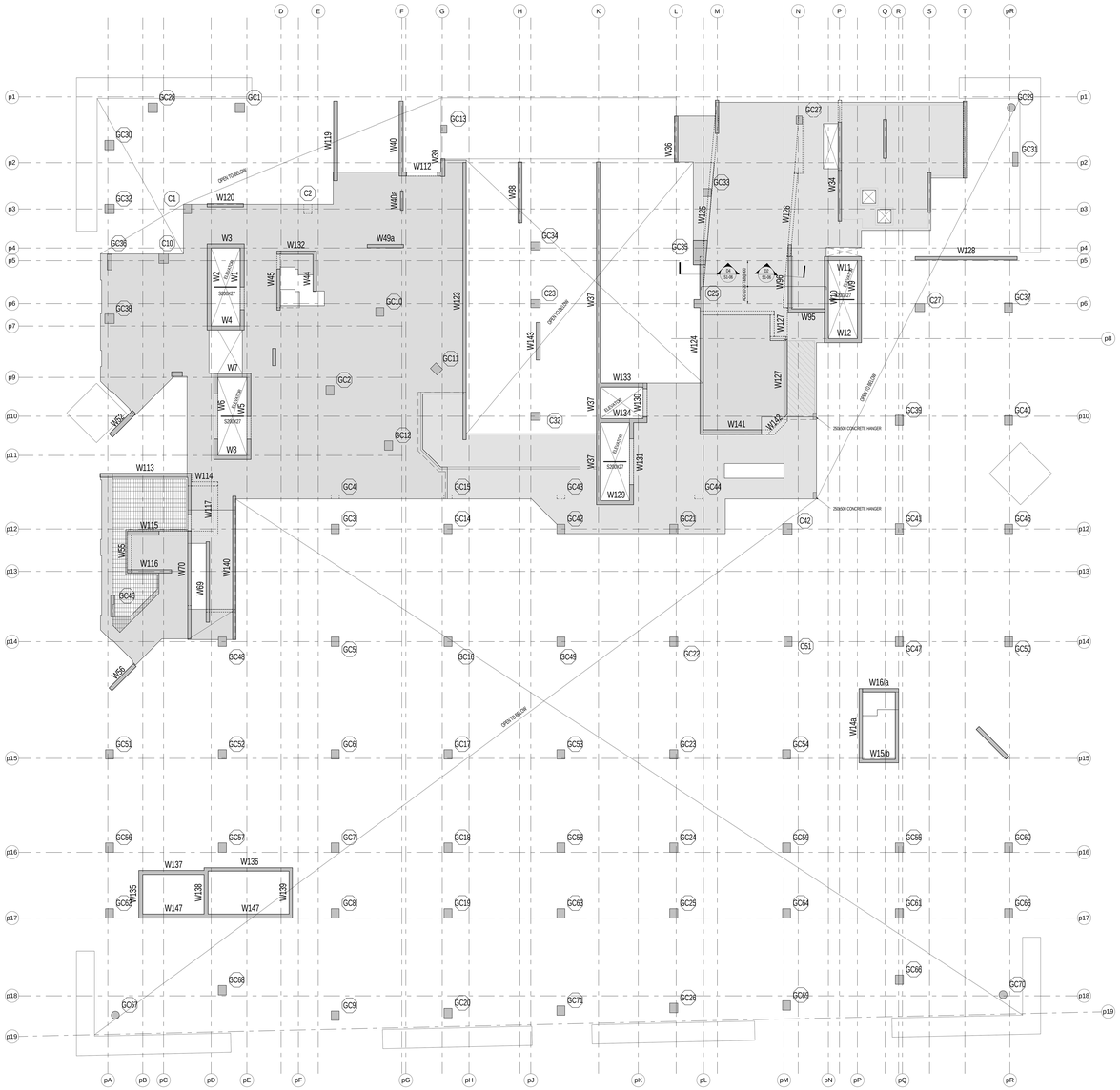
1	ISSUED FOR WT REVIEW	May 01,2015

AQUAVISTA AT BAYSIDE TORONTO

Drawing Title
LEVEL 1-GROUND FLOOR FRAMING PLAN WITH BOTTOM REINFORCEMENT AND SCHEDULES

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	Author
Checked by	Checker
Project No.	20140031
Date	JUNE 12,2015
Drawing No.	S1-07



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1	ISSUED FOR MT REVIEW	May 01,2015

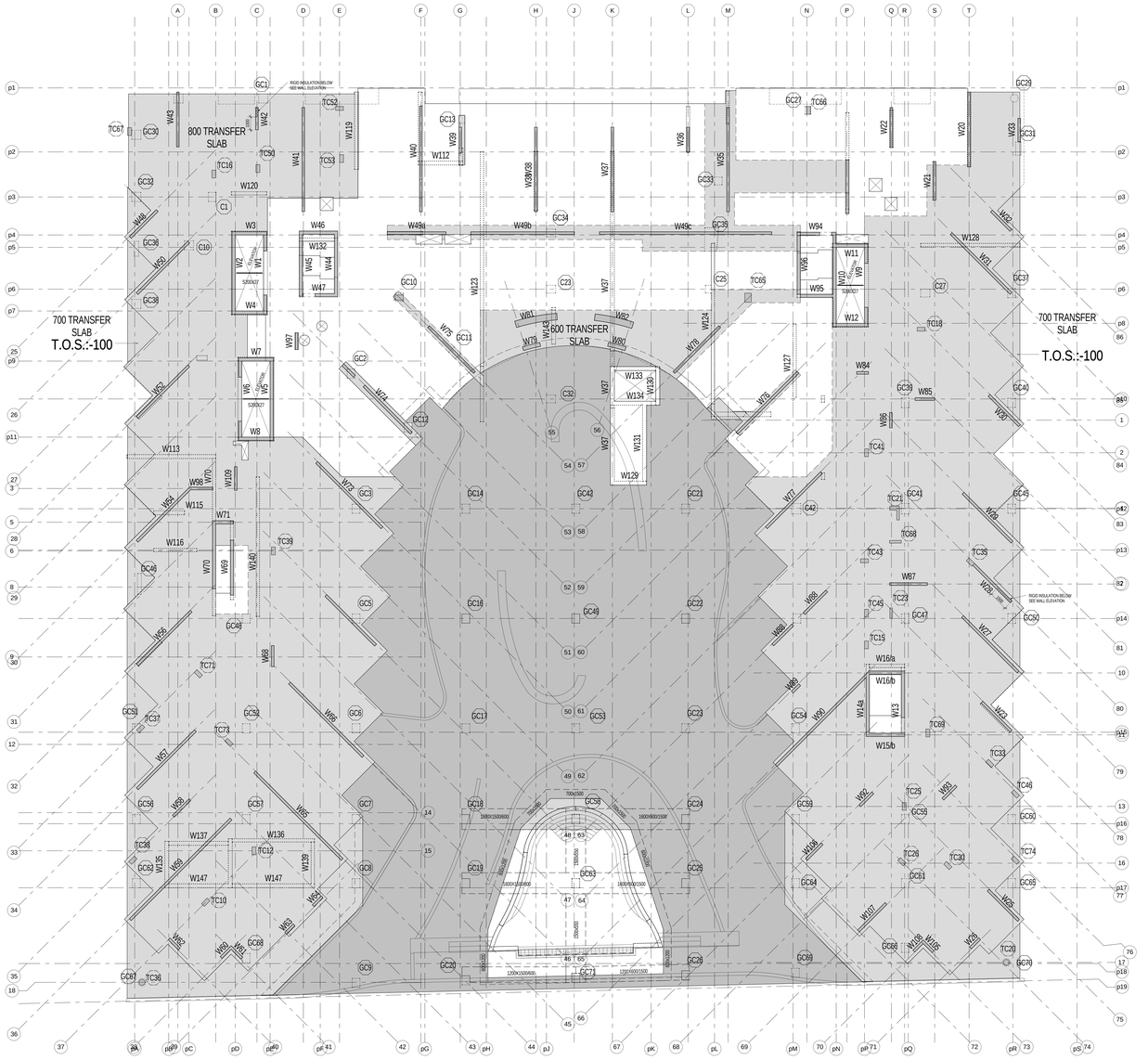
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL L1 MEZZANINE FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale: As indicated
Drawn by: Author
Checked by: Checker
Project No: 2014/0031
Date: JUNE 12,2015
Drawing No: S1-08

Plot Date: 6/11/2015 12:03:55 PM



LEVEL 2 FRAMING PLAN

1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.2 mm BELOW FINISHED FLOOR DATUM ELEVATION EXCEPT AS CROSSED AND NOTED. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATION.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
SUITES	1.9 kPa*	1.25 kPa
CORRIDOR	4.8 kPa*	2.5 kPa
STAIRS/BALCONIES/HALL/LOBBY	4.8 kPa*	2.5 kPa
CANOPY	2.4 kPa*	0.5 kPa

* INCLUDES ALLOWANCE FOR SLOPES AND ROOFING MATERIALS
** 4% ALLOWANCE FOR SNOW LOAD OF 1.0 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SNOWLOAD

3. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm.
4. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
5. THE CONCRETE SUPPLY AGREEMENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
6. SEE TYPICAL DETAIL DRAWING FOR LINTELS IN NON-LOAD BEARING BLOCK WALLS.
7. SEE ALSO TYPICAL NOTES AND DETAILS.
8. REFER TO BEAM SCHEDULES.
9. REFER TO ARCH. DRAWINGS FOR SLAB/STAIR REINFORCEMENT.
10. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.
11. IN TRANSFER SLABS, HOOK BOTTOM BARS AND EXTEND ALL TOP BARS INTO ADJACENT SLABS BY 2000mm (TYP)
12. RC ON PLAN DENOTES - ARCHITECTURAL COLUMN, REFER TO S2-02 FOR DETAILS
13. TRANSFER SLABS TO BE SHORED UNTIL ROADY SPECIFIED 28 DAYS CONCRETE STRENGTH. SHORING SHOULD NOT BE SUPPORTED ON BUILDING SLABS.

TLL = TOP LAYER LL
TLL = TOP LAYER LL
BLL = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB:	250 MIN. t/h
SLAB:	200 MIN.
INTEGRITY BARS:	3-15M BEW
CONCRETE STRENGTH:	35 MPa
BOTTOM STEEL EW:	2@400 t/h
TOP STEEL:	2@400 t/h
SLAB:	350 MIN.
INTEGRITY BARS:	3-15M BEW
CONCRETE STRENGTH:	35 MPa
TOP STEEL:	2@200 t/h
BOTTOM STEEL:	2@300 t/h
FACE STEEL:	3-15M HEF
SLAB:	500 MIN.
INTEGRITY BARS:	3-15M BEW
CONCRETE STRENGTH:	35 MPa
BOTTOM STEEL EW:	2@200 t/h
TOP STEEL:	2@200 t/h
SLAB:	600 MIN.
INTEGRITY BARS:	3-15M BEW
CONCRETE STRENGTH:	40 MPa
BOTTOM STEEL EW:	2@200 t/h
TOP STEEL:	2@200 t/h
SLAB:	1000 MIN.
INTEGRITY BARS:	3-15M BEW
CONCRETE STRENGTH:	40 MPa
BOTTOM STEEL EW:	2@200 t/h
TOP STEEL:	2@300 t/h

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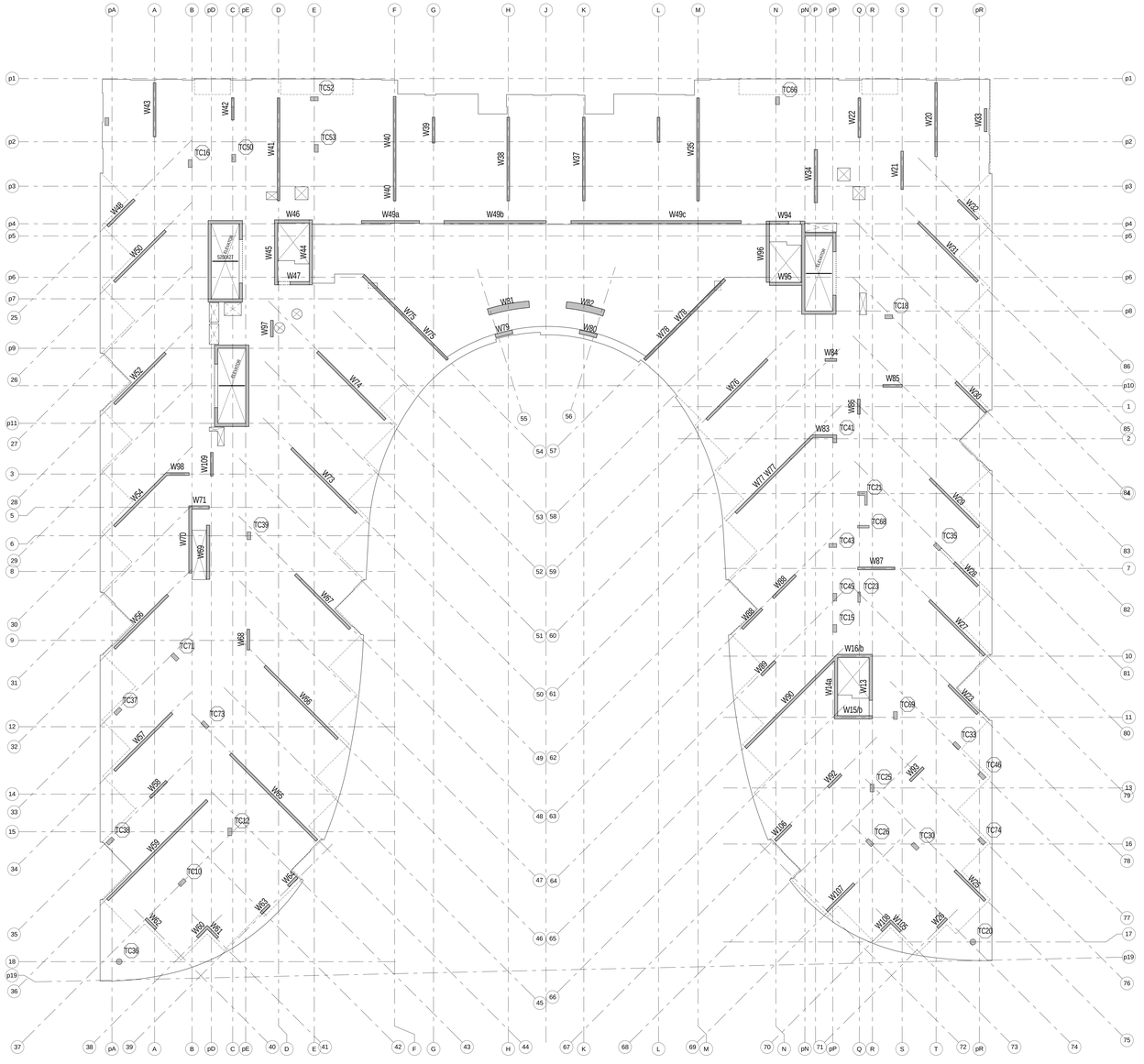
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL 2 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	DC
Checked by	BBS
Project No.	2014003I
Date	JUNE 12,2015
Drawing No.	S1-09



LEVEL 3 FRAMING PLAN

1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED FLOOR DATUM ELEVATION EXCEPT AS CROSSED AND NOTED. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATION.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
SUITES	1.9 kPa	1.25 kPa
CORRIDOR	4.8 kPa*	1.9 kPa
STAIRS/HALLS/LOBBY	4.8 kPa*	1.9 kPa
BALCONIES	4.8 kPa*	2.3 kPa
GREEN ROOF	4.8 kPa*	5.2 kPa

* INCLUDES ALLOWANCE FOR SLOPES AND ROOFING MATERIALS
** 4% ALLOWANCE FOR SNOW LOAD OF 1.35 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SNOWLOAD

3. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm.
4. NO OPENINGS TO BE DEFINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
5. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
6. SEE TYPICAL DETAIL DRAWING FOR LINTELS IN NON-LOAD BEARING BLOCK WALLS
7. SEE TYPICAL DETAIL DRAWINGS FOR DETAILS.
8. REFER TO BEAM SCHEDULES.
9. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.
10. AC ON PLAN DENOTES - ARCHITECTURAL COLUMN. REFER TO S2-02 FOR DETAILS
11. IN TRANSFER SLABS, NON BOTTOM BARS AND EXTEND ALL TOP BARS INTO ADJACENT SLABS BY 2000mm (TYP)

TUL = TOP UPPER LAYER
TLL = TOP LOWER LAYER
BUL = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	200 MIN. d/n 3-20M BEW 35 MPa 15@300
SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	250 MIN. 3-20M BEW 35 MPa 15@300
SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	300 MIN. 3-20M BEW 35 MPa 15@300
SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TOP STEEL: BOTTOM STEEL:	600 MIN. 3-25M BEW 40 MPa 25@300 25@300
SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL: FACE STEEL:	1000 MIN. 3-25M BEW 45MPa 25@250 4-15M HEF

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1	ISSUED FOR MT REVIEW	May 01,2015

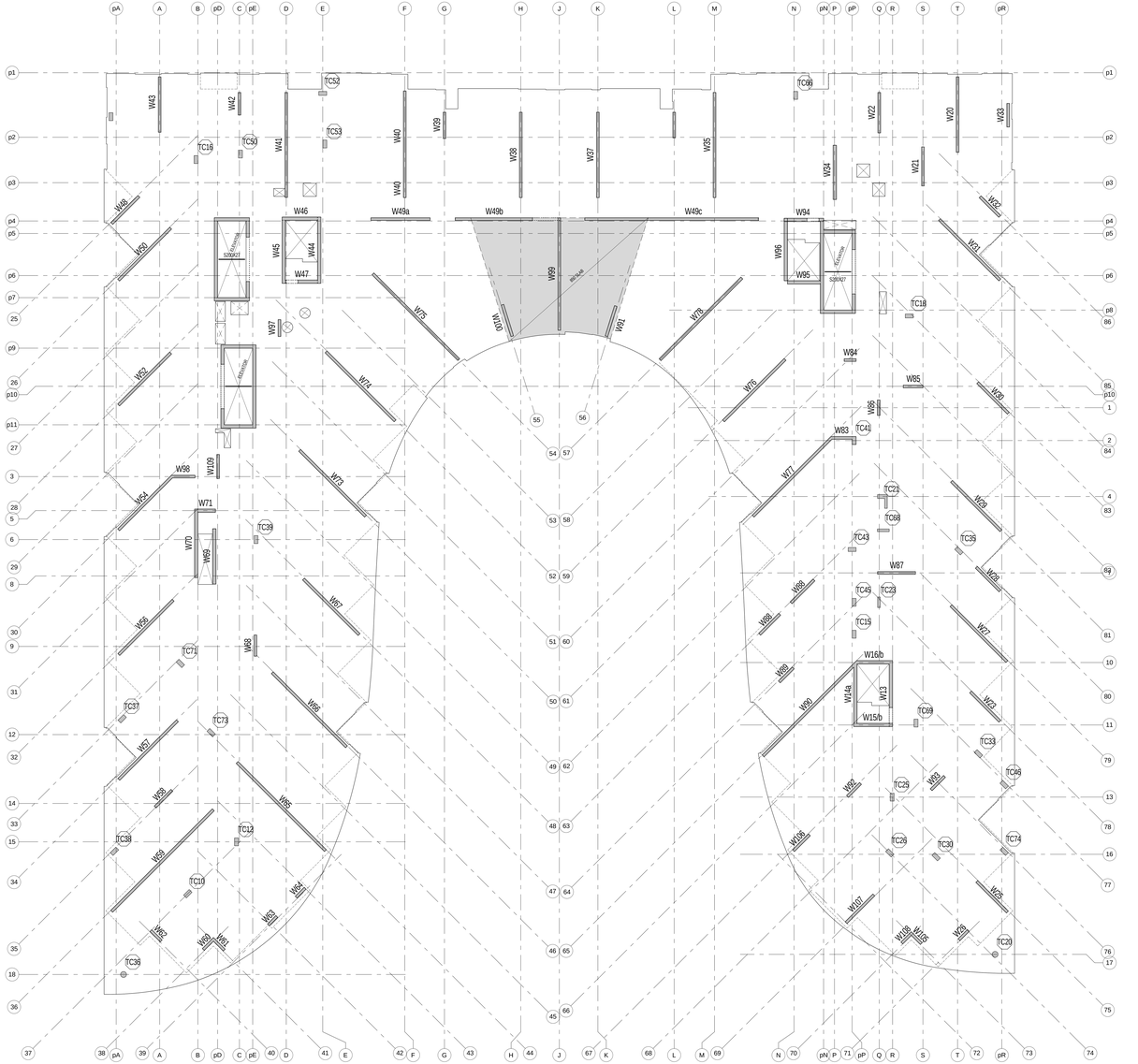
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL 3 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	DC
Checked by	BK/BBS
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S1-10

Plot Date: 6/12/2015 12:04:21 PM



LEVEL 4 FRAMING PLAN

1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED FLOOR DATUM ELEVATION EXCEPT AS CROSSED AND NOTED. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATIONS.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
SUITES	1.9 kPa	1.0 kPa
TERRACE	4.8 kPa**	3.8 kPa*
ETHERSHALL LOBBY	4.8 kPa**	0.5 kPa
BALCONIES	4.8 kPa**	0.5 kPa
GREEN ROOF	4.8 kPa**	8.2 kPa*

* INCLUDES ALLOWANCE FOR SLOPES AND ROOFING MATERIALS
** AN ALLOWANCE FOR SNOW LOAD OF 1.78 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SNOW LOAD

3. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm.
4. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
5. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
6. REFER TO MASONRY DRAWINGS FOR LINTELS IN NON-LOAD BEARING BLOCK WALLS.
7. SEE A-0.0 TYPICAL NOTES AND DETAILS.
8. REFER TO BEAM SCHEDULES.
9. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND AROUND OPENINGS.
10. SEE A-0.0 TYPICAL SLAB ARCHITECTURAL COLUMN, REFER TO S2-02 FOR DETAILS
11. IN TRANSFER SLABS, HOOK BOTTOM BARS AND EXTEND ALL TOP BARS INTO ADJACENT SLABS BY 2000mm (TYP)
12. TRANSFER SLABS TO BE SHORED UNTIL REACH SPECIFIED 28 DAYS CONCRETE STRENGTH. SHORING SHOULD NOT BE SUPPORTED ON BUILDING SLABS.

TLX = TOP UPPER LAYER
TLL = TOP LOWER LAYER
BLX = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	200 MIN. ph 3.25M BEW 35 MPa 15@300
SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	250 MIN. 3.25M BEW 35 MPa 15@400
SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL:	300 MIN. 3.25M BEW 35 MPa 15@300
SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TOP STEEL: BOTTOM STEEL:	600 MIN. 3.25M BEW 45 MPa 25@300 25@250
SLAB: INTEGRITY BARS: CONCRETE STRENGTH: TEMP. STEEL: FACE STEEL:	200 MIN. 3.25M BEW 40MPa 25@300 4-25M HEF

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No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

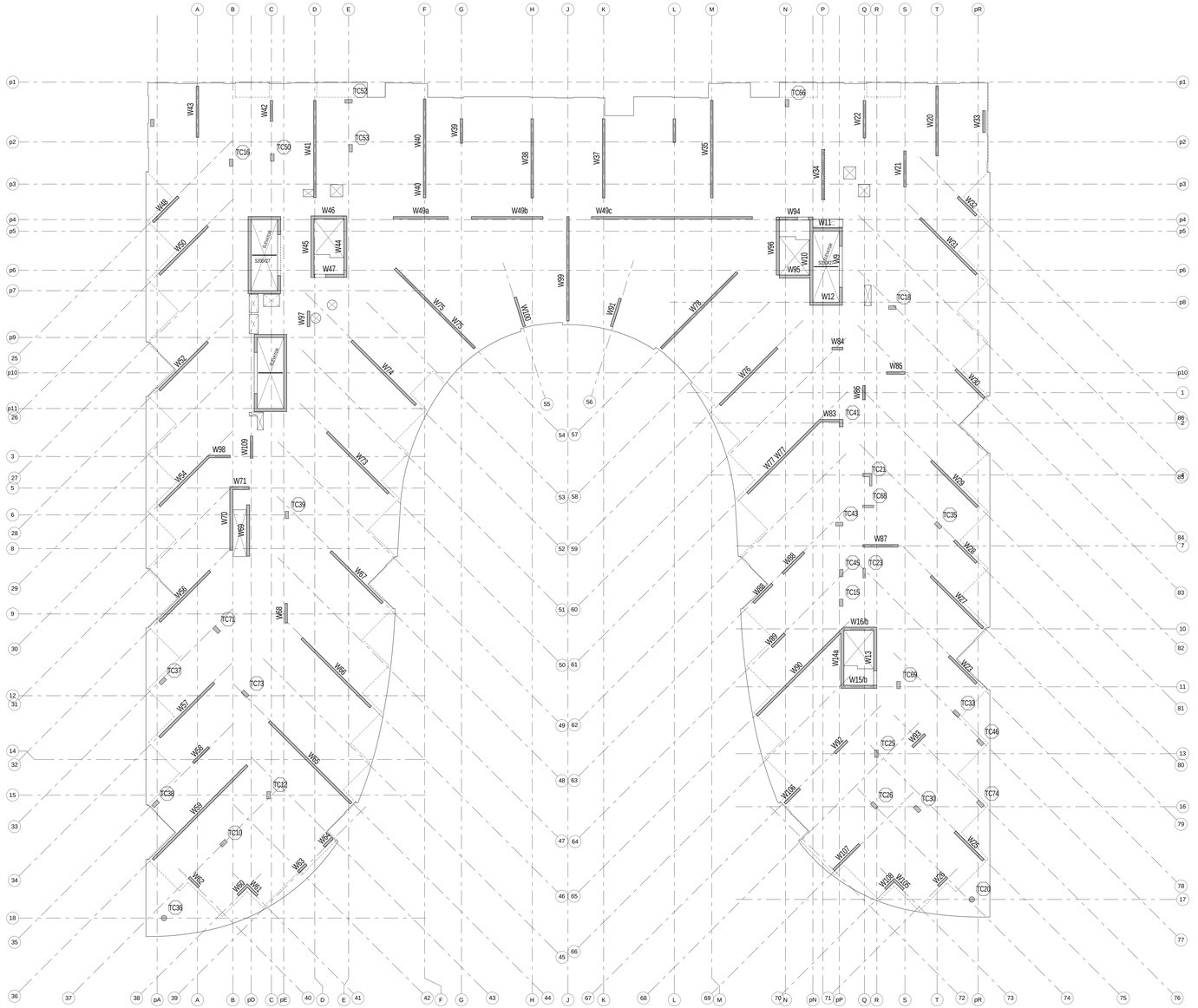
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL 4 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	DC
Checked by	BK/BBS
Project No.	2014003S
Date	JUNE 12,2015
Drawing No.	S1-11

Plot Date: 6/11/2015 12:04:14 PM



LEVEL 5 FRAMING PLAN

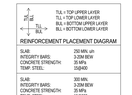
1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED FLOOR DATUM ELEVATION EXCEPT AS CROSSED AND NOTED. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATIONS.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT

	LL	SDL
SUITES	1.9 kPa	1.25 kPa
TERRACE	4.8 kPa**	3.6 kPa*
STAIRS/HALL/LOBBY	4.8 kPa	0.5 kPa
BALCONY	4.8 kPa**	6.3 kPa*
GREEN ROOF	4.8 kPa**	9.4 kPa*

* INCLUDES ALLOWANCE FOR SLOPES AND ROOFING MATERIALS
** AN ALLOWANCE FOR SNOW LOAD OF 1.2 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SNOWLOAD.

3. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm.
4. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
5. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO ANY CONCRETE POUR TO ARRANGE FOR A REVIEW OF PREPARATIONS.
6. SEE TYPICAL DETAIL DRAWING FOR LINTELS IN NON-LOAD BEARING BLOCK WALLS.
7. SEE ALSO TYPICAL NOTES AND DETAILS.
8. REFER TO BEAM SCHEDULES.
9. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, EQUAL TO THE MAIN BOTTOM STEEL.
10. AT CIVIL PLAN BENDTES - ARCHITECTURAL COLUMN, REFER TO S2-02 FOR DETAILS
11. IN TRANSFER SLABS, HOOK BOTTOM BARS AND EXTEND ALL TOP BARS INTO ADJACENT SLABS BY 2000mm (TYP)



REINFORCEMENT PLACEMENT DIAGRAM

SLAB:	250 MIN. sh
INTGERY BARS:	1-20M @ 300
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	10@400
SLAB:	300 MIN.
INTGERY BARS:	1-20M @ 300
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	10@300
SLAB:	450 MIN.
INTGERY BARS:	1-20M @ 300
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	10@200
SLAB:	550 MIN.
INTGERY BARS:	1-20M @ 300
CONCRETE STRENGTH:	35MPa
TEMP. STEEL:	20@250
SLAB:	650 MIN.
INTGERY BARS:	1-25M @ 300
CONCRETE STRENGTH:	35MPa
TEMP. STEEL:	25@250
SLAB:	850 and 900 MIN.
INTGERY BARS:	1-25M @ 300
CONCRETE STRENGTH:	35MPa
TEMP. STEEL:	25@250
FACE STEEL:	3-15M HEF

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

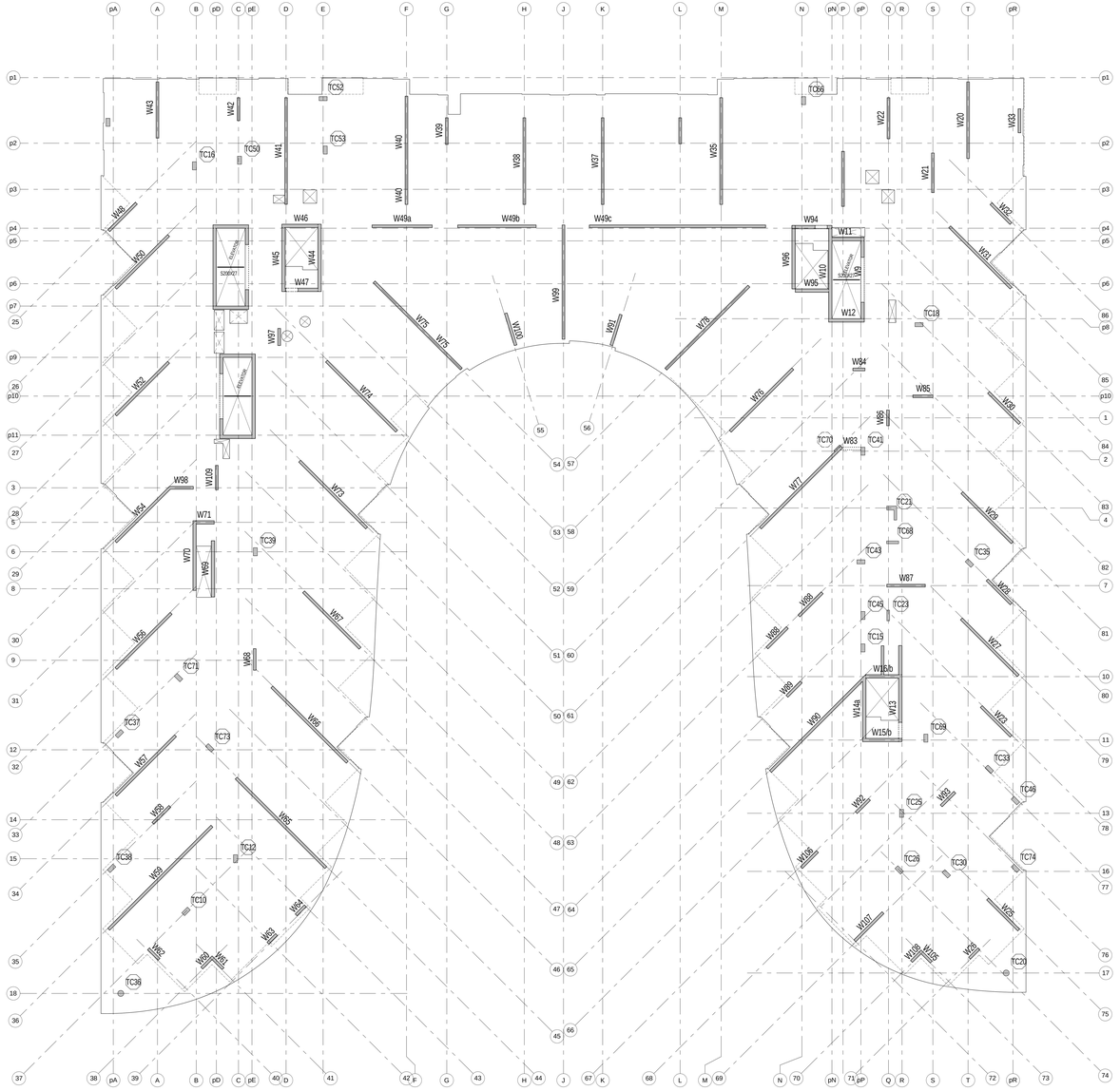
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL 5 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	DC
Checked by	BBS
Project No.	2014003I
Date	JUNE 12,2015
Drawing No.	S1-12

Plot Date: 6/11/2015 12:04:18 PM



LEVEL 6 FRAMING PLAN

1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED FLOOR DATUM ELEVATION EXCEPT AS CROSSED AND NOTED. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATION.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
SUITES	1.9 kPa	1.25 kPa
STAIRS/HALLLOBBY	4.8 kPa	0.5 kPa
BALCONIES	4.8 kPa**	0.5 kPa

** AN ALLOWANCE FOR SNOW LOAD OF 1.12 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SNOW LOAD

3. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm.
4. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
5. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
6. SEE TYPICAL DETAIL DRAWING FOR LINTELS IN NON-LOAD BEARING BLOCK WALLS.
7. SEE ALSO TYPICAL NOTES AND DETAILS.
8. REFER TO BEAM SCHEDULES.
9. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.

TLL = TOP UPPER LAYER	
TLL = TOP LOWER LAYER	
BLL = BOTTOM UPPER LAYER	
BLL = BOTTOM LOWER LAYER	

REINFORCEMENT PLACEMENT DIAGRAM

SLAB	200 MIN. uh
INTEGRITY BARS	3-20M BEW
CONCRETE STRENGTH:	25 MPa
TEMP. STEEL:	15@500

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR WT REVIEW	May 01,2015

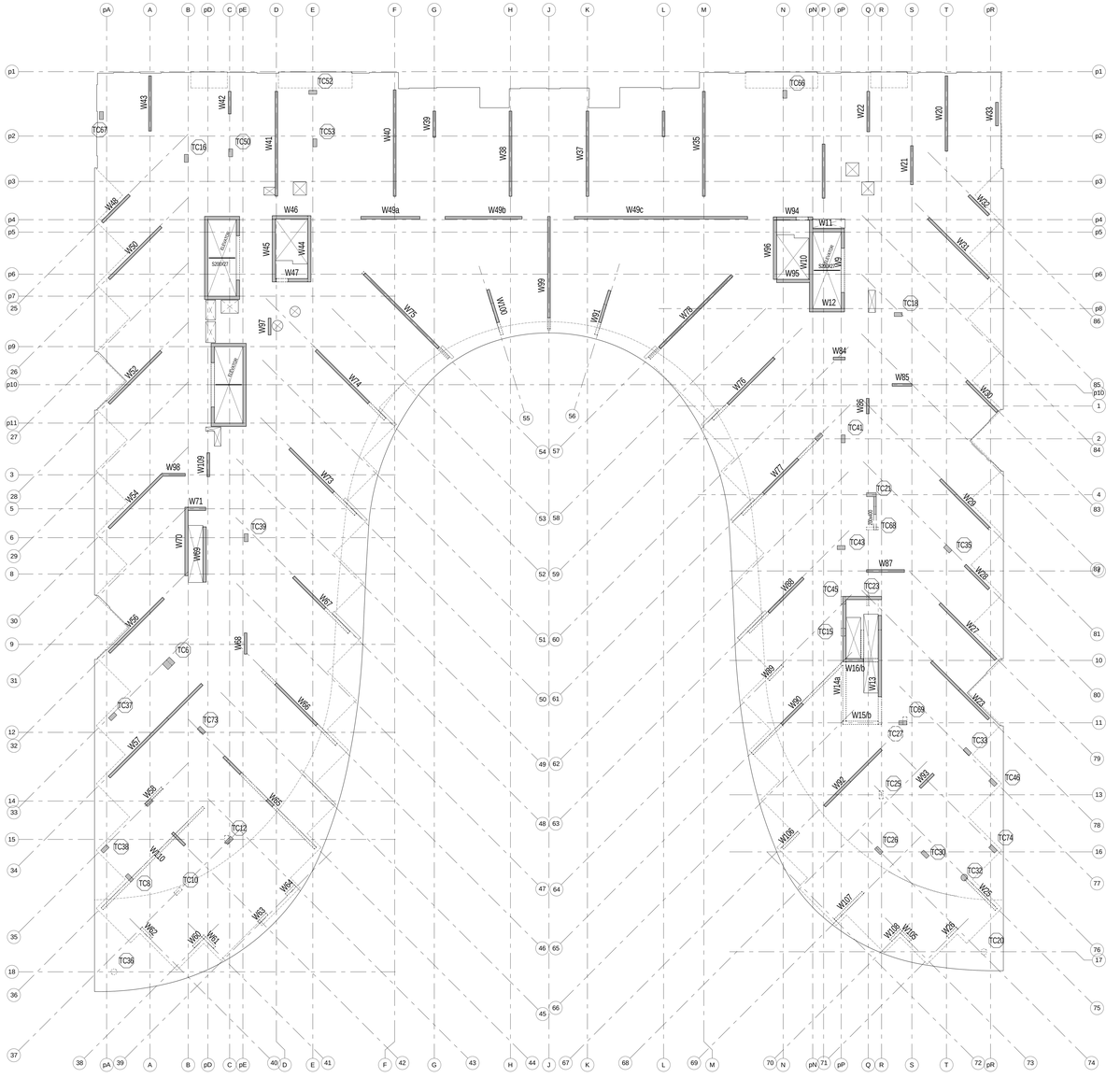
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL 6 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale: As indicated
Drawn by: DC
Checked by: BK/BBS
Project No.: 20140031
Date: JUNE 12,2015
Drawing No.: S1-13

Plot Date: 6/12/2015 12:04:31 PM



LEVEL 7 FRAMING PLAN

1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED FLOOR DATUM ELEVATION EXCEPT AS CROSSED AND NOTED. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATION.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
SLOTS	1.9 kPa	1.25 kPa
TERRACE	4.8 kPa*	3.6 kPa*
STAIRS/HALL/LOBBY	4.8 kPa	5.1 kPa
BALCONIES	4.8 kPa*	5.1 kPa
CANOPY	2.4 kPa*	5.1 kPa
GREEN ROOF	4.8 kPa*	6.2kPa*

* INCLUDES ALLOWANCE FOR SLOPES AND ROOFING MATERIALS
** AN ALLOWANCE FOR SNOW LOAD OF 1.12 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SNOWLOAD

3. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm.
4. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
5. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
6. SEE TYPICAL DETAIL DRAWING FOR LINTELS IN NON-LOAD BEARING BLOCK WALLS.
7. SEE ALSO TYPICAL NOTES AND DETAILS.
8. REFER TO BEAM SCHEDULES.
9. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.
10. AC ON PLAN DENOTES - ARCHITECTURAL COLUMN. REFER TO S2-02 FOR DETAILS

TLL = TOP UPPER LAYER
TLL = TOP LOWER LAYER
BLL = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB	250 MIN. uR
INTEGRITY BARS	3-20M BEW
CONCRETE STRENGTH	35 MPa
TEMP. STEEL	15@300
SLAB	450 MIN.
INTEGRITY BARS	3-20M BEW
CONCRETE STRENGTH	35 MPa
TEMP. STEEL	15@200

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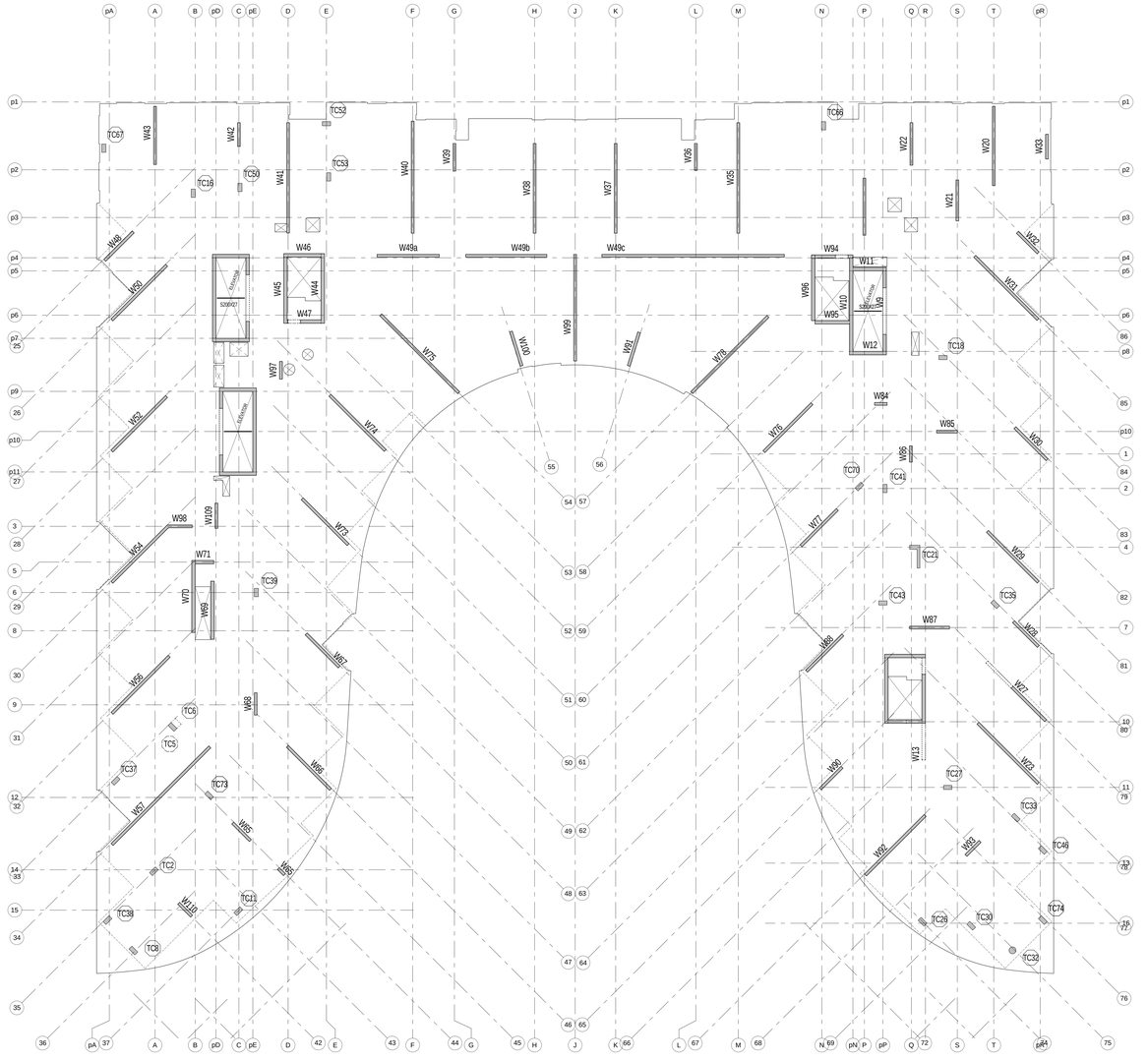
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL 7 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale: As indicated
Drawn by: DC
Checked by: BK/BBS
Project No.: 20140031
Date: JUNE 12,2015
Drawing No.: S1-14

Plot Date: 6/12/2015 12:04:34 PM



LEVEL 8 FRAMING PLAN

1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED FLOOR DATUM ELEVATION EXCEPT AS CROSSED AND NOTED. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATIONS.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
SUITES	1.9 kPa	1.25 kPa
STAIRS/BALCONIES/HALLS/CRRT	4.8 kPa	2.5 kPa

** AN ALLOWANCE FOR SNOW LOAD OF 1.12 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SNOW LOAD

3. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm.
4. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
5. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO POURING CONCRETE FOR ARRANGEMENTS FOR A REVIEW OF REINFORCING.
6. SEE TYPICAL DETAIL DRAWING FOR PARTELS IN NON-LOAD BEARING BLOCK WALLS.
7. SEE ALSO TYPICAL NOTES AND DETAILS.
8. REFER TO BEAM SCHEDULES.
9. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.

TLL = TOP UPPER LAYER
TLL = TOP LOWER LAYER
BLL = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM	
SLAB INTEGRITY BARS CONCRETE STRENGTH TEMP. STEEL	200 MIN. c/c 3-20M BEW 25 MPa 2@400
SLAB INTEGRITY BARS CONCRETE STRENGTH TEMP. STEEL	250 MIN. 3-20M BEW 25 MPa 2@400

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1	ISSUED FOR MT REVIEW	May 01,2015

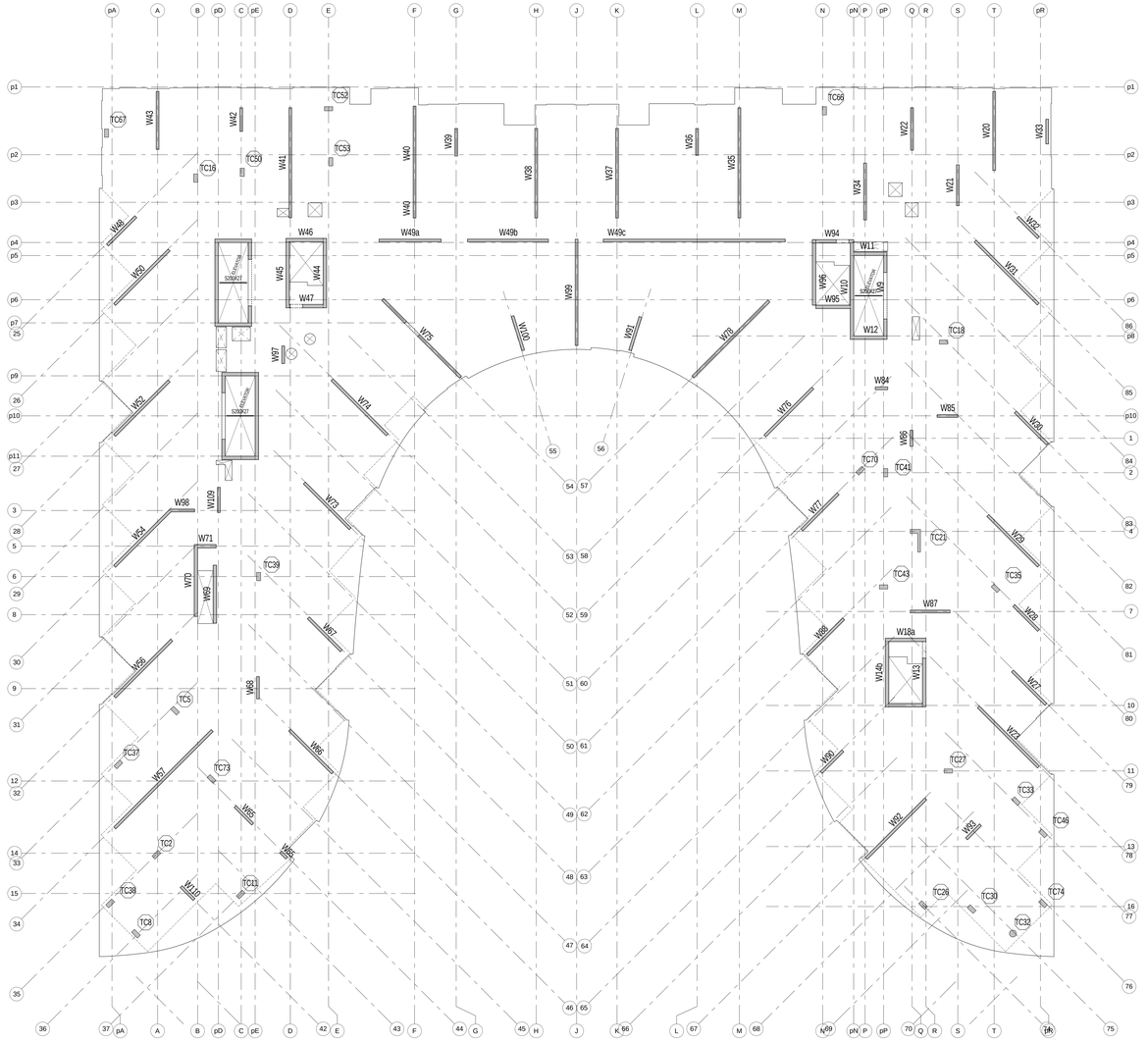
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL 8 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	DC
Checked by	BK/BBS
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S1-15

Plot Date: 6/11/2015 12:04:37 PM



LEVEL 9 FRAMING PLAN

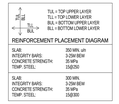
1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED FLOOR DATUM ELEVATION EXCEPT AS CROSSED AND NOTED. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATION.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT:

	LL	SDL
ROOF AREA	1.9 kPa*	1.5 kPa*
AMENITY AREA	4.8 kPa*	1.5 kPa*
GREEN ROOF	2.4 kPa*	1.5 kPa*
STAIR/SHALLOBBY	4.8 kPa	0.5 kPa
PLANTERS	4.8 kPa*	8.2 kPa*
DRIVE AISLE	2.4 kPa	0.5 kPa
DRIVE RAMP	2.4 kPa	0.5 kPa
TERRACE	4.8 kPa	5.0 kPa
POOL	14.4 kPa	7.2 kPa

* INCLUDES ALLOWANCE FOR SLOPES AND ROOFING MATERIALS
* INCLUDES ALLOWANCE FOR PONDING. 12 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SNOW LOAD.

3. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm.
4. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
5. THE CONCRETE SUBCONTRACTOR MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STEEL/CONCRETE, CONCRETE FOR A REVIEW OF PREPARATIONS.
6. SEE TYPICAL DETAIL DRAWING FOR LINTELS IN NON-LOAD BEARING BLOCK WALLS.
7. SEE ALSO TYPICAL NOTES AND DETAILS.
8. REFER TO BEAM SCHEDULES.
9. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.
10. AC ON PLAN DENOTES - ARCHITECTURAL COLUMN. REFER TO S2-02 FOR DETAILS



REINFORCEMENT PLACEMENT DIAGRAM

SLAB	300 MIN. c/c
INTEGRITY BARS:	3-20M BEW
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	15@250
SLAB	300 MIN.
INTEGRITY BARS:	3-20M BEW
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	15@300

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

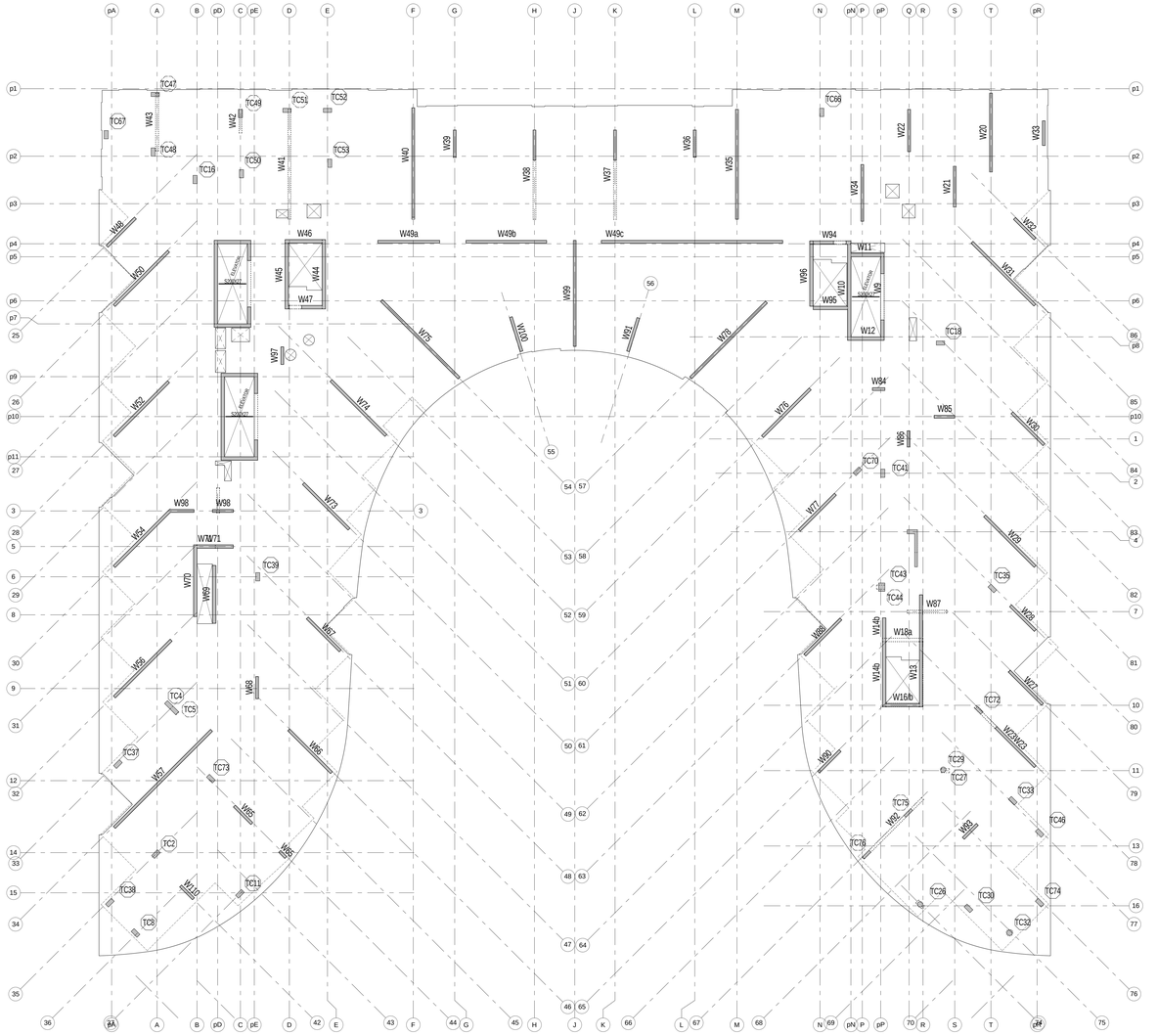
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL 9 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	DC
Checked by	BK/BBS
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S1-16

Plot Date: 6/12/2015 12:04:42 PM



LEVEL 10 FRAMING PLAN

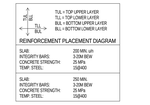
1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED FLOOR DATUM ELEVATION EXCEPT AS CROSSED AND NOTED. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATION.
2. THE FLOOR STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
SUITES	1.9 kPa	1.25 kPa
STAIRS/BALCONIES/HALLS/ OBBY	4.8 kPa	0.5 kPa

** AN ALLOWANCE FOR SNOW LOAD OF 1.12 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SNOW LOAD

3. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm.
4. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
5. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
6. SEE PLAN FOR LOCATIONS OF CONCRETE WALLS IN NON-LOAD BEARING BLOCK WALLS.
7. SEE ALSO TYPICAL NOTES AND DETAILS.
8. REFER TO BEAM SCHEDULES.
9. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.



TLL = TOP UPPER LAYER
TLL = TOP LOWER LAYER
BUL = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB:	200 MIN. d/t
TOP BOTH BARS:	2-15M BARS
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	3fy400
SLAB:	250 MIN.
TOP BOTH BARS:	2-15M BARS
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	3fy400

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

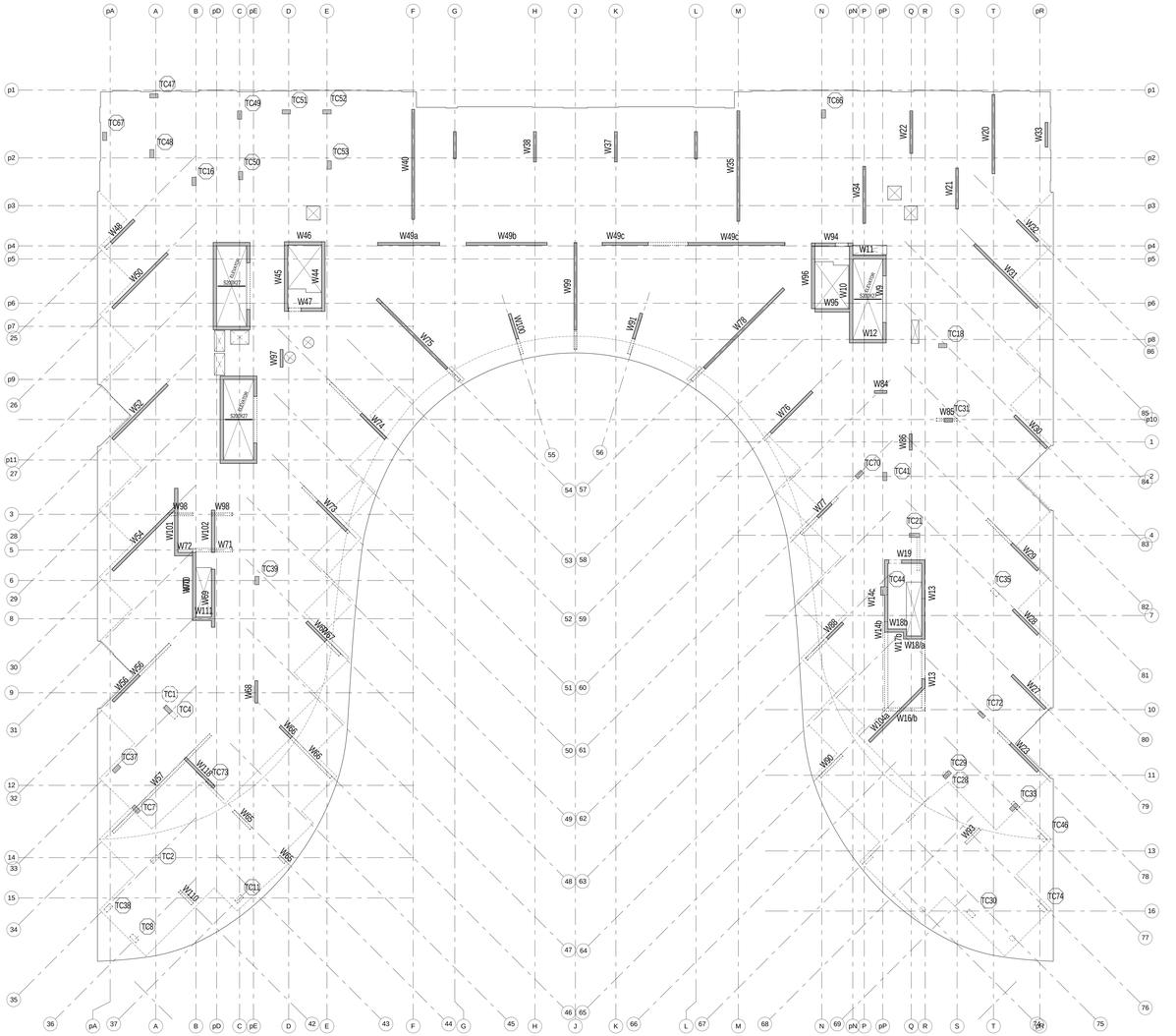
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL 10 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	DC
Checked by	BK/BBS
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S1-17

Plot Date: 6/12/2015 12:04:44 PM



LEVEL 11 FRAMING PLAN

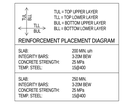
1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED FLOOR DATUM ELEVATION EXCEPT AS CROSSED AND NOTED. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATION.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF-WEIGHT.

	LL	SDL
SUITES	1.9 kPa	1.25 kPa
STAIRS/BALCONIES/HALLS OBBT	4.8 kPa	0.5 kPa

** AN ALLOWANCE FOR SNOW LOAD OF 1.12 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SNOWLOAD

3. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm.
4. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
5. THE CONCRETE SUBCONTRACTANTIDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
6. SEE TYPICAL DETAIL DRAWING FOR LINTELS IN NON-LOAD BEARING BLOCK WALLS.
7. SEE ALSO TYPICAL NOTES AND DETAILS.
8. REFER TO BEAM SCHEDULES.
9. WHERE SLAB OPENINGS ARE SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.



REINFORCEMENT PLACEMENT DIAGRAM

SLAB	200 MIN. o/c
INTEGRITY BARS	3-15M BOM
CONCRETE STRENGTH:	25 MPa
TEMP. STEEL	15M@300
SLAB	250 MIN.
INTEGRITY BARS	3-15M BOM
CONCRETE STRENGTH:	25 MPa
TEMP. STEEL	15M@300

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

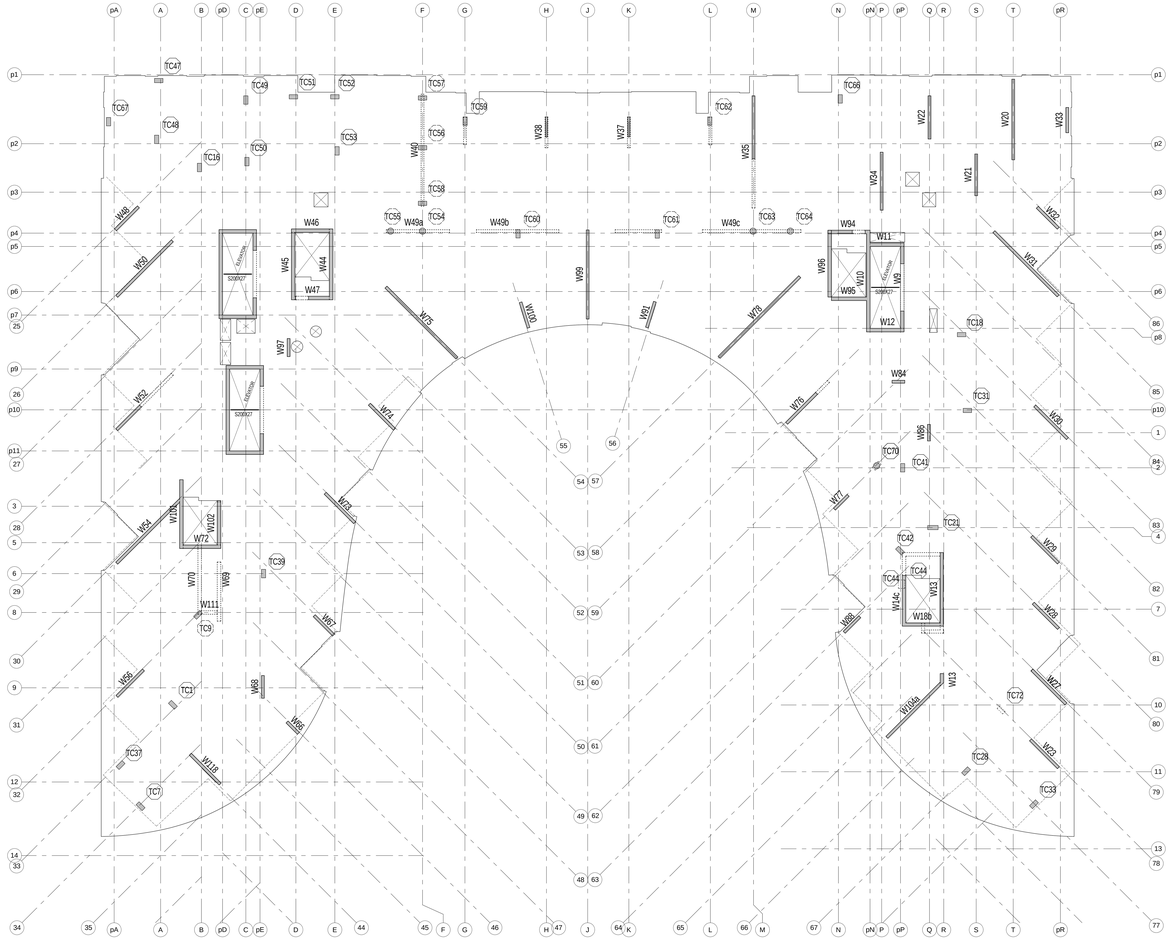
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL 11 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale: As indicated
Drawn by: Author
Checked by: Checker
Project No: 2014003I
Date: JUNE 12,2015
Drawing No: S1-18

Plot Date: 6/12/2015 12:04:47 PM



LEVEL 12 FRAMING PLAN

1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED FLOOR DATUM ELEVATION EXCEPT AS CROSSED AND NOTED. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATION.
2. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
SUITES	1.9 kPa	1.25 kPa
STAIRS/BALCONIES/HALLS/ORBIT	4.8 kPa	5.5 kPa

** AN ALLOWANCE FOR SHOW LOAD OF 1.13 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SHOWLOAD

3. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm.
4. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
5. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
6. SEE TYPICAL DETAIL DRAWING FOR LINTELS IN NON-LOAD BEARING BLOCK WALLS.
7. SEE ALSO TYPICAL NOTES AND DETAILS.
8. REFER TO BEAM SCHEDULES.
9. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.

TLL = TOP UPPER LAYER
TLL = TOP LOWER LAYER
BLL = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB:	200 kPa, u/n
INTEGRITY BARS:	3-20M BEW
CONCRETE STRENGTH:	25 kPa
TEMP. STEEL:	15@400
SLAB:	250 kPa
INTEGRITY BARS:	3-20M BEW
CONCRETE STRENGTH:	25 kPa
TEMP. STEEL:	15@400

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No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR WT REVIEW	May 01,2015

AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL 12 / PH1 FLOOR FRAMING PLAN

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale: As indicated
Drawn by: Author
Checked by: Checker
Project No.: 20140031
Date: JUNE 12,2015
Drawing No.: S1-19

Plot Date: 6/12/2015 12:04:50 PM

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LICENSED PROFESSIONAL ENGINEER JUNE 12,2015 B. BARADARAN-SEYED PROVINCE OF ONTARIO

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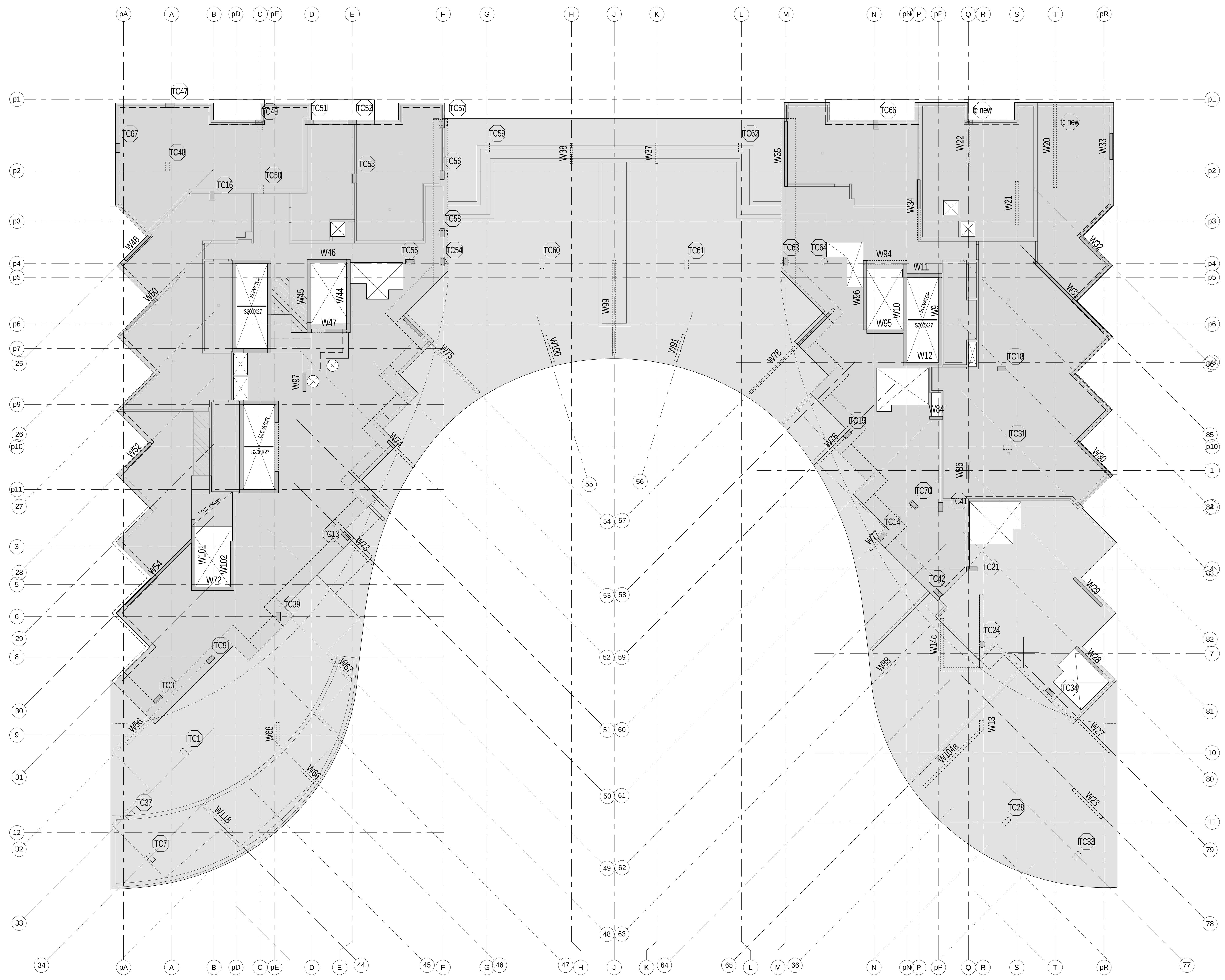


LEVEL 13 FRAMING PLAN

1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED FLOOR DATUM ELEVATION WITHIN THE MECHANICAL ROOM AND AT SLAB LOW POINTS. TOP OF SLAB IS SLOPED TO DRAINS AS SHOWN ON THE ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATION.
2. TOP OF STRUCTURAL SLAB TO BE 0.0 mm BELOW FINISHED FLOOR DATUM ELEVATION WITHIN THE MECHANICAL ROOM AND AT SLAB HIGH POINTS. TOP OF SLAB IS SLOPED TO DRAINS AS SHOWN ON THE ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATION.
3. ROOF DRAINAGE SLOPES ARE TO BE ACHIEVED BY SLOPING THE TOP OF THE SLAB. NOTE THAT THE SOFFIT OF SLAB IS FLAT.
4. THE STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) SUPERIMPOSED DEAD LOADS (SDL) AND SNOW LOADS (S) IN ADDITION TO THE SELF WEIGHT.

	LL	SDL
INTERIOR MECHANICAL PENTHOUSE	6.0 kPa	3.6 kPa
STAIRS/HALL/LOBBY	4.8 kPa	0.5 kPa
ROOF AREA	1.5 kPa**	2.4 kPa**
AMENITY AREA	4.8kPa**	1.5 kPa*
GREEN ROOF	4.8kPa**	8.2kPa*
PLANTERS	4.8kPa**	8.2kPa*
MECHANICAL	6.0 kPa	3.6 kPa

* INCLUDES ALLOWANCE FOR SLOPES AND ROOFING MATERIALS
** AN ALLOWANCE FOR SNOW LOAD OF 1.12 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SNOWLOAD

5. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 25MPa. REFER ALSO TO WALL SCHEDULE.
6. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm.
7. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
8. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
9. SEE TYPICAL DETAIL DRAWING FOR LINTELS IN NON-LOAD BEARING BLOCK WALLS.
10. SEE COLUMN AND SHEAR WALL SCHEDULES.
11. SEE ALSO TYPICAL NOTES AND DETAILS.
12. FOR CURB LOCATIONS DETAILS AND EXTENT, SEE ARCHITECTURAL AND MECHANICAL DRAWINGS. REINFORCE AS PER TYPICAL DETAIL.
13. SEE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR LOCATION, EXTENT AND SIZE OF HOUSEKEEPING PADS. REINFORCE PADS WITH WWF 152x152 MW13.3xMW13.3 PLUS 10M @ 460 ON CENTRE DOWELS x 200 LONG.
14. SEE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR LOCATION, EXTENT AND SIZE OF FLOATING FLOOR, REINFORCE SLABS WITH 10M@200 EW C/L EPOXY COATED.
15. REFER TO BEAM SCHEDULES.
16. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.

TUL BLL TLL BUL

TUL = TOP UPPER LAYER
TLL = TOP LOWER LAYER
BUL = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB:	200 MIN. u/n
INTEGRITY BARS:	3-20M BEW
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	15@400
SLAB:	250 MIN.
INTEGRITY BARS:	3-20M BEW
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	15@400
SLAB:	300 MIN.
INTEGRITY BARS:	3-20M BEW
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	15@300
SLAB:	350 MIN.
INTEGRITY BARS:	3-20M BEW
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	15@250

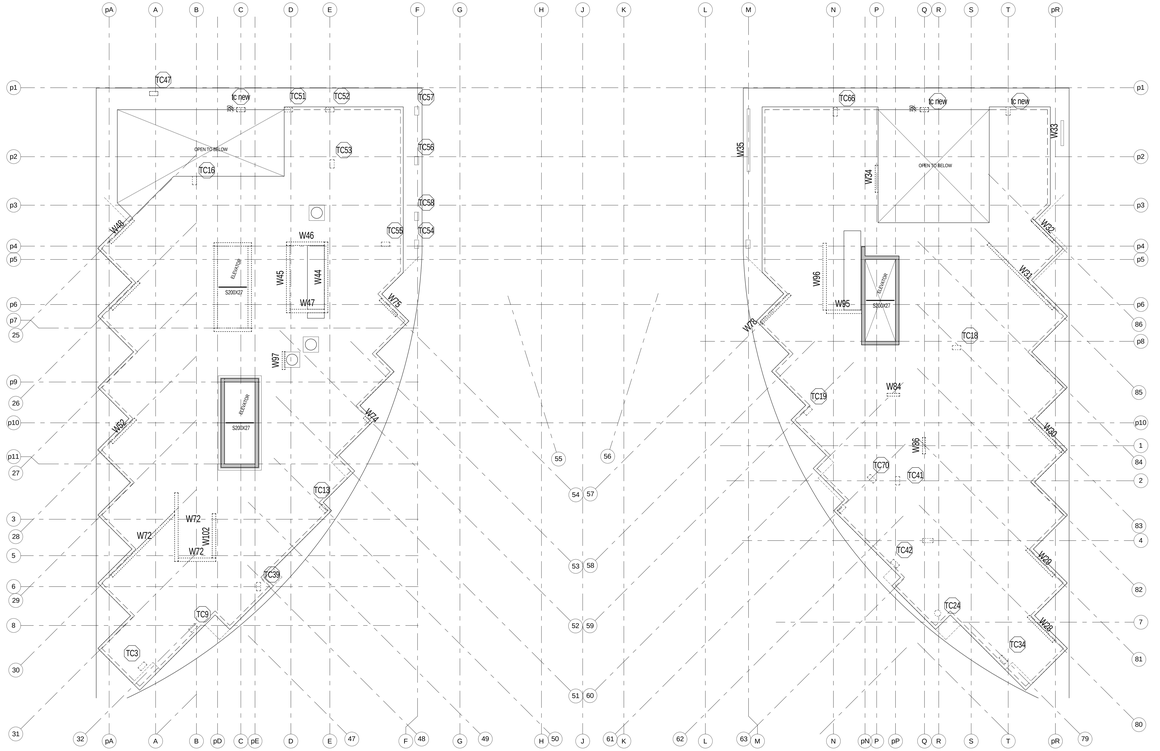
No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR WT REVIEW	May 01,2015

AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
LEVEL 13/PH2/ MECH. FLOOR FRAMING PLAN

Project
Hines TRIDEL®
AQUAVISTA
Block 3 - R3 & R4
Merchant's Whart
Toronto, ON

Scale: As indicated
Drawn by: Author
Checked by: Checker
Project No.: 20140031
Date: JUNE 12,2015
Drawing No.: S1-20



LEVEL 14 FRAMING PLAN

1 : 100

1. TOP OF STRUCTURAL SLAB TO BE 0.0 mm ABOVE ROOF DATUM ELEVATION EXCEPT AS CROSSED AND NOTED AND AS SLOPED TO DRAINS AS SHOWN BY THE ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL DRAWINGS FOR DATUM ELEVATION.
2. ROOF DRAINAGE SLOPES ARE TO BE ACHIEVED BY SLOPING THE TOP OF THE SLAB. NOTE THAT THE TOP OF SLAB IS FLAT.
3. THE ROOF STRUCTURAL SLAB HAS BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS (LL) AND SUPERIMPOSED DEAD LOADS (SDL), AND SNOW LOADS (S) IN ADDITION TO THE SELF WEIGHT:

	LL	SDL
ROOF AREA	1.2 kPa	1.0 kPa
TERRACE	4.8 kPa*	1.5 kPa

* INCLUDES ALLOWANCE FOR SLOPES AND ROOFING MATERIALS
** AN ALLOWANCE FOR SNOW LOAD OF 1.15 kPa HAS BEEN CONSIDERED. SEE PLAN FOR ADDITIONAL SNOW LOAD

4. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 25 MPa AT 28 DAYS. REFER ALSO TO COLUMN AND WALL ELEVATION DRAWINGS.
5. CONCRETE COVER FOR TOP AND BOTTOM BARS IN SLABS TO BE 25 mm. REINFORCING TO BE CHAIRED TO MAINTAIN PROPER COVER.
6. APPROVAL MUST BE OBTAINED FROM ENGINEER FOR ALL OPENINGS OTHER THAN THOSE SHOWN ON PLAN.
7. THE PROJECT SUPERINTENDENT MUST CONTACT THIS OFFICE 24 HOURS PRIOR TO PLACING STRUCTURAL CONCRETE FOR A REVIEW OF PREPARATIONS.
8. SEE ALSO TYPICAL NOTES AND DETAIL DRAWINGS.
9. REFER TO BEAM SCHEDULES.
10. WHERE BOTTOM STEEL NOT SHOWN, PROVIDE CONTINUOUS TEMPERATURE STEEL, HOOKED AT EDGE OF SLAB AND OPENINGS.

TLL = TOP LOWER LAYER
TLL = TOP LOWER LAYER
BUL = BOTTOM UPPER LAYER
BLL = BOTTOM LOWER LAYER

REINFORCEMENT PLACEMENT DIAGRAM

SLAB:	250 MIN. d/h
TOP BENDY BARS:	15@400EW
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	15@400EW o/h
SLAB:	200 MIN.
TOP BENDY BARS:	15@400EW
CONCRETE STRENGTH:	35 MPa
TEMP. STEEL:	15@400

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1	ISSUED FOR MT REVIEW	May 01,2015

AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
ROOF FRAMING PLANS

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale: As indicated
Drawn by: DC
Checked by: BK/BBS
Project No: 2014/0031
Date: JUNE 12,2015
Drawing No: S1-21

Plot Date: 6/11/2015 12:05:07 PM

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CONCRETE COLUMN SCHEDULE (LEVEL 2 AND BELOW)

CONCRETE COLUMN SCHEDULE (LEVEL 2 AND BELOW)

CONCRETE COLUMN SCHEDULE NOTES

1. COLUMN LOADS PROVIDED ARE ULS FACTORED VALUES AND INCLUDES LIVE LOAD REDUCTION FACTOR.
2. FOR LOCATION OF COLUMNS WITH RESPECT TO GRID LINES AND SLAB EDGES SEE ARCHITECTURAL DRAWINGS AND PLANS.
3. SLAB ELEVATIONS VARY. SEE ARCHITECTURAL DRAWINGS.
4. CONCRETE STRENGTH AS NOTED IN SCHEDULE.
5. PROVIDE DOWELS FROM COLUMN CAP INTO COLUMN TO MATCH VERTICALS EXCEPT AS NOTED IN SCHEDULE.
6. PROVIDE COMPRESSION LAP SPLICES (CLS) FOR COLUMNS. DOWEL LENGTHS WILL BE TWICE CLS. IN CASE COLUMNS ARE SPLICED AT MIDHEIGHT, PROVIDE TENSION LAP SPLICES (TLS)
7. LAP SPLICES FOR REINFORCING BARS GREATER THAN 35M ARE NOT PERMITTED. PROVIDE APPROVED MECHANICAL SPLICES.
8. WHERE COLUMN FACES ARE OFFSET 75mm OR MORE, SPLICES OF VERTICAL BARS ADJACENT TO THE OFFSET FACE SHALL BE MADE BY SEPARATE DOWELS LAPPED AS SPECIFIED IN NOTE 6 ABOVE.
9. WHERE ADDITIONAL REINFORCING STEEL IS REQUIRED IN THE LIFT ABOVE, PROVIDE DOWELS AS PER NOTE #6 TO MATCH REINFORCING ABOVE.
10. PROVIDE BENDING MOMENT BARS AT TOP OF ALL COLUMNS AS FOLLOWS:

    PROVIDE 4-20 EXCEPT AS NOTED IN SCHEDULE THUS (8-25).
    SEE ALSO TYPICAL BENDING MOMENT BARS PLACEMENT DIAGRAM.

    20M BAR: 800 / 800   25M BAR: 1250 / 1250

11. PROVIDE 2-10M TIES AROUND COLUMN REINFORCING WITHIN DEPTH OF SLABS ON ALL PERIMETER COLUMNS.
12. NO OPENINGS OTHER THAN THOSE INDICATED ON PLAN OR ELEVATIONS ARE TO BE MADE IN COLUMNS WITHOUT APPROVAL OF ENGINEER. SUBMIT DRAWINGS INDICATING ADDITIONAL OPENINGS FOR APPROVAL.
13. WHERE MASONRY ABUTS OR FACES A COLUMN PROVIDE STANDARD DOVETAIL ANCHOR SLOTS AT EVERY 3RD BLOCK COURSE, FROM COLUMNS INTO MORTAR JOINTS.
14. SEE ALSO SECTIONS, PLANS, ELEVATIONS AND DETAILS.
15. IN CASE COLUMNS AND WALLS OVERLAP THEY SHOULD BE POURED MONOLITHICALLY, SEE T-C41.
16. REFER TO NOTES AND TYPICAL DETAILS.

* INDICATES STAGGER SPLICE EVERY OTHER BAR AT FLOOR LEVEL AND MID HEIGHT.

No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR WT REVIEW	May 01,2015

AQUAVISTA AT BAYSIDE TORONTO

Drawing Title
COLUMN SCHEDULES - LEVEL 2 AND BELOW

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Whart
Toronto, ON

Scale: As indicated
Drawn by: DC
Checked by: BK/BBS
Project No.: 20140031
Date: JUNE 12,2015
Drawing No.: S2-01

CONCRETE COLUMN SCHEDULE (LEVEL 2 ABOVE)

Each cell: SIZE / VERT. REINF. / TIES / STRENGTH

LEVEL	TC1	TC2	TC3	TC4	TC5	TC6	TC7	TC8	TC9	TC10	TC11	TC12	TC13
LEVEL 14 / 124050 / LEVEL 13			300x600 10-30M V. 10M@300 25 MPa						300x600 8-25M V. 10M@300 25 MPa				300x600 8-20M V. 10M@300 25 MPa
118950 / LEVEL 12	300x600 8-20M V. 10M@300 25 MPa						300x600 8-20M V. 10M@300 25 MPa		300x600 8-25M V. 10M@300 25 MPa				
115000 / LEVEL 11	300x600 8-20M V. 10M@300 25 MPa						300x600 8-20M V. 10M@300 25 MPa						
111750 / LEVEL 10		300x600 8-20M V. 10M@300 25 MPa		300x1150 12-20M V. 10M@300 25 MPa				300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa		
108500 / LEVEL 9		300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa		
105250 / LEVEL 8		300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa		
102300 / LEVEL 7		300x600 8-20M V. 10M@300 25 MPa				735x600 10-25M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa		
99350 / LEVEL 6										300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	
96100 / LEVEL 5										300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	
93150 / LEVEL 4										300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	
90200 / LEVEL 3										300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	
87250 / LEVEL 2										300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	
84000 COLUMN LOAD(KN)	400	900	100	750	1000	1200	400	1100	500	1100	1200	2100	300
COLUMN NUMBER	TC1	TC2	TC3	TC4	TC5	TC6	TC7	TC8	TC9	TC10	TC11	TC12	TC13

LEVEL	TC14	TC15	TC16	TC17	TC18	TC19	TC20	TC21	TC23	TC24	TC25	TC26	TC27
LEVEL 14 / 124050 / LEVEL 13	300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa		300x740 SEE SETAIL 10M@200 35 MPa		450Ø 8-20M V. 10M@300 25 MPa			
118950 / LEVEL 12			300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa			300x740 SEE SETAIL 10M@200 35 MPa					
115000 / LEVEL 11			300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa			300x740 SEE SETAIL 10M@200 35 MPa					
111750 / LEVEL 10			300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa			L-740x2700 SEE SETAIL 10M@200 35 MPa				400Ø 8-20M V. 10M@300 25 MPa	
108500 / LEVEL 9			300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa			L-740x1650 SEE SETAIL 10M@200 35 MPa				300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
105250 / LEVEL 8			300x600 8-20M V. 10M@300 25 MPa		300x600 8-25M V. 10M@300 35 MPa			L-740x1650 SEE SETAIL 10M@200 35 MPa				300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
102300 / LEVEL 7			300x600 8-20M V. 10M@300 25 MPa		300x600 8-25M V. 10M@300 35 MPa			L-740x1650 SEE SETAIL 10M@200 35 MPa				300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
99350 / LEVEL 6		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa		300x600 8-25M V. 10M@300 35 MPa		450Ø 8-20M V. 10M@300 25 MPa	L-740x1050 SEE SETAIL 10M@200 35 MPa	200x800 12-20M V. 10M@300 35 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	
96100 / LEVEL 5		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa		300x600 8-25M V. 10M@300 35 MPa		450Ø 8-20M V. 10M@300 25 MPa	L-740x1050 SEE SETAIL 10M@200 35 MPa	200x800 12-20M V. 10M@300 35 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	
93150 / LEVEL 4		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa		300x600 12-30M V. 10M@300 35 MPa		450Ø 8-20M V. 10M@300 25 MPa	L-740x1050 SEE SETAIL 10M@200 35 MPa	200x800 12-20M V. 10M@300 35 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	
90200 / LEVEL 3		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa		300x600 12-30M V. 10M@300 35 MPa		450Ø 8-20M V. 10M@300 25 MPa	L-740x1050 SEE SETAIL 10M@200 35 MPa	200x800 12-20M V. 10M@300 35 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	
87250 / LEVEL 2		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa	400x700 14-30M V. 10M@300 50 MPa	300x600 12-30M V. 10M@300 35 MPa		450Ø 8-20M V. 10M@300 25 MPa	L-740x1050 SEE SETAIL 10M@200 35 MPa	200x800 12-20M V. 10M@300 35 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	
84000 COLUMN LOAD(KN)	300	1500	2400	5400	4100	400	900	4800	1600	300	1100	2100	2100
COLUMN NUMBER	TC14	TC15	TC16	TC17	TC18	TC19	TC20	TC21	TC23	TC24	TC25	TC26	TC27

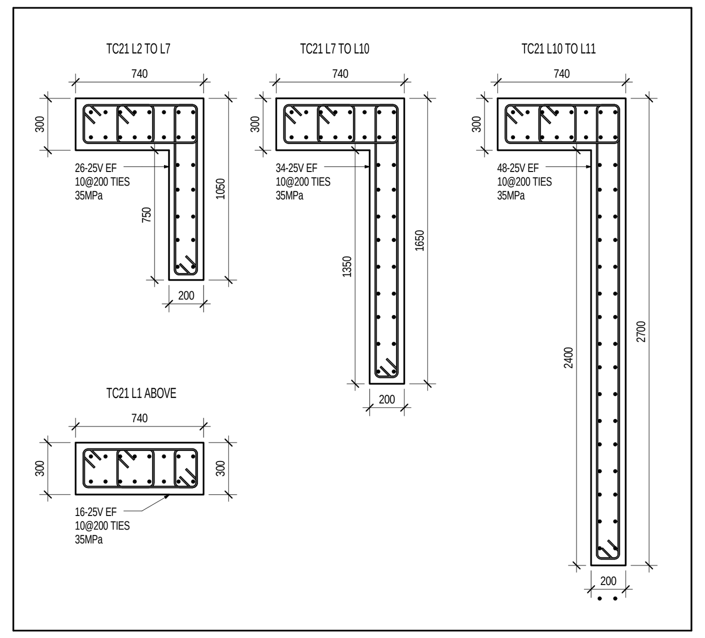
LEVEL	TC28	TC29	TC30	TC31	TC32	TC34	TC35	TC33	TC36	TC37	TC38	TC39	TC41
LEVEL 14 / 124050 / LEVEL 13						300x600 8-20M V. 10M@300 25 MPa						300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
118950 / LEVEL 12	300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa				300x600 10-30M V. 10M@300 35 MPa		300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
115000 / LEVEL 11	300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa				300x600 10-30M V. 10M@300 35 MPa		300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
111750 / LEVEL 10		400Ø 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa		400Ø 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 10-30M V. 10M@300 35 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 6-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
108500 / LEVEL 9			300x600 8-20M V. 10M@300 25 MPa		500Ø 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 10-30M V. 10M@300 35 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 6-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
105250 / LEVEL 8			300x600 8-20M V. 10M@300 25 MPa		500Ø 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 10-30M V. 10M@300 35 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 6-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
102300 / LEVEL 7			300x600 8-20M V. 10M@300 25 MPa		500Ø 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 10-30M V. 10M@300 35 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 6-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa
99350 / LEVEL 6			300x600 8-20M V. 10M@300 25 MPa				300x600 8-20M V. 10M@300 25 MPa	300x600 10-30M V. 10M@300 35 MPa	450Ø 12-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa	300x600 6-20M V. 10M@300 25 MPa	300x600 10-25M V. 10M@300 35 MPa	300x600 8-25M V. 10M@300 35 MPa
96100 / LEVEL 5			300x600 8-25M V. 10M@300 25 MPa				300x600 8-25M V. 10M@300 25 MPa	300x600 10-30M V. 10M@300 35 MPa	450Ø 12-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 10-25M V. 10M@300 35 MPa	300x600 8-25M V. 10M@300 35 MPa
93150 / LEVEL 4			300x600 8-25M V. 10M@300 25 MPa				300x600 8-25M V. 10M@300 25 MPa	300x600 10-30M V. 10M@300 35 MPa	450Ø 12-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 10-25M V. 10M@300 35 MPa	300x600 8-25M V. 10M@300 35 MPa
90200 / LEVEL 3			300x600 8-25M V. 10M@300 25 MPa				300x600 8-25M V. 10M@300 25 MPa	300x600 10-30M V. 10M@300 35 MPa	450Ø 12-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 10-25M V. 10M@300 35 MPa	300x600 8-25M V. 10M@300 35 MPa
87250 / LEVEL 2			300x600 8-25M V. 10M@300 25 MPa				300x600 8-25M V. 10M@300 25 MPa	300x600 10-30M V. 10M@300 35 MPa	450Ø 12-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 10-25M V. 10M@300 35 MPa	300x600 8-25M V. 10M@300 35 MPa
84000 COLUMN LOAD(KN)	1300	1600	2300	600	900	300	1000	2100	1300	2400	1500	3200	2100
COLUMN NUMBER	TC28	TC29	TC30	TC31	TC32	TC34	TC35	TC33	TC36	TC37	TC38	TC39	TC41

LEVEL	TC42	TC43	TC44	TC45	TC74	TC46	TC47	TC48	TC50	TC49	TC51	TC52	TC53
LEVEL 14 / 124050 / LEVEL 13	300x600 8-20M V. 10M@300 25 MPa						300x600 8-25M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
118950 / LEVEL 12	300x600 8-20M V. 10M@300 25 MPa						300x600 8-25M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
115000 / LEVEL 11			450x600 10-20M V. 10M@300 25 MPa				300x600 8-25M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
111750 / LEVEL 10			450x600 10-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
108500 / LEVEL 9		300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
105250 / LEVEL 8		300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa			300x600 8-25M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
102300 / LEVEL 7		300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa			300x600 8-25M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa
99350 / LEVEL 6		300x600 8-20M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa			300x600 8-25M V. 10M@300 25 MPa			300x600 8-30M V. 10M@300 35 MPa	300x600 8-25M V. 10M@300 35 MPa
96100 / LEVEL 5		300x600 8-25M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	300x600 10-25M V. 10M@300 35 MPa			300x600 8-25M V. 10M@300 25 MPa			300x600 8-30M V. 10M@300 35 MPa	300x600 8-25M V. 10M@300 35 MPa
93150 / LEVEL 4		300x600 8-25M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	300x600 10-25M V. 10M@300 35 MPa			300x600 8-25M V. 10M@300 25 MPa			300x600 8-30M V. 10M@300 35 MPa	300x600 8-25M V. 10M@300 35 MPa
90200 / LEVEL 3		300x600 8-25M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	300x600 10-25M V. 10M@300 35 MPa			300x600 8-25M V. 10M@300 25 MPa			300x600 8-30M V. 10M@300 35 MPa	300x600 8-25M V. 10M@300 35 MPa
87250 / LEVEL 2		300x600 8-25M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	300x600 10-25M V. 10M@300 35 MPa			300x600 8-25M V. 10M@300 25 MPa			300x600 8-30M V. 10M@300 35 MPa	300x600 8-25M V. 10M@300 35 MPa
84000 COLUMN LOAD(KN)	500	1600	300	1000	1800	2000	400	900	2050	500	400	2600	2800
COLUMN NUMBER	TC42	TC43	TC44	TC45	TC74	TC46	TC47	TC48	TC50	TC49	TC51	TC52	TC53

LEVEL	TC54	TC55	TC56	TC57	TC58	TC59	TC60	TC61	TC62	TC63	TC64	TC65	TC66
LEVEL 14 / 124050 / LEVEL 13	300x600 8-25M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa					300x600 8-20M V. 10M@300 25 MPa			300x600 10-30M V. 10M@300 35 MPa
118950 / LEVEL 12	450Ø 10-25M V. 10M@300 25 MPa	450Ø 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa	450Ø 8-20M V. 10M@300 25 MPa	450Ø 8-20M V. 10M@300 25 MPa		300x600 10-30M V. 10M@300 35 MPa
115000 / LEVEL 11													300x600 10-30M V. 10M@300 35 MPa
111750 / LEVEL 10													300x600 10-30M V. 10M@300 35 MPa
108500 / LEVEL 9													300x600 10-30M V. 10M@300 35 MPa
105250 / LEVEL 8													300x600 10-30M V. 10M@300 35 MPa
102300 / LEVEL 7													300x600 10-30M V. 10M@300 35 MPa
99350 / LEVEL 6													300x600 12-30M V. 10M@300 35 MPa
96100 / LEVEL 5													300x600 12-30M V. 10M@300 35 MPa
93150 / LEVEL 4													300x600 12-30M V. 10M@300 35 MPa
90200 / LEVEL 3													300x600 12-30M V. 10M@300 35 MPa
87250 / LEVEL 2												500x700 14-30M V. 10M@300 50MPa	300x600 12-30M V. 10M@300 35 MPa
84000 COLUMN LOAD(KN)	700	400	200	200	700	500	200	700	500	800	200	5100	4400
COLUMN NUMBER	TC54	TC55	TC56	TC57	TC58	TC59	TC60	TC61	TC62	TC63	TC64	TC65	TC66

LEVEL			TC67	TC68	TC69	TC70	TC71	TC72	TC73	TC75	TC76
LEVEL 14 / 124050 / LEVEL 13			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa					
118950 / LEVEL 12			300x600 8-20M V. 10M@300 25 MPa			450Ø 8-20M V. 10M@300 25 MPa					
115000 / LEVEL 11			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa		250x550 6-20M V. 10M@250 25 MPa			
111750 / LEVEL 10			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa		250x550 6-20M V. 10M@250 25 MPa	300x600 8-20M V. 10M@300 25 MPa	200x700 8-20M V. 10M@300 25 MPa	200x700 8-20M V. 10M@300 25 MPa
108500 / LEVEL 9			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa		
105250 / LEVEL 8			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa		
102300 / LEVEL 7			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa			300x600 8-20M V. 10M@300 25 MPa		
99350 / LEVEL 6			300x600 8-25M V. 10M@300 35 MPa	200x900 10-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa	300x600 8-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 25 MPa		300x600 8-20M V. 10M@300 25 MPa		
96100 / LEVEL 5			300x600 8-25M V. 10M@300 35 MPa	200x900 10-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa		300x600 8-25M V. 10M@300 25 MPa		300x600 8-25M V. 10M@300 25 MPa		
93150 / LEVEL 4			300x600 8-25M V. 10M@300 35 MPa	200x900 10-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa		300x600 8-25M V. 10M@300 25 MPa		300x600 8-25M V. 10M@300 25 MPa		
90200 / LEVEL 3			300x600 8-25M V. 10M@300 35 MPa	200x900 10-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa		300x600 8-25M V. 10M@300 25 MPa		300x600 8-25M V. 10M@300 25 MPa		
87250 / LEVEL 2			300x600 8-25M V. 10M@300 35 MPa	200x900 10-20M V. 10M@300 25 MPa	300x600 8-25M V. 10M@300 35 MPa		300x600 8-25M V. 10M@300 25 MPa		300x600 8-25M V. 10M@300 25 MPa		
84000 COLUMN LOAD(KN)			2600	1000	2600	1500	2300	300	2200	1600	1600
COLUMN NUMBER			TC67	TC68	TC69	TC70	TC71	TC72	TC73	TC75	TC76

SEE COLUMN NSCHEDULE NOTES ON S2-01



Contractor Must Check And Verify All Dimensions On The Job.

Do Not Scale The Drawings.

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Date:

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LICENSED PROFESSIONAL ENGINEER JUNE 12,2015 B. BARADARAN-SEYED PROVINCE OF ONTARIO

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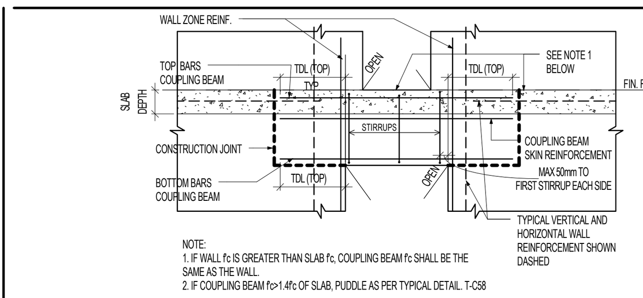
No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR WT REVIEW	May 01,2015

AQUAVISTA AT BAYSIDE TORONTO

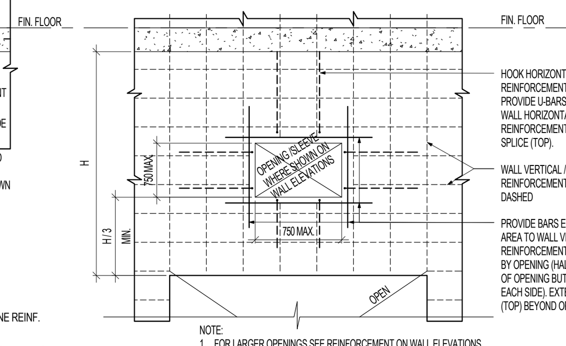
Drawing Title
COLUMN SCHEDULE - LEVEL 2 ABOVE AND CONNECTION PLATE SCHEDULE

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Whart
Toronto, ON

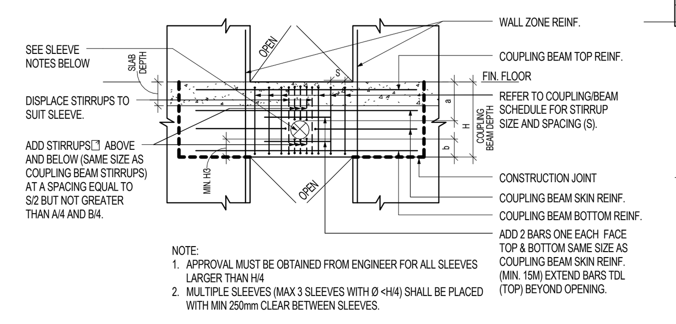
Scale: As indicated
Drawn by: DC
Checked by: BK/BBS
Project No.: 20140031
Date: JUNE 12,2015
Drawing No.: S2-02



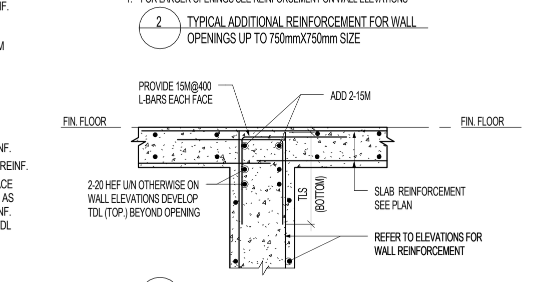
1 TYPICAL COUPLING BEAM REINFORCEMENT DETAIL



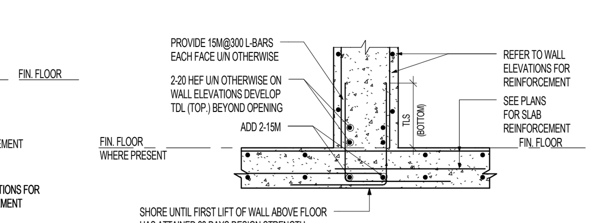
2 TYPICAL ADDITIONAL REINFORCEMENT FOR WALL OPENINGS UP TO 750mmX750mm SIZE



3 TYPICAL ADDITIONAL REINFORCEMENT COUPLING BEAM SLEEVES UP TO H/4



4 TYPICAL DETAIL AT TOP OF WALLS AND BELOW OPENINGS WHERE COUPLING BEAM IS NOT PROVIDED



5 TYPICAL DETAIL AT BOTTOM OF WALLS AND ABOVE OPENINGS WHERE COUPLING BEAM IS NOT PROVIDED

SHEAR WALL NOTES:

1. SHEAR WALL ELEVATIONS SERVE THE PURPOSE OF DEFINING THE ZONE, DISTRIBUTED AND COUPLING BEAM REINFORCEMENT, ALONG WITH THE SIZE OF THE SHEAR WALLS, COUPLING BEAMS AND CONCRETE STRENGTH. SHEAR WALL ELEVATIONS SHOULD BE READ IN CONJUNCTION WITH SCHEDULES AND TYPICAL SHEAR WALL DETAILS. THE GRAVITY ELEMENTS (COLUMNS, SLABS, BEAMS, FOUNDATION SYSTEM, ETC.) ARE SHOWN CONCEPTUALLY FOR CLARIFICATION ONLY. FOR SIZE, GEOMETRY, STEPS, ETC. OF THE GRAVITY ELEMENTS REFER TO THE APPROPRIATE PLANS, DETAILS AND SCHEDULES.
2. CONCRETE STRENGTHS TO BE AS NOTED IN WALL ELEVATIONS.
3. FOR ALL GARAGE SLABS, WALLS, AND COLUMNS, CONCRETE SHALL CONFORM TO THE LATEST EDITION OF CAN/CSA S413 AND MEET THE REQUIREMENTS FOR EXPOSURE CLASSIFICATION C-1 AS DEFINED IN TABLE 14 OF CSA STANDARD CAN/CSA-A23.1 LATEST EDITION. REFER TO THE SPECIFICATIONS.
4. PROVIDE WALL REINFORCEMENT IN ACCORDANCE WITH THE VALUES INDICATED ON THE WALL ELEVATIONS OR PLANS. WHERE 'NWR' IS INDICATED PROVIDE THE NOMINAL WALL REINFORCEMENT, AS SHOWN IN TABLE 1.

TABLE 1: NOMINAL WALL REINFORCEMENT (NWR)

WALL THICKNESS (mm)	NOMINAL WALL REINFORCEMENT	
	HORIZONTAL	VERTICAL
150	10@320H (CENTERED)	10@440V (CENTERED)
200	10@460 HEF	10@460 VEF
250	10@400 HEF	10@460 VEF
300	10@320 HEF	10@440 VEF
350	10@280 HEF	10@380 VEF
400	10@240 HEF	10@320 VEF

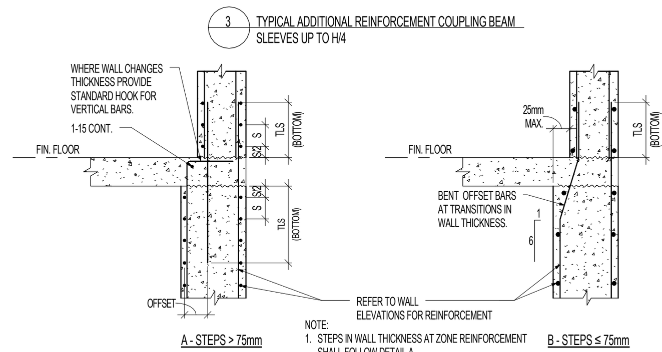
WALL THICKNESS (mm)	NOMINAL WALL REINFORCEMENT	
	HORIZONTAL	VERTICAL
450	15@440 HEF	10@280 VEF
500	15@400 HEF	15@460 VEF
600	15@320 HEF	15@440 VEF

5. WALL REINFORCEMENT IS VERTICAL AND HORIZONTAL EACH FACE (H&VEF) UNLESS NOTED OTHERWISE.
6. PROVIDE TIES FOR DISTRIBUTED VERTICAL REINFORCEMENT IF THE BAR SIZE IS LARGER THAN 20M OR IF BAR SPACING IS LESS THAN THAT OUTLINED IN TABLE 2. REFER TO DETAIL 7 FOR ADDITIONAL INFORMATION.
7. UNLESS OTHERWISE SHOWN PROVIDE DOWELS FROM SHEAR WALL, CAPS, OR FOOTINGS INTO SHEAR WALLS TO MATCH VERTICALS IN FIRST LIFT OF WALLS. SEE DETAIL 13.
8. UNLESS NOTED OTHERWISE AT TOPS OF ALL SHEAR WALLS PROVIDE 15 @ 400 DOWELS FROM WALL TO SLAB EACH FACE. WHERE WALL IS CONTINUOUS ABOVE SLAB SEE DETAIL 14.
9. PROVIDE TLS (BOTTOM) FOR ALL VERTICAL BARS (REFER TO DETAIL T-C56B (fy = 400MPa) OR T-C57B (fy = 500MPa) FOR LENGTHS)
10. PROVIDE TLS (TOP) FOR ALL HORIZONTAL BARS (REFER TO DETAIL T-C56B (fy = 400MPa) OR T-C57B (fy = 500MPa) FOR LENGTHS).
11. MINIMUM DOWEL LENGTH SHALL BE 2xTLS (BOTTOM).
12. PLACE HORIZONTAL REINFORCING ON OUTSIDE FACE OF WALL UNLESS NOTED OTHERWISE.
13. FOR WALLS THAT ARE UNBRACED FOR TWO STORIES OR MORE PROVIDE EITHER CONTINUOUS VERTICAL REINFORCEMENT FOR THE ENTIRE UNSUPPORTED HEIGHT OR USE MECHANICAL COUPLER AT THE LOCATION OF THE INTERMEDIATE SPLICE. PROVIDE TENSION LAP SPLICE (TOP) FOR ALL HORIZONTAL WALL REINFORCEMENT. IF BARS OF DIFFERENT DIAMETER ARE SPLICED, USE THE SPLICE LENGTH OF THE LARGER BAR.
14. UNLESS NOTED OTHERWISE, REFER TO THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR EXACT DIMENSIONS AND LOCATIONS OF WALL OPENINGS. THE CONTRACTOR SHALL PROVIDE, FOR THE ENGINEERS REVIEW, SLEEVING DRAWINGS SHOWING THE PROPOSED LOCATION AND INVERT (DIMENSIONED FROM THE SLAB DATUMS AND GRIDS) FOR ALL SLEEVES 75mm DIAMETER AND LARGER. NO OPENINGS OTHER THAN THOSE WHICH ARE INDICATED ON PLAN OR ELEVATION SHALL BE MADE WITHOUT THE APPROVAL OF THE ENGINEER.
15. WHERE MASONRY VENEER FACES A WALL, PROVIDE STANDARD DOVETAIL ANCHOR SLOTS, REFER TO TYPICAL DETAILS.
16. CLEAR CONCRETE COVER SHALL BE 25mm FOR WALLS ABOVE GRADE AND 40mm FOR WALLS BELOW GRADE UNLESS NOTED OTHERWISE.
17. PROVIDE 40mm CLEAR CONCRETE COVER FOR ALL ABOVE GRADE WALLS WHICH ARE EXPOSED TO FIRE ON BOTH SIDES SIMULTANEOUSLY AS DEFINED BY NBCC APPENDIX D.

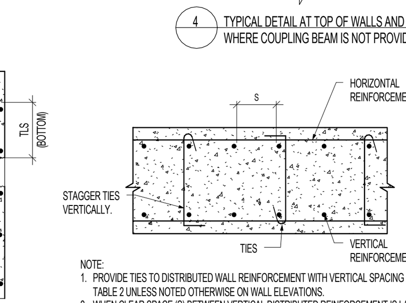
TABLE 2: TIES FOR DISTRIBUTED VERTICAL REINFORCEMENT

WALL THICKNESS (mm)	TIE SPACING FOR DISTRIBUTED VERTICAL REINFORCEMENT f'c ≤ 50MPa								
	10M		15M		20M		25M	30M	35M
	VERTICAL BAR SPACING (mm)	TIE VERTICAL SPACING (mm)	VERTICAL BAR SPACING (mm)	TIE VERTICAL SPACING (mm)	VERTICAL BAR SPACING (mm)	TIE VERTICAL SPACING (mm)	TIE VERTICAL SPACING (mm)	TIE VERTICAL SPACING (mm)	TIE VERTICAL SPACING (mm)
200	< 200	160	< 400	200	< 600	200	-	-	-
250	< 160	160	< 320	240	< 480	250	250	-	-
300	< 140	160	< 270	240	< 400	300	300	300	-
350	< 120	160	< 230	240	< 350	320	350	350	350
400	< 100	160	< 200	240	< 300	320	400	400	400
450	-	-	< 180	240	< 270	320	400	450	450
500	-	-	< 160	240	< 240	320	400	480	480
600	-	-	< 140	240	< 200	320	400	480	480

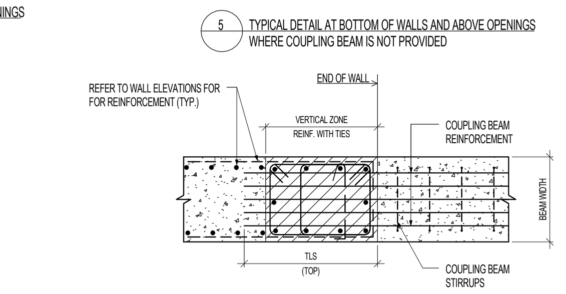
NOTES;
1. TIES FOR DISTRIBUTED VERTICAL REINFORCEMENT ARE 10M.
2. THIS TABLE IS BASED ON f'c ≤ 50MPa. FOR Fc' > 50MPa REDUCE TIE VERTICAL SPACING IN TABLE BY MULTIPLYING BY 0.75.
3. DISTRIBUTED VERTICAL REINFORCEMENT WITH BAR SPACING LESS THAN THAT INDICATED FOR 10M, 15M AND 20M BARS SHALL BE TIED WITH MINIMUM TIE VERTICAL SPACING AS INDICATED.
4. DISTRIBUTED VERTICAL REINFORCEMENT WITH BAR SIZE LARGER THAN 20M SHALL BE TIED AT MINIMUM TIE VERTICAL SPACING INDICATED.
5. SEE ALSO DETAIL 7.



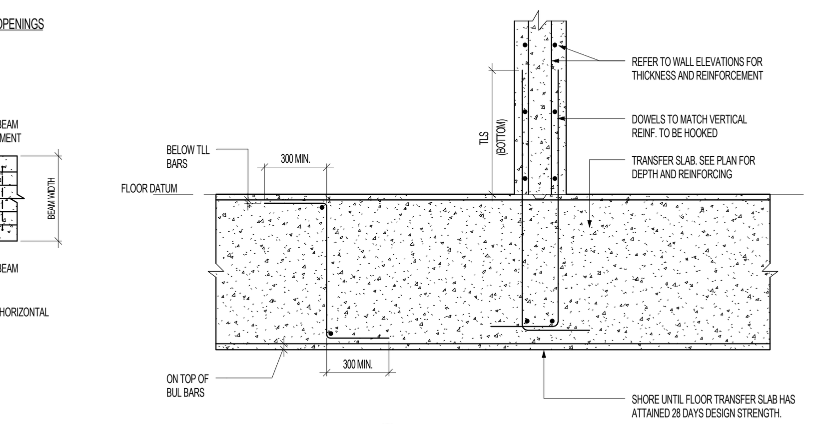
6 TYPICAL DETAIL AT WALL THICKNESS TRANSITIONS



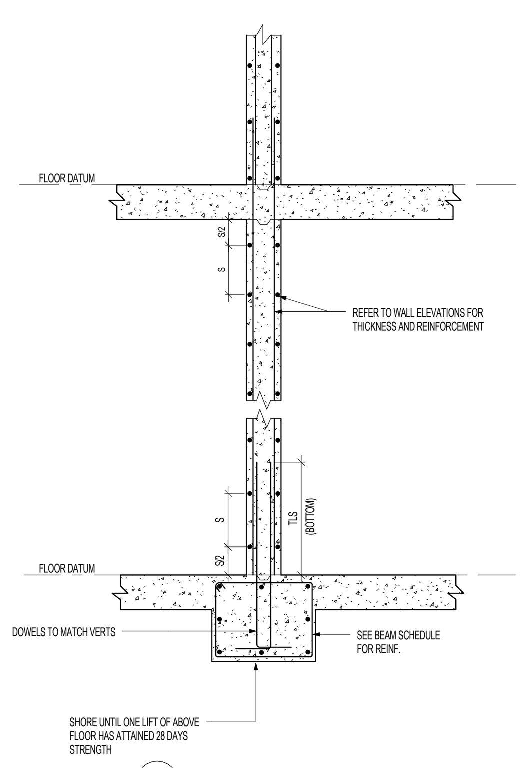
7 TYPICAL TIE DETAIL FOR DISTRIBUTED VERTICAL WALL REINFORCEMENT



8 TYPICAL COUPLING BEAM DETAIL PLAN DETAIL



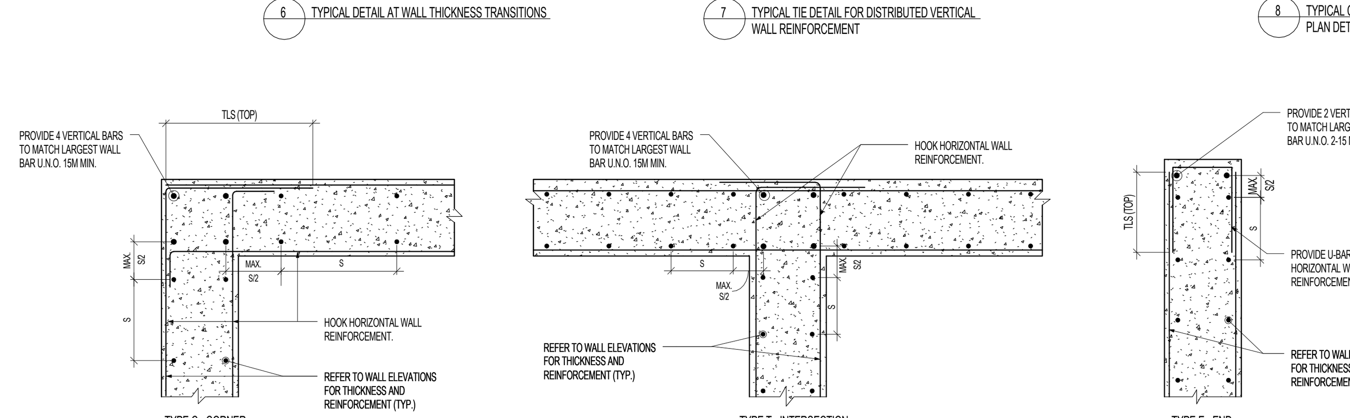
9 TYPICAL DETAIL OF SHEAR WALL SUPPORTED BY TRANSFER SLAB.



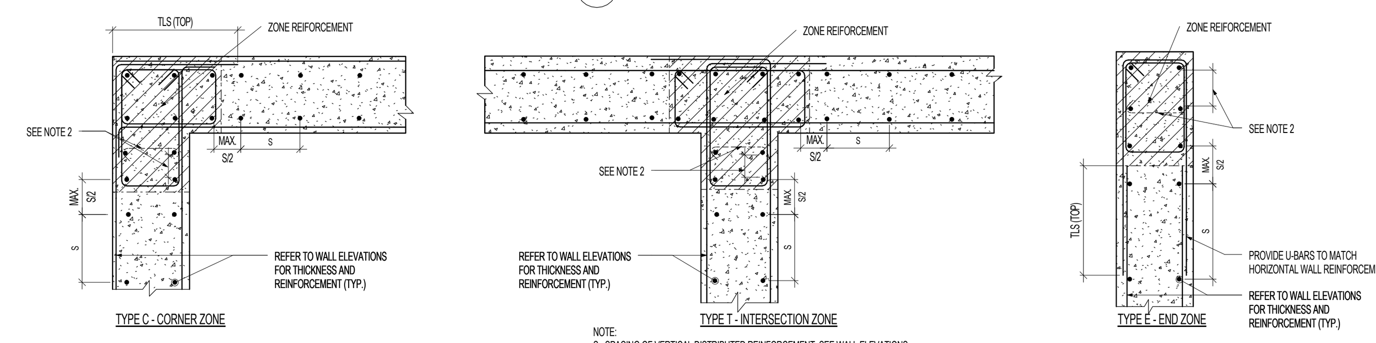
10 TYPICAL DETAIL OF SHEAR WALL SUPPORTED BY BEAM.

LEGEND

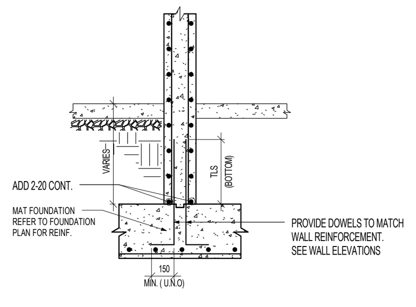
(dashed outline)	WALLS AND COLUMN SHADED AS THUS INDICATE WALL AND COLUMN BELOW SLAB
(shaded)	WALLS AND COLUMN SHADED AS THUS INDICATE WALL AND COLUMN ABOVE SLAB
(-)	- INDICATES COLUMN NUMBER OF BELOW SLAB. REFER TO COLUMN SCHEDULE
(-) dashed	- INDICATES COLUMN NUMBER OF ABOVE SLAB. REFER TO COLUMN SCHEDULE



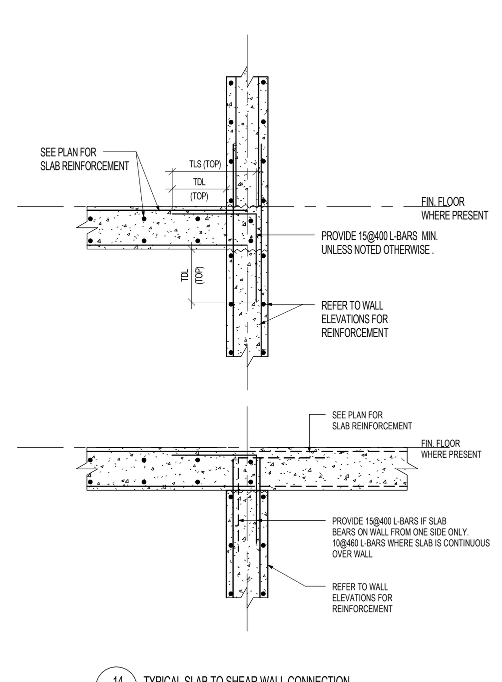
11 TYPICAL WALL DETAILS WITHOUT ZONE REINFORCEMENT



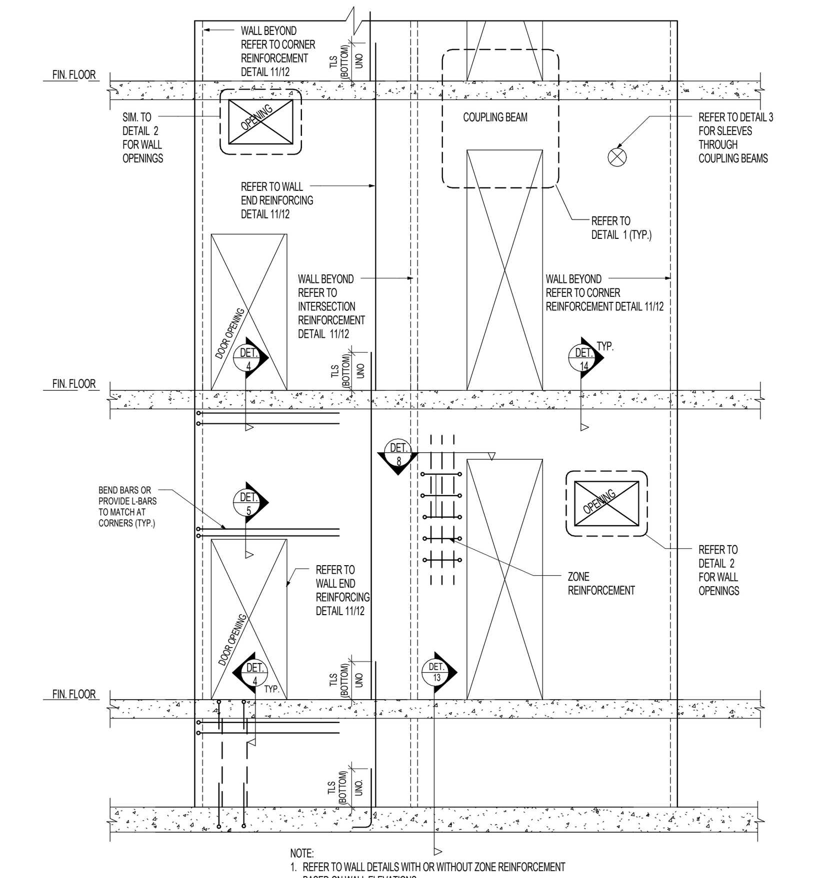
12 TYPICAL WALL DETAILS WITH ZONE REINFORCEMENT



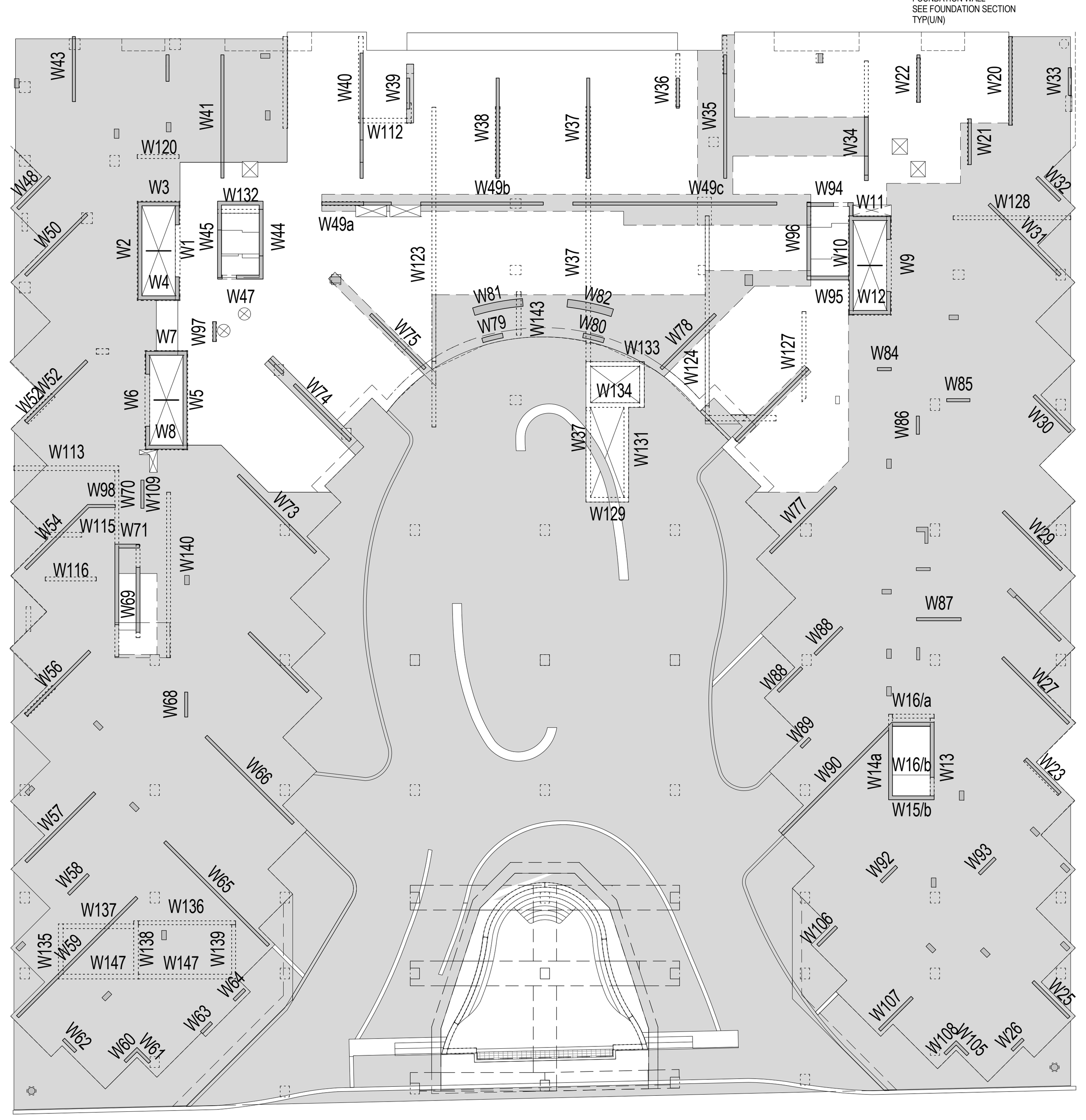
13 TYPICAL SHEAR WALL DOWELS AT FOUNDATION



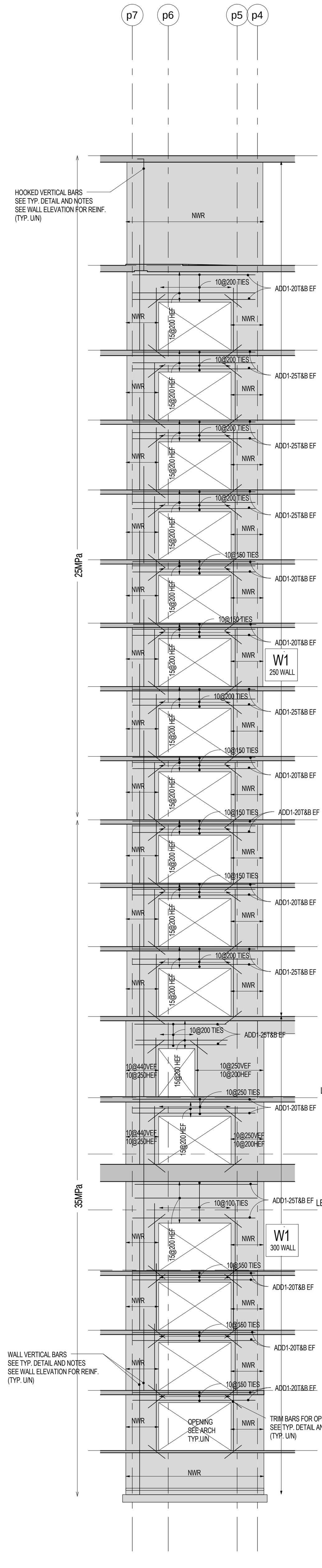
14 TYPICAL SLAB TO SHEAR WALL CONNECTION WHERE SLAB IS BEARING ON ONE SIDE OF THE WALL ONLY



15 TYPICAL WALL ELEVATIONS



WALL ELEVATION KEY PLAN
1 : 175



W1 WALL ELEVATION
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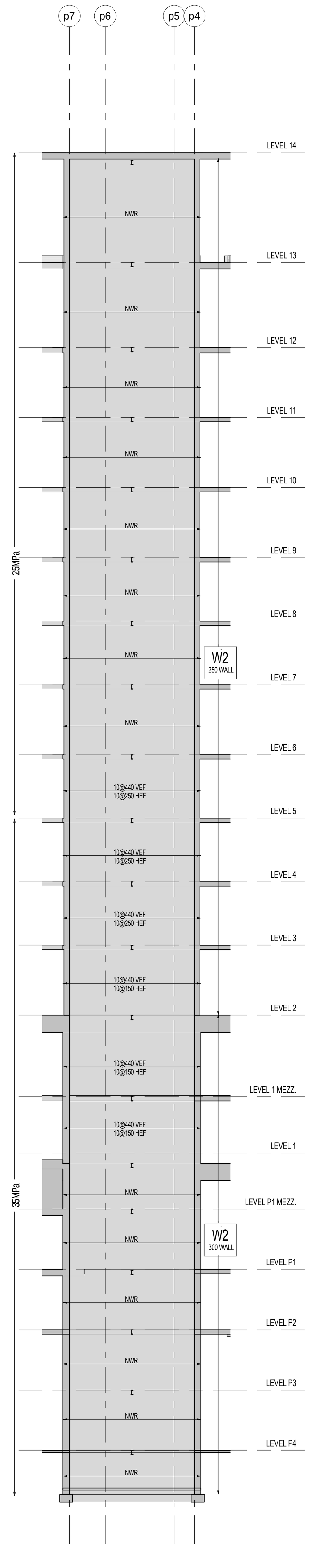

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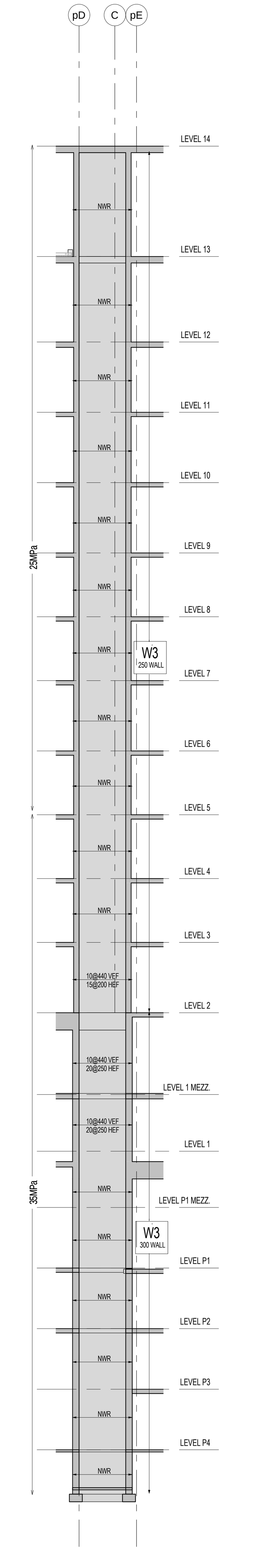

W3 WALL ELEVATION
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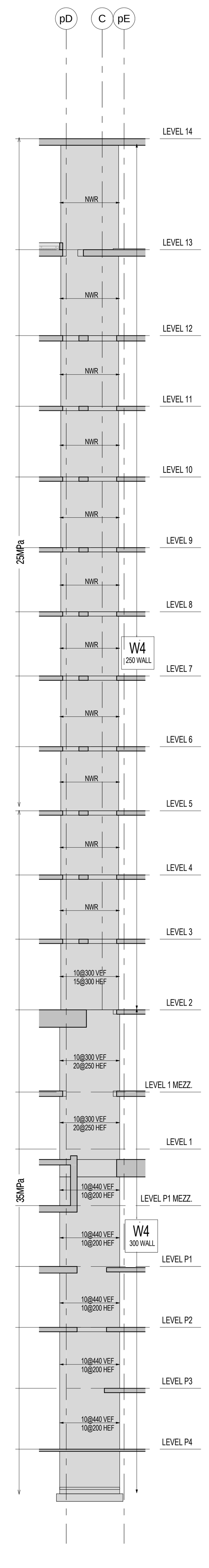

W4 WALL ELEVATION
S3-01 1 : 100

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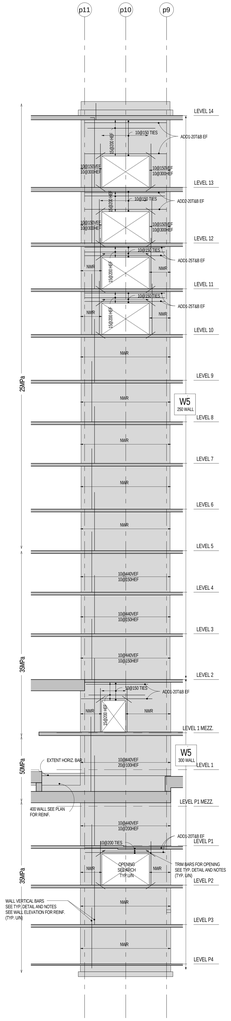
No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR WT REVIEW	May 01,2015

AQUAVISTA AT BAYSIDE TORONTO

Drawing Title
WALL ELEVATIONS AND NOTES

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Whart
Toronto, ON

Scale: As indicated
Drawn by: DC
Checked by: BK/BBS
Project No.: 20140031
Date: JUNE 12,2015
Drawing No.: S3-01



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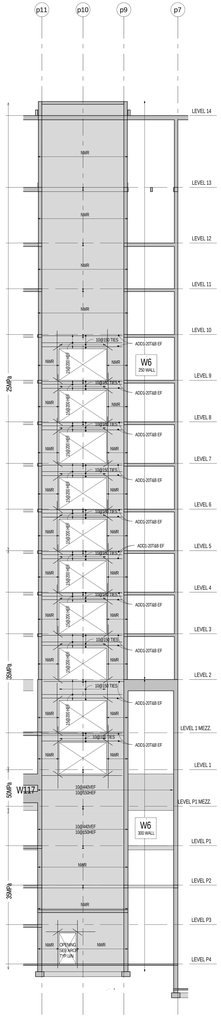

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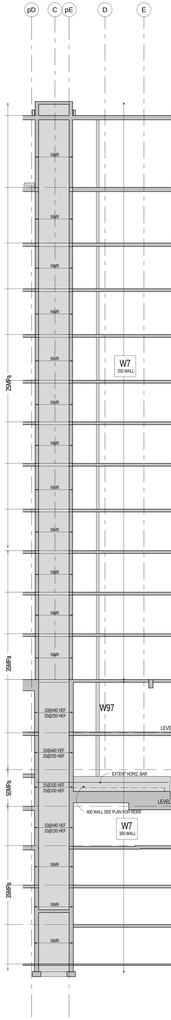

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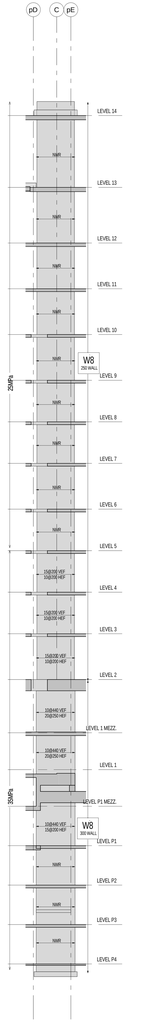

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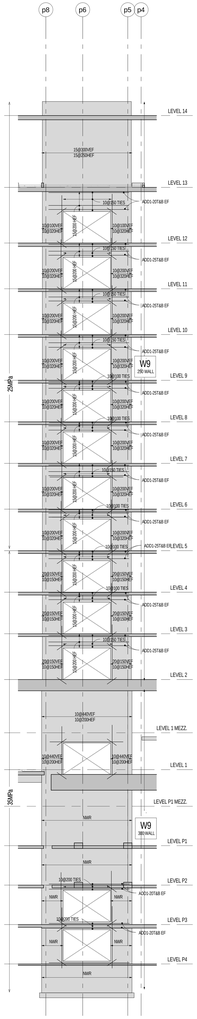

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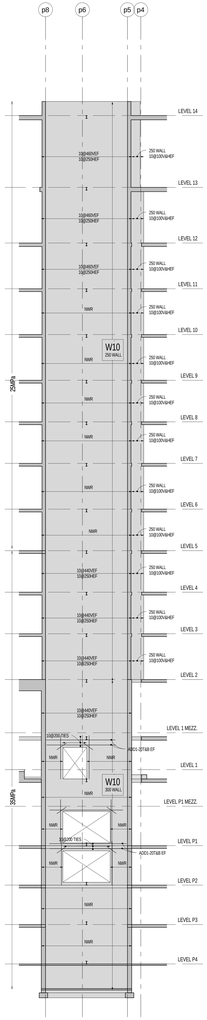

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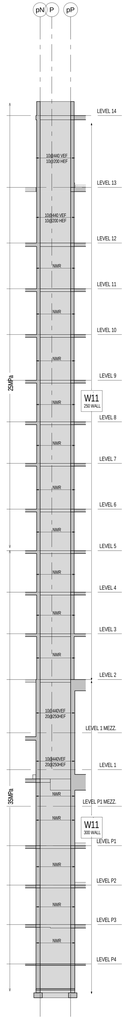

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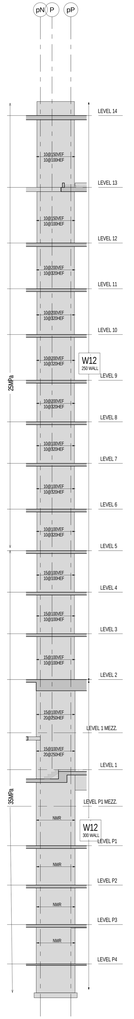

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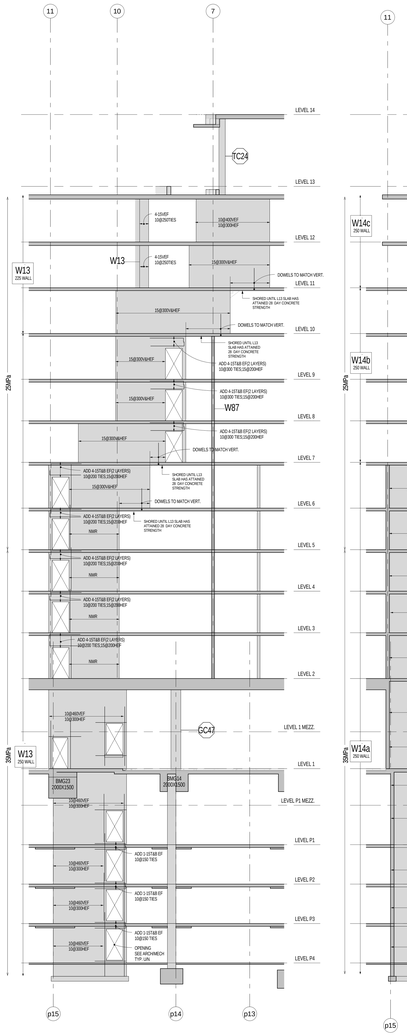

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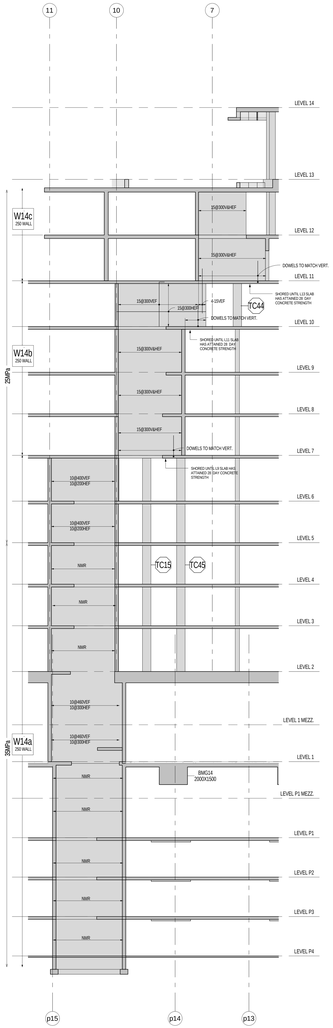

W14 WALL ELEVATION
S3-02 1 : 100

REFER TO NOTES ON SHEET S3-01

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

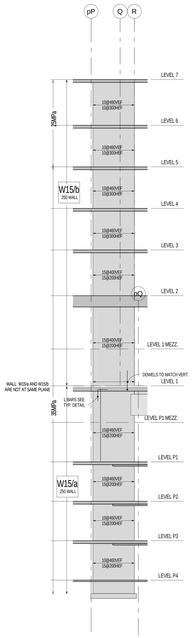
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
WALL ELEVATIONS

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale: 1 : 100
Drawn by: Author
Checked by: Checker
Project No: 2014-00031
Date: JUNE 12,2015
Drawing No: S3-02

Plot Date: 6/11/2015 12:05:25 PM



W15 WALL ELEVATION
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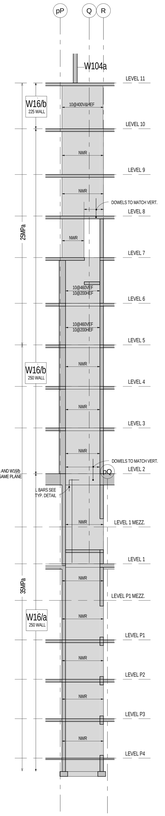

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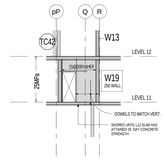

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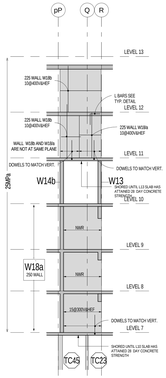
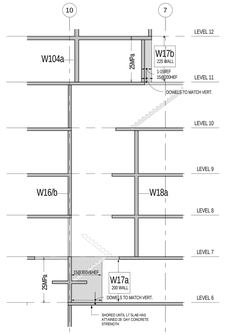


W18 WALL ELEVATION
S3-03 1 : 100



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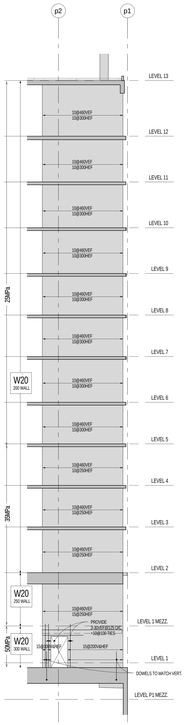

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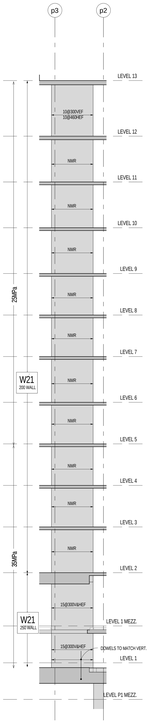

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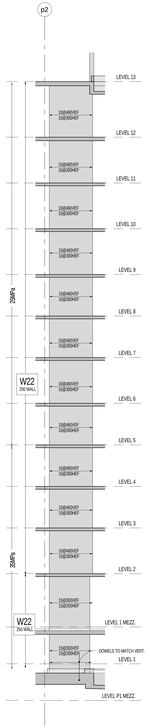

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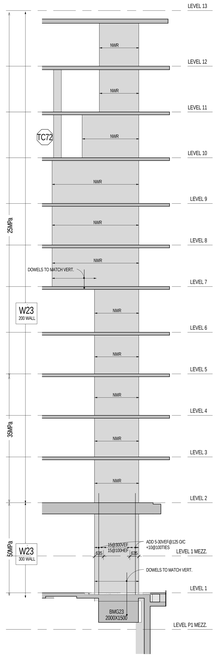

W23 WALL ELEVATION
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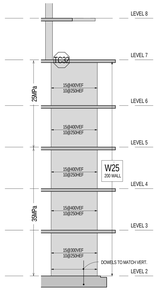
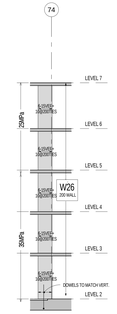
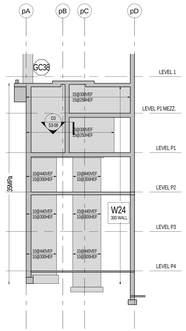


W25 WALL ELEVATION
S3-03 1 : 100



W26 WALL ELEVATION
S3-03 1 : 100



W24 WALL ELEVATION
S3-03 1 : 100

REFER TO NOTES ON SHEET S3-01

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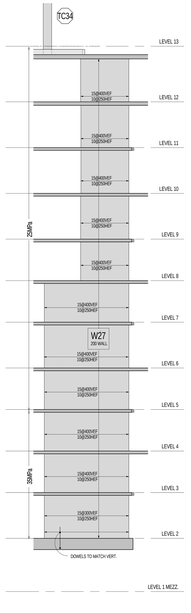
No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

AQUAVISTA
AT BAYSIDE TORONTO

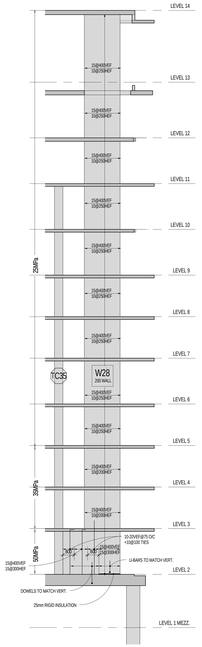
Drawing Title
WALL ELEVATIONS

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	1 : 100
Drawn by	DC
Checked by	BBS
Project No.	2014-0031
Date	JUNE 12,2015
Drawing No.	S3-03



W27 WALL ELEVATION
S3-04 1 : 100



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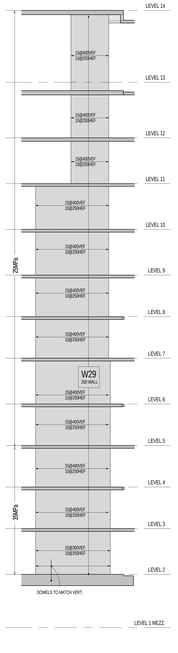
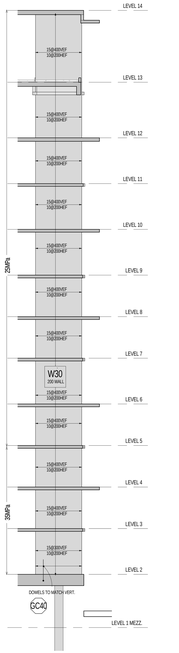
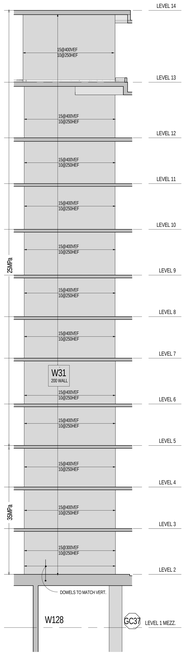
S3-04 1 : 100



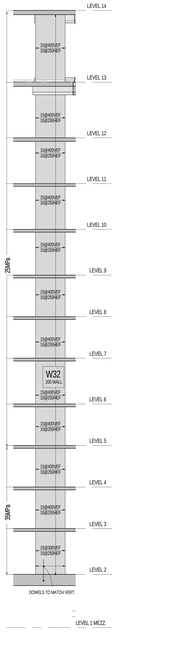
W29 WALL ELEVATION
S3-04 1 : 100



W30 WALL ELEVATION
S3-04 1 : 100



W31 WALL ELEVATION
S3-04 1 : 100



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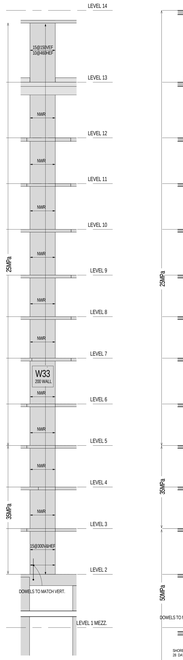
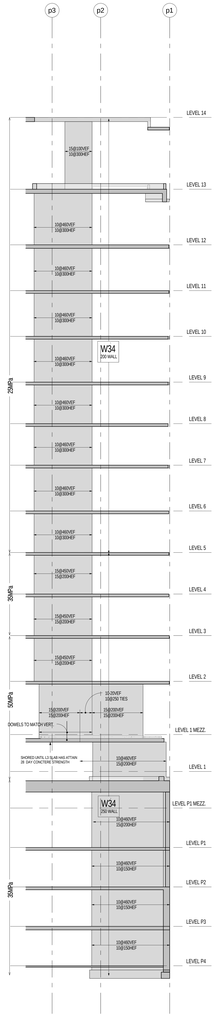
S3-04 1 : 100



W33 WALL ELEVATION
S3-04 1 : 100



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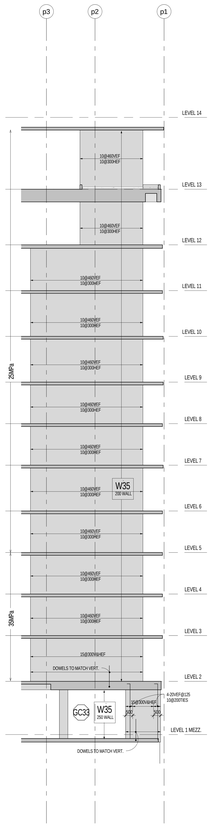
S3-04 1 : 100



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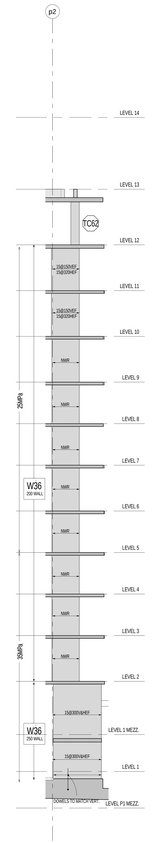
S3-04 1 : 100



W36 WALL ELEVATION
S3-04 1 : 100

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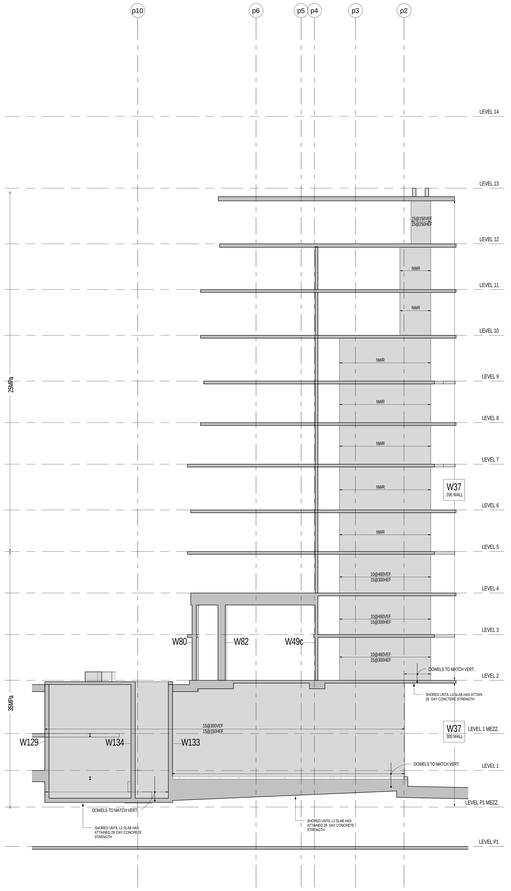
AQUAVISTA
AT BAYSIDE TORONTO

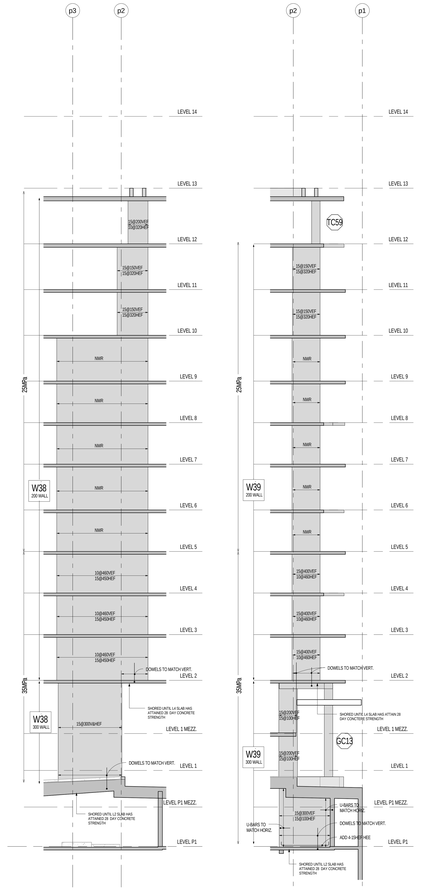
Drawing Title
WALL ELEVATIONS

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	1 : 100
Drawn by	DC
Checked by	BK/BBS
Project No.	2014-0031
Date	JUNE 12,2015
Drawing No.	S3-04

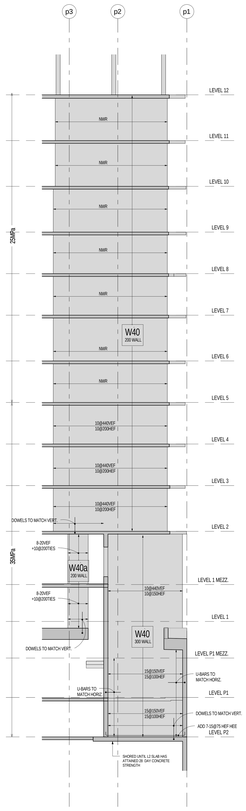


W37 WALL ELEVATION
S3-05 1 : 100



W38 WALL ELEVATION
S3-05 1 : 100

W39 WALL ELEVATION
S3-05 1 : 100



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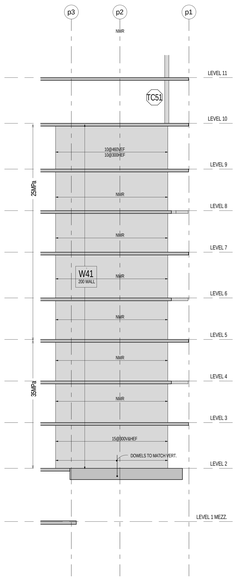
S3-05 1 : 100



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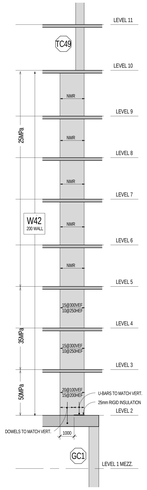
S3-05 1 : 100



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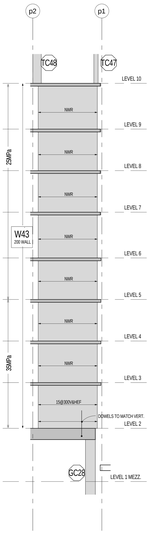
S3-05 1 : 100



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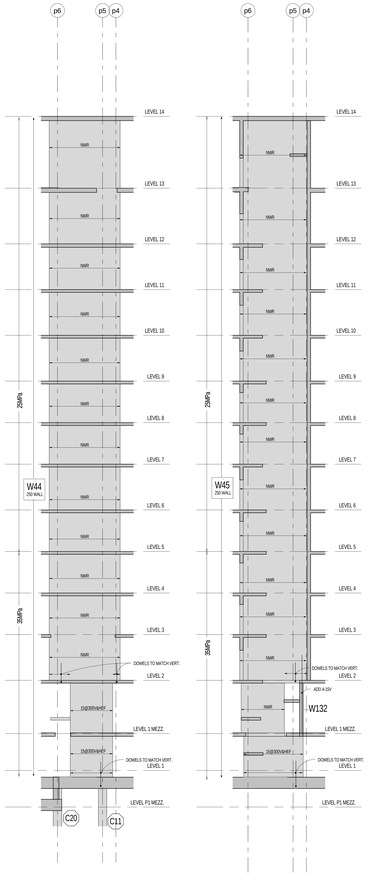
S3-05 1 : 100



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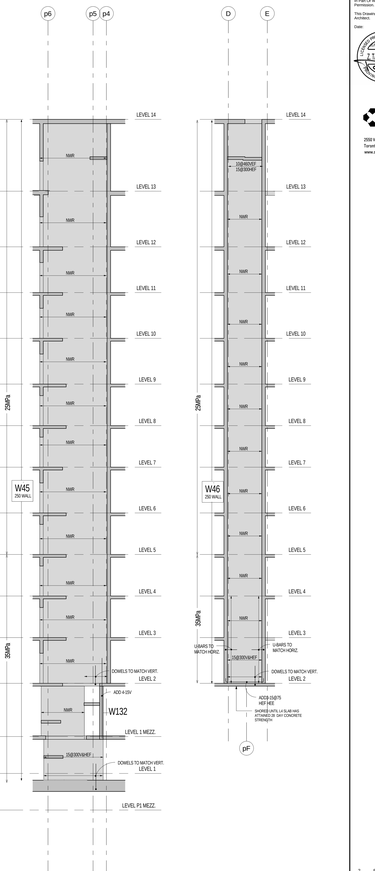
S3-05 1 : 100



W45 WALL ELEVATION
S3-05 1 : 100

W46 WALL ELEVATION
S3-05 1 : 100

REFER TO NOTES ON SHEET S3-01

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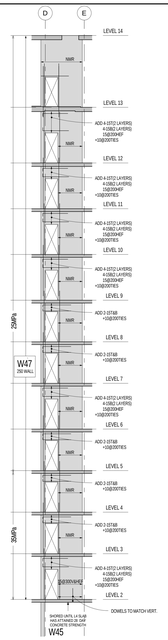
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
WALL ELEVATIONS

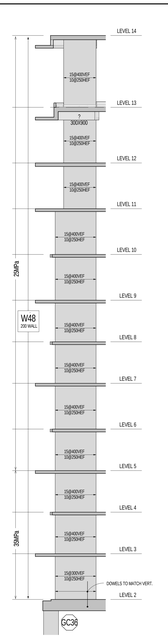
Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	1 : 100
Drawn by	DC
Checked by	Checker
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S3-05

Plot Date: 6/11/2015 12:05:34 PM



W47 WALL ELEVATION
S3-06 1 : 100



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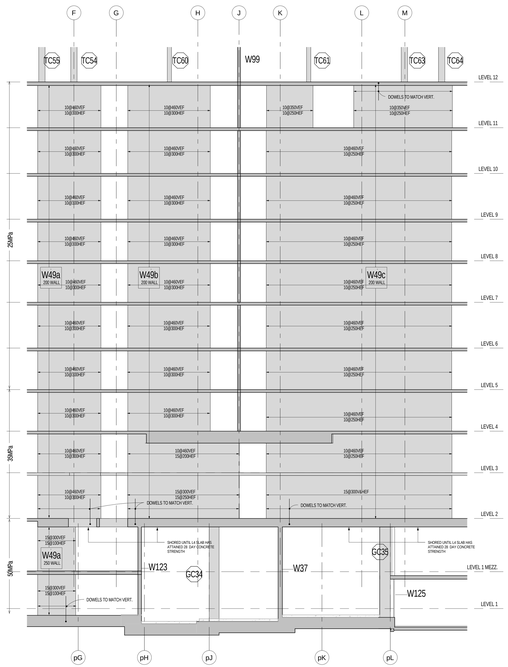
S3-06 1 : 100



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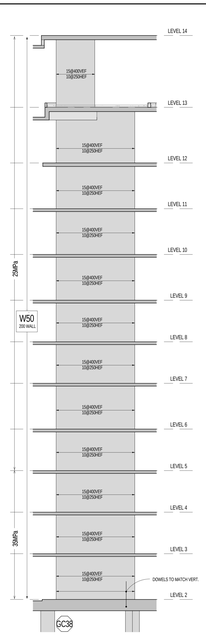

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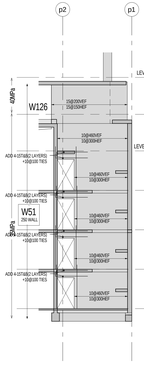
S3-06 1 : 100



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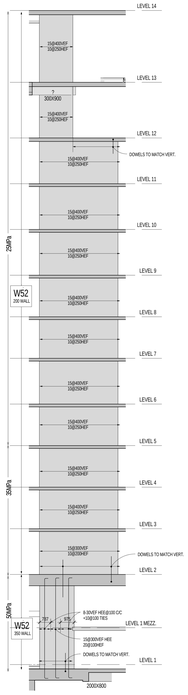
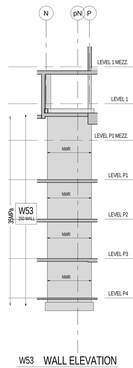
S3-06 1 : 100

C10



W52 WALL ELEVATION
S3-06 1 : 100



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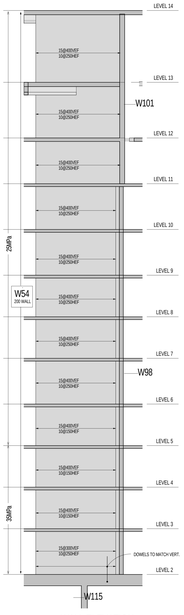
S3-06 1 : 100



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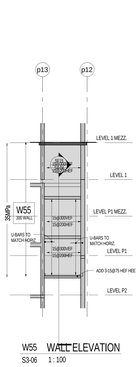

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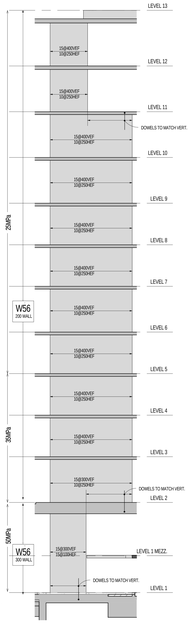

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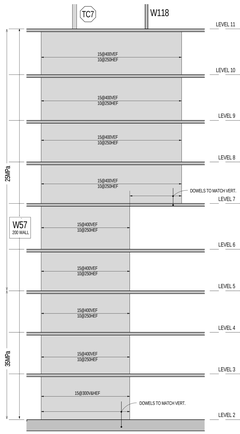

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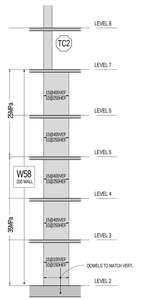

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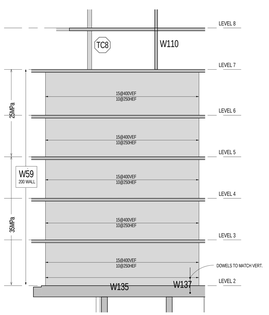

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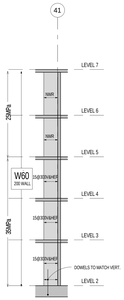

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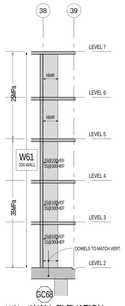
S3-06 1 : 100



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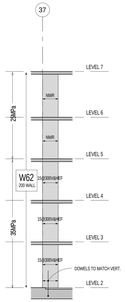

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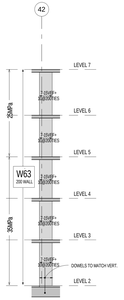
S3-06 1 : 100



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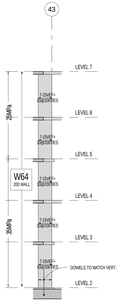

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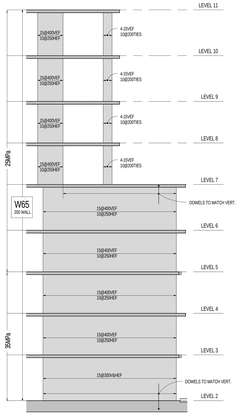

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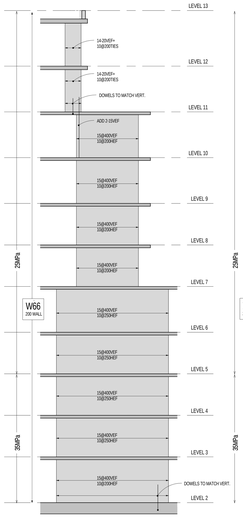
S3-06 1 : 100



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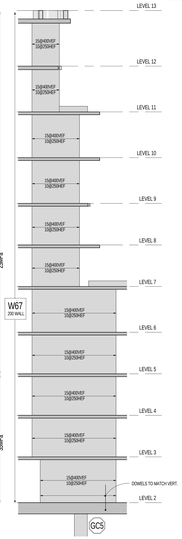
S3-06 1 : 100



W67 WALL ELEVATION
S3-06 1 : 100

REFER TO NOTES ON SHEET S3-01

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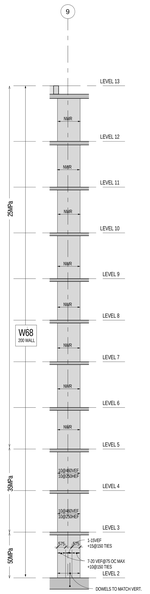
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
WALL ELEVATIONS

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

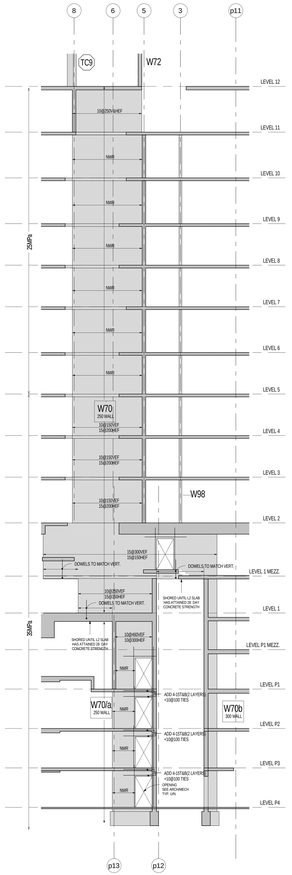
Scale	1 : 100
Drawn by	DC
Checked by	BK/BBS
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S3-06

Plot Date: 6/12/2015 12:05:39 PM

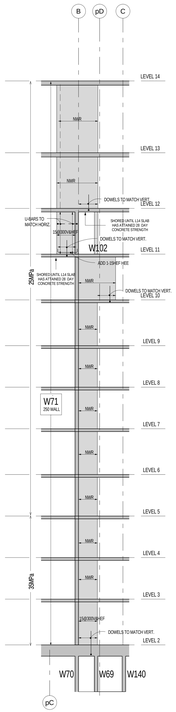


W68 WALL ELEVATION
S3-07 1 : 100

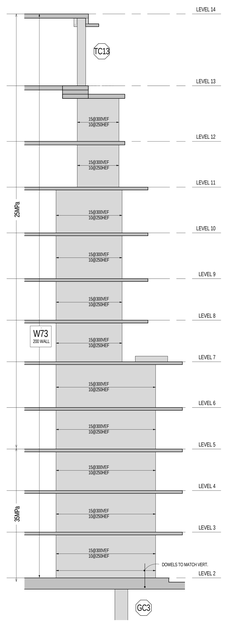
W69 WALL ELEVATION
S3-07 1 : 100



W70 WALL ELEVATION
S3-07 1 : 100



W71,W72 WALL ELEVATION
S3-07 1 : 100



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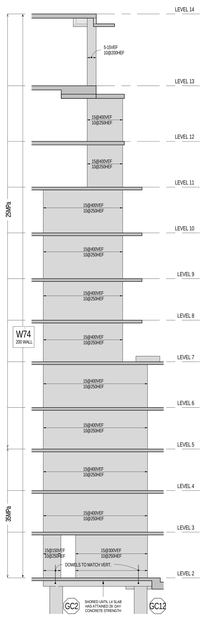
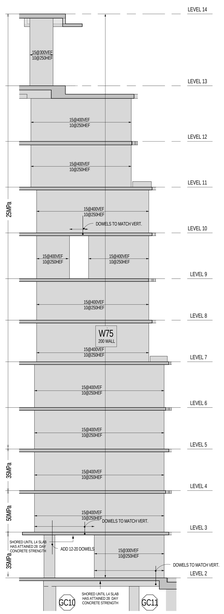
S3-07 1 : 100



W74 WALL ELEVATION
S3-07 1 : 100



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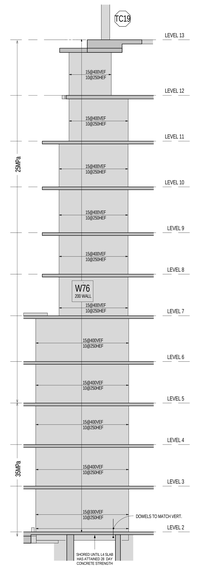

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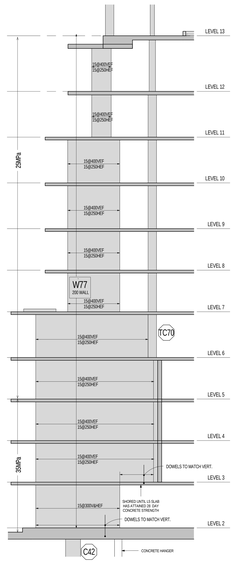
S3-07 1 : 100



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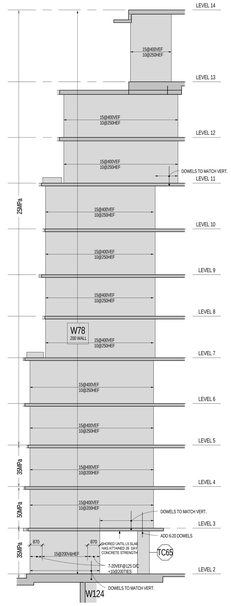
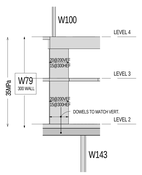
S3-07 1 : 100



W78 WALL ELEVATION
S3-07 1 : 100



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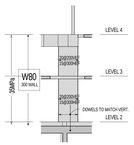
S3-07 1 : 100



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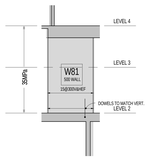
S3-07 1 : 100



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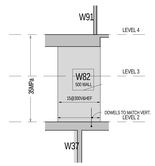
S3-07 1 : 100



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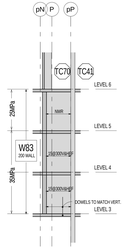
S3-07 1 : 100



W83 WALL ELEVATION
S3-07 1 : 100

REFER TO NOTES ON SHEET S3-01

Contractor Must Check And Verify All Dimensions On The Job.
Do Not Scale The Drawings.
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Date:

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

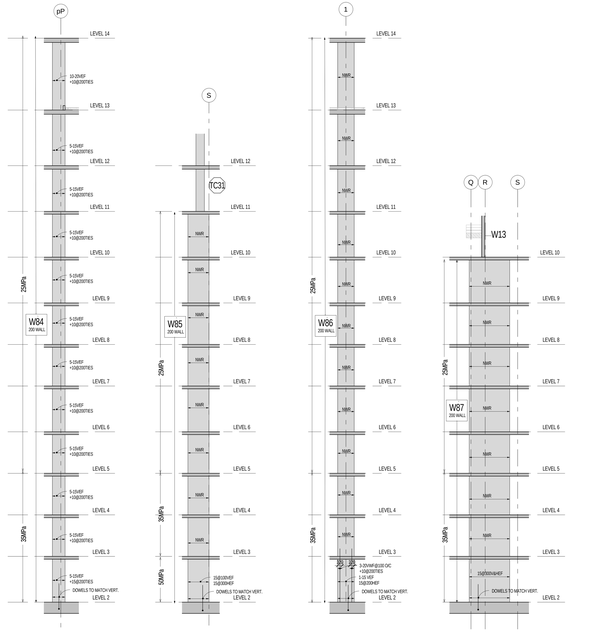
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
WALL ELEVATIONS

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	1 : 100
Drawn by	DC
Checked by	Checker
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S3-07

Plot Date: 6/11/2015 12:05:42 PM

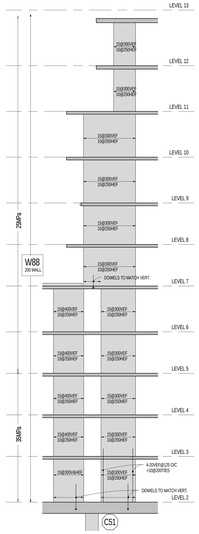


W84 WALL ELEVATION
S3-08 1 : 100

W85 WALL ELEVATION
S3-08 1 : 100

W86 WALL ELEVATION
S3-08 1 : 100

W87 WALL ELEVATION
S3-08 1 : 100



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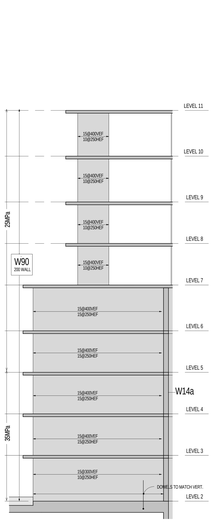
S3-08 1 : 100

W89 WALL ELEVATION
S3-08 1 : 100



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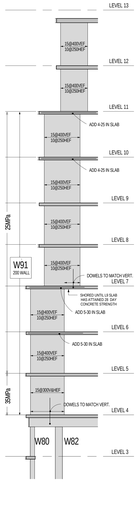

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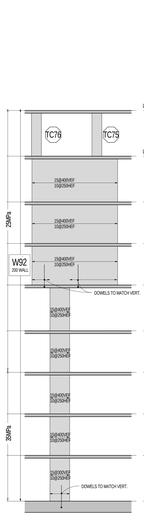
S3-08 1 : 100



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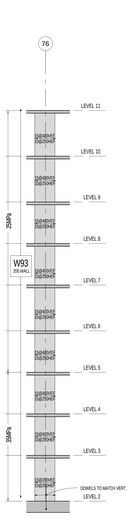
S3-08 1 : 100



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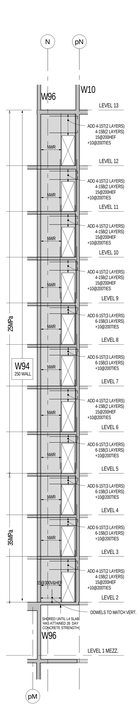
S3-08 1 : 100



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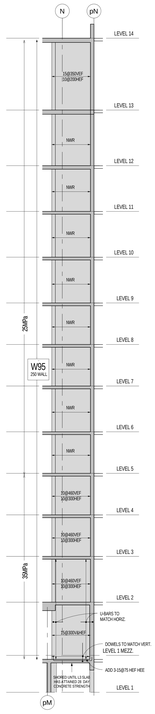
S3-08 1 : 100



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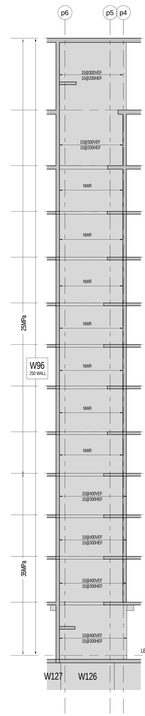
S3-08 1 : 100



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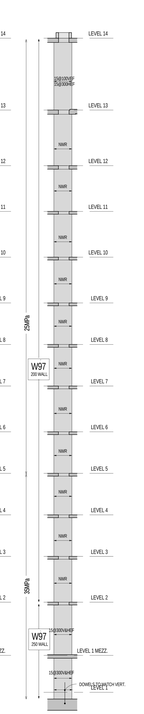

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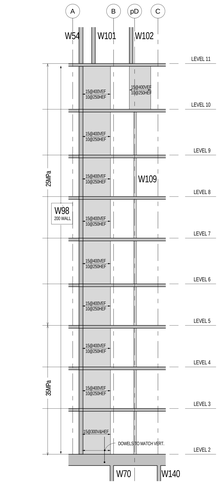
S3-08 1 : 100



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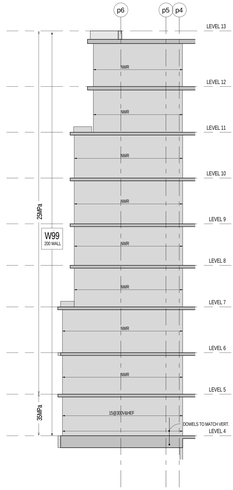

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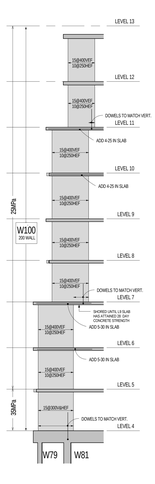

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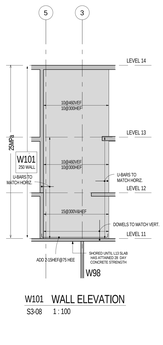
S3-08 1 : 100



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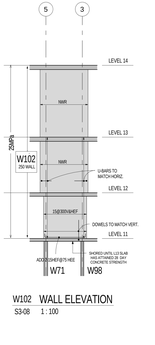
S3-08 1 : 100



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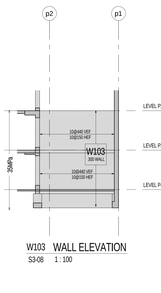

W103 WALL ELEVATION
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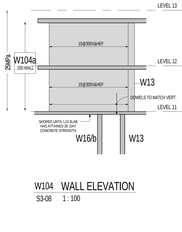

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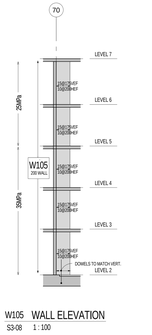

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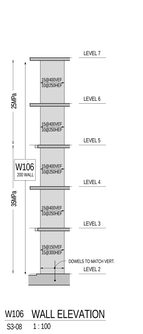

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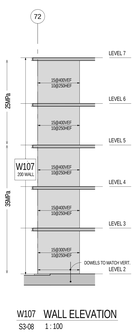
S3-08 1 : 100



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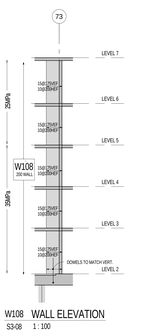
S3-08 1 : 100



W108 WALL ELEVATION
S3-08 1 : 100

REFER TO NOTES ON SHEET S3-01

Contractor Must Check And Verify All Dimensions On The Job.
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Date

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Toronto ON M2J 5A9 | Tel: (416) 499 9676
www.stephenson-eng.com | info@stephenson-eng.com

No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

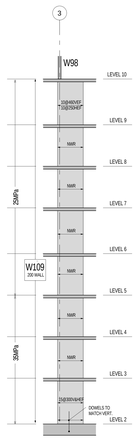
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
WALL ELEVATIONS

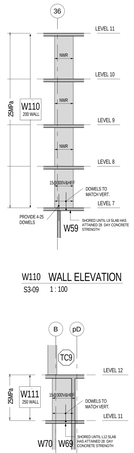
Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	1 : 100
Drawn by	DC
Checked by	BK/BBS
Project No	2014003I
Date	JUNE 12,2015
Drawing No	S3-08

Plot Date: 6/12/2015 12:05:48 PM



W109 WALL ELEVATION
S3-09 1 : 100



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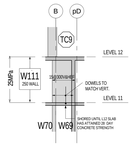

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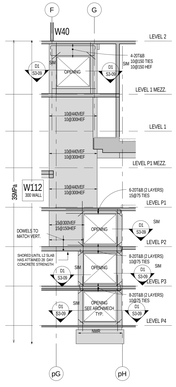
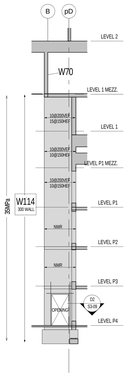
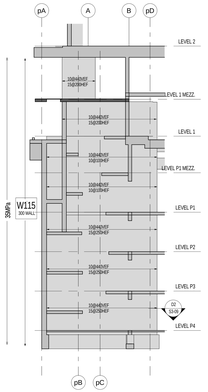


W112 WALL ELEVATION
S3-09 1 : 100

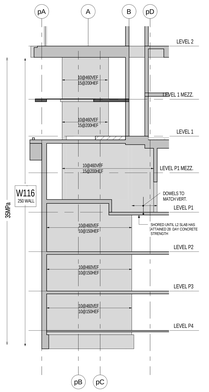
W113 WALL ELEVATION
S3-09 1 : 100



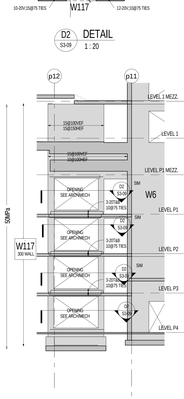
W114 WALL ELEVATION
S3-09 1 : 100



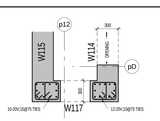
W115 WALL ELEVATION
S3-09 1 : 100



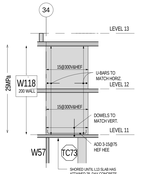
W116 WALL ELEVATION
S3-09 1 : 100



W117 WALL ELEVATION
S3-09 1 : 100



D2 DETAIL
S3-09 1 : 20



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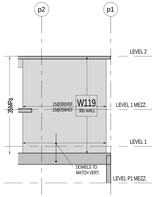
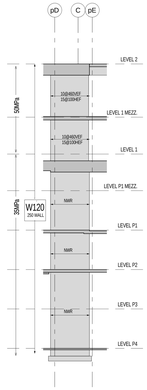
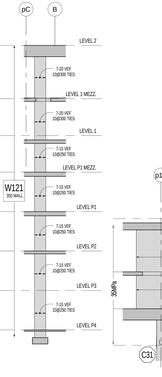
S3-09 1 : 100



W119 WALL ELEVATION
S3-09 1 : 100



W120 WALL ELEVATION
S3-09 1 : 100



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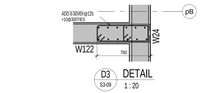
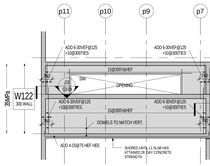
S3-09 1 : 100



D3 DETAIL
S3-09 1 : 20



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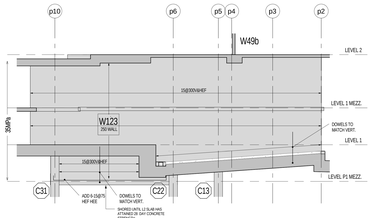
S3-09 1 : 100



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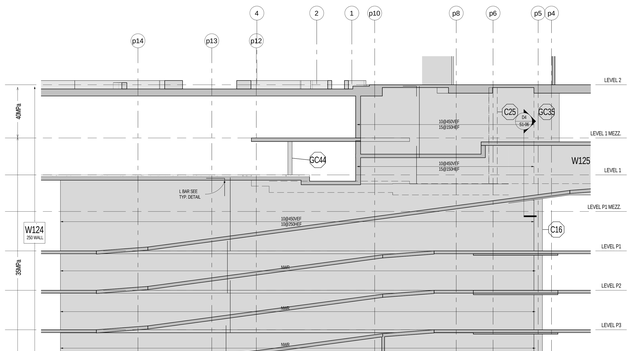
S3-09 1 : 100



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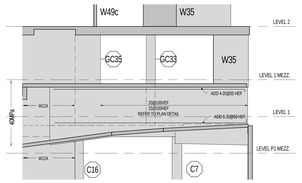

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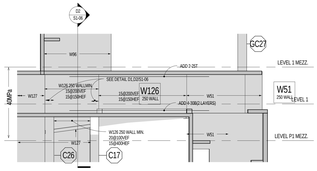
S3-09 1 : 100



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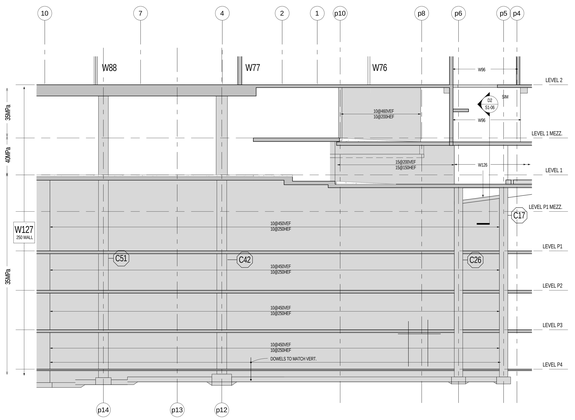
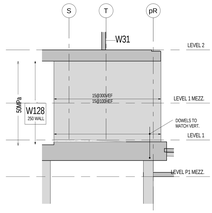
S3-09 1 : 100



W127 WALL ELEVATION
S3-09 1 : 100



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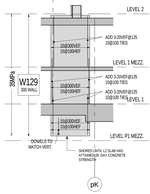
S3-09 1 : 100



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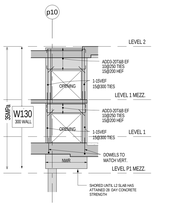
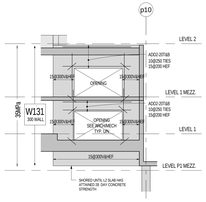
S3-09 1 : 100



W130 WALL ELEVATION
S3-09 1 : 100



W131 WALL ELEVATION
S3-09 1 : 100

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

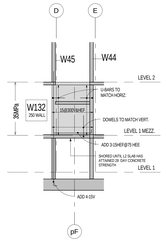
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
WALL ELEVATIONS

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	DC
Checked by	BK/BBS
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S3-09

Plot Date: 6/11/2015 12:05:53 PM



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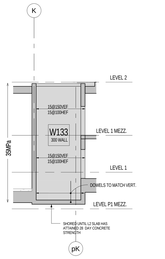
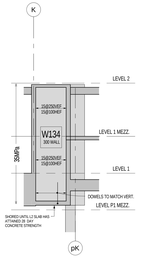
S3-10 1 : 100



W133 WALL ELEVATION
S3-10 1 : 100



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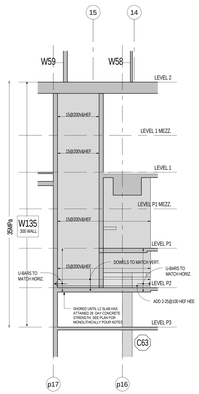
S3-10 1 : 100



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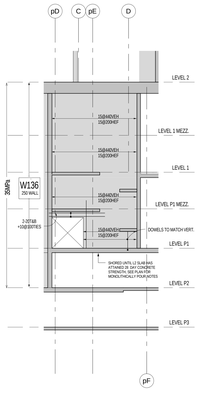

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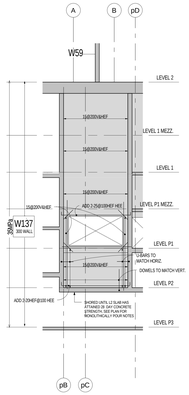

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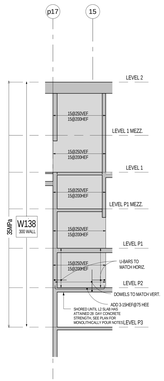
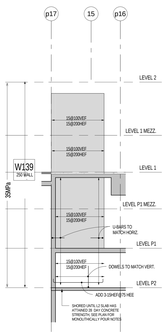


W138 WALL ELEVATION
S3-10 1 : 100



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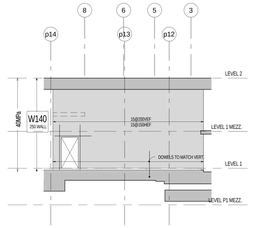

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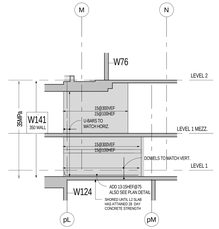

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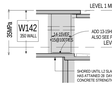
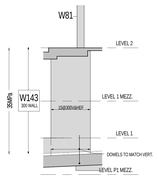
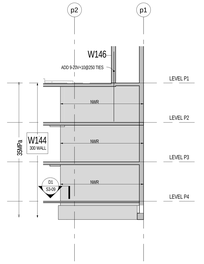


W142 WALL ELEVATION
S3-10 1 : 100



W143 WALL ELEVATION
S3-10 1 : 100



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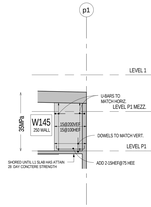
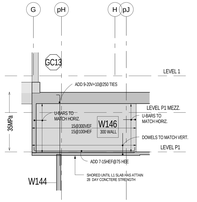
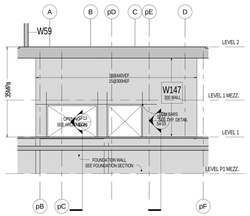


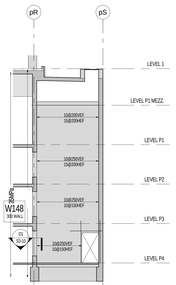
W145 WALL ELEVATION
S3-10 1 : 100



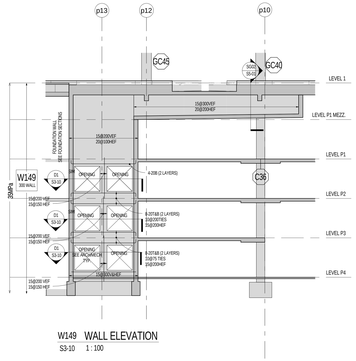
W146 WALL ELEVATION
S3-10 1 : 100



W147 WALL ELEVATION
S3-10 1 : 100



W148 WALL ELEVATION
S3-10 1 : 100



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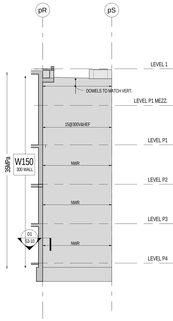
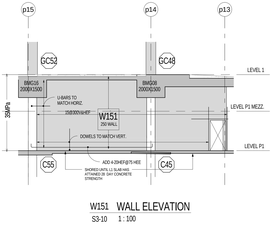
S3-10 1 : 100



W150 WALL ELEVATION
S3-10 1 : 100



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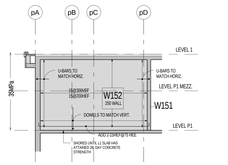
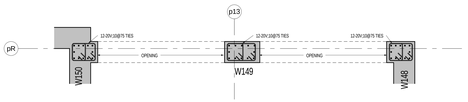


W152 WALL ELEVATION
S3-10 1 : 100



D1 DETAIL
S3-10 1 : 20

REFER TO NOTES ON SHEET S3-01

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

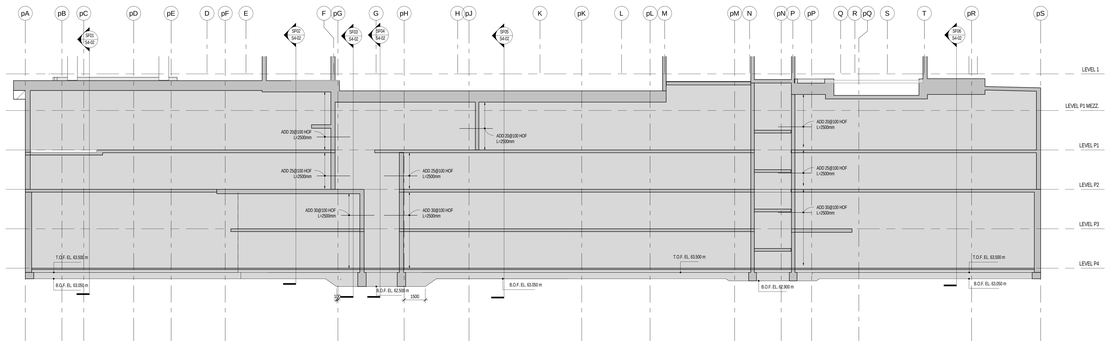
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
WALL ELEVATIONS

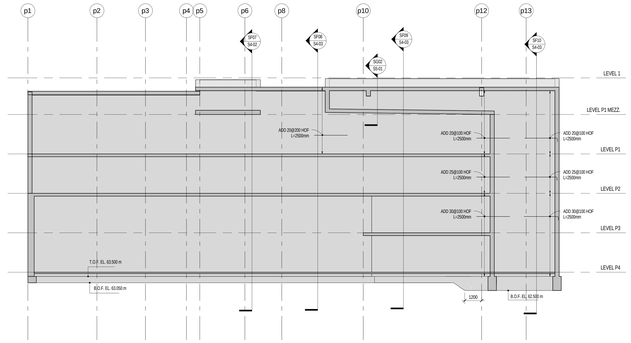
Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	Author
Checked by	Checker
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S3-10

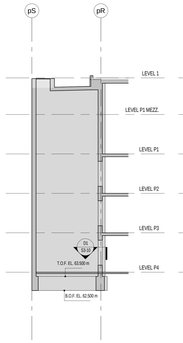
Plot Date: 6/11/2015 12:05:58 PM



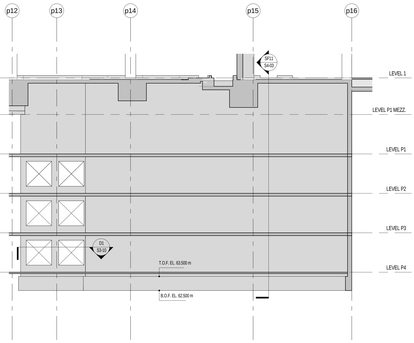
WEST FOUNDATION ELEVATION
1 : 100



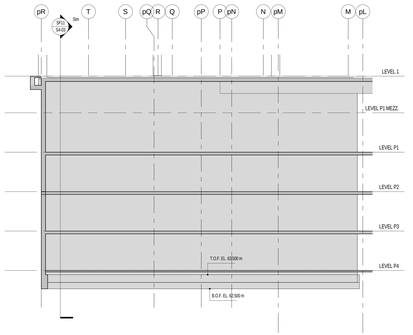
NORTH FOUNDATION ELEVATION 1
1 : 100



NORTH FOUNDATION ELEVATION 2
1 : 100

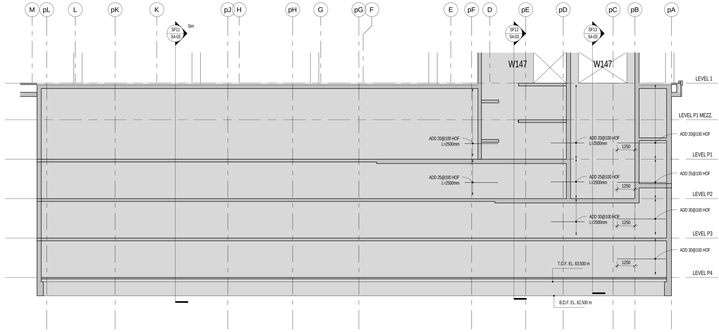


NORTH FOUNDATION ELEVATION 3
1 : 100

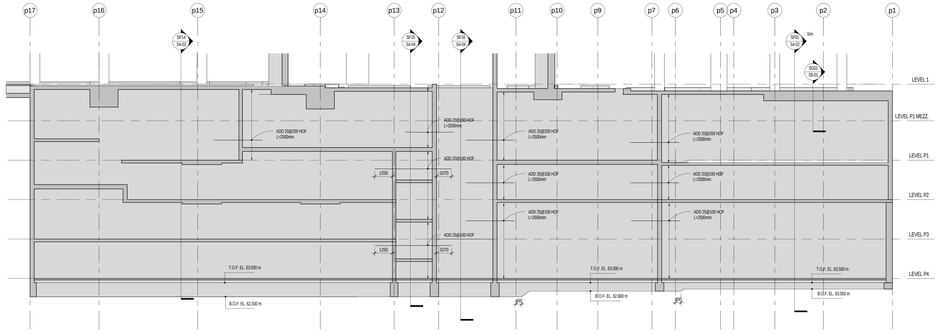


EAST FOUNDATION ELEVATION 1
1 : 100

EAST FOUNDATION ELEVATION 2
1 : 100



EAST FOUNDATION ELEVATION 3
1 : 100



SOUTH FOUNDATION ELEVATION
1 : 100

1. REFER TO TYPICAL DETAILS T-C56A AND TC56B FOR TLS (TENSION LAP SPLICE) AND CLS (COMPRESSION LAP SPLICE) LENGTHS.
2. OVERALL LENGTH OF REINFORCEMENT SHOULD ACCOUNT FOR VARIATIONS IN TOP OF FOOTING ELEVATION WHILE STILL MEETING THE MINIMUM LAP SPLICES INDICATED. VARIATIONS IN FOOTING ELEVATION DURING CONSTRUCTION WILL REQUIRE MODIFICATION TO REINFORCEMENT TO ACCOMMODATE ACCORDINGLY.

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No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR WT REVIEW	May 01,2015

AQUAVISTA
AT BAYSIDE TORONTO

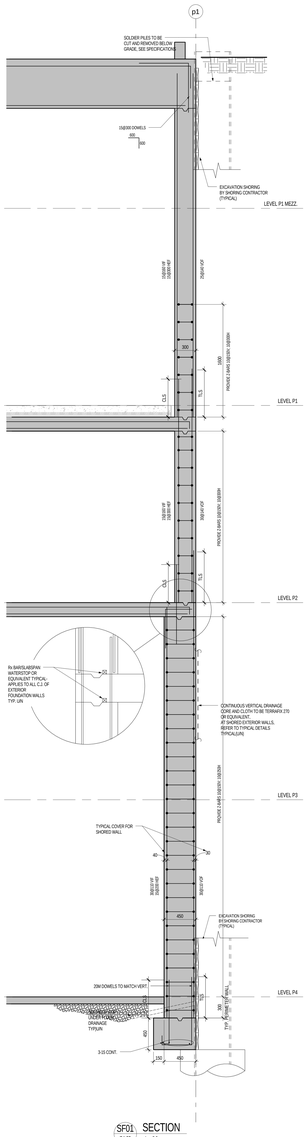
Drawing Title
FOUNDATION ELEVATIONS

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

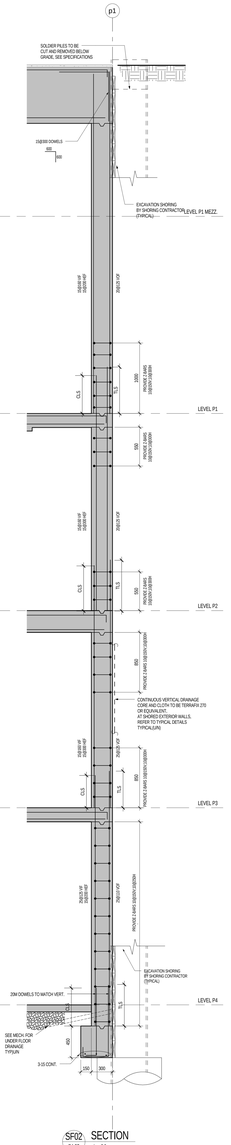
Scale: As indicated
Drawn by: DC
Checked by: BK/BBS
Project No.: 20140031
Date: JUNE 12,2015
Drawing No.: S4-01

Plot Date: 6/12/2015 12:06:01 PM

1. REFER TO TYPICAL DETAILS T-C56A AND TC56B FOR TLS (TENSION LAP SPLICE) AND CLS (COMPRESSION LAP SPLICE) LENGTHS.
2. OVERALL LENGTH OF REINFORCEMENT SHOULD ACCOUNT FOR VARIATIONS IN TOP OF FOOTING ELEVATION WHILE STILL MEETING THE MINIMUM LAP SPLICES INDICATED. VARIATIONS IN FOOTING ELEVATION DURING CONSTRUCTION WILL REQUIRE MODIFICATION TO REINFORCEMENT TO ACCOMMODATE ACCORDINGLY.



SF01 SECTION
S4-02 1 : 20



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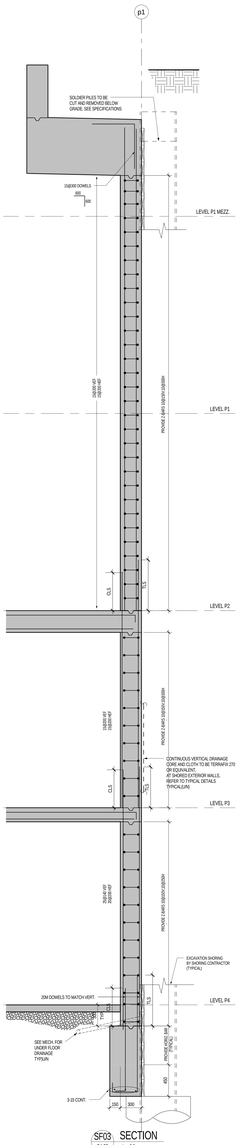

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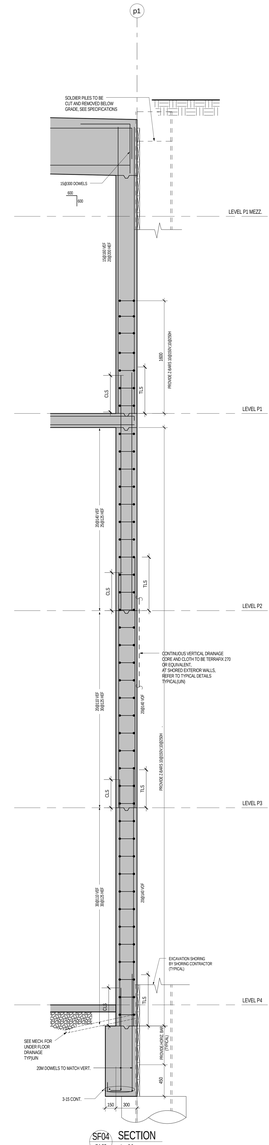

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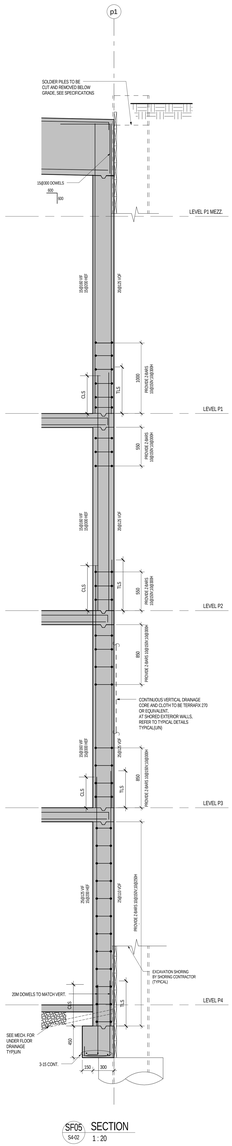

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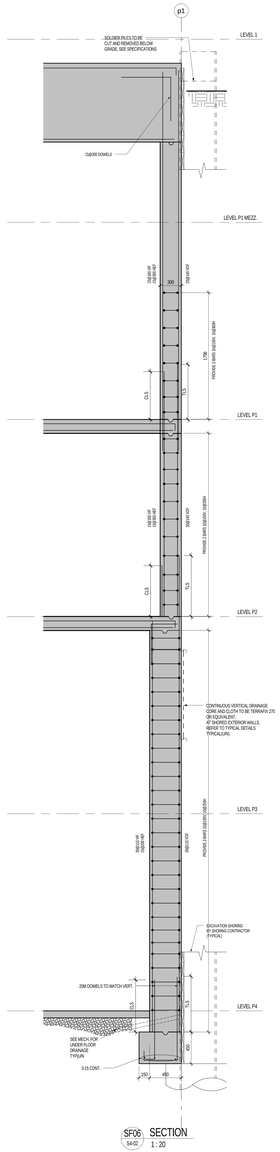

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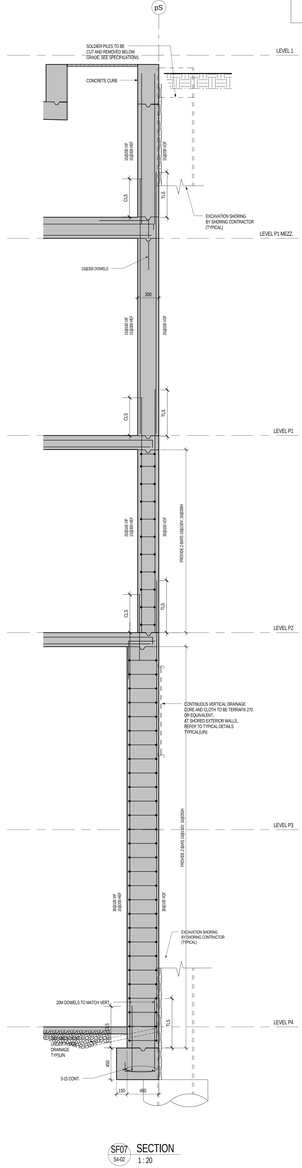

SF07 SECTION
S4-02 1 : 20

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No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

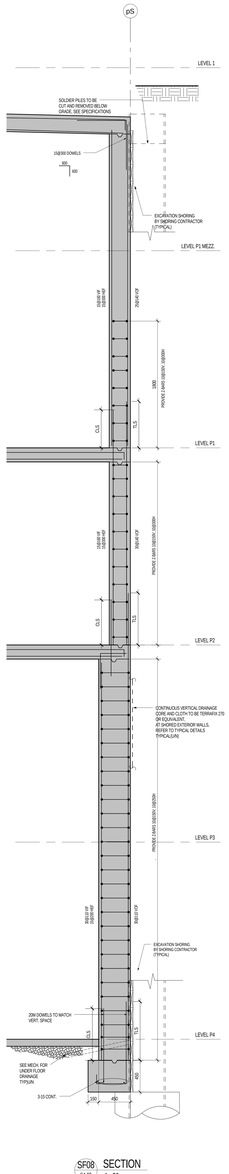
AQUAVISTA
AT BAYSIDE TORONTO

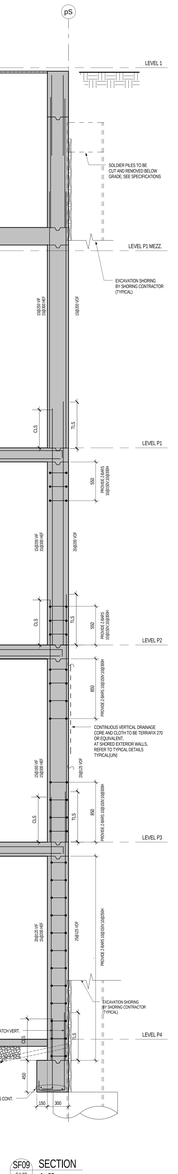
Drawing Title
FOUNDATION SECTIONS

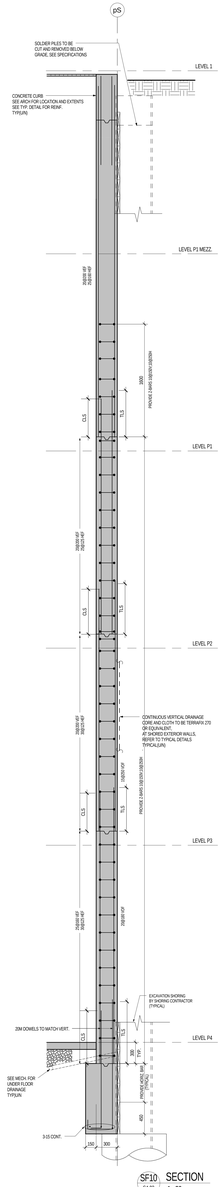
Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

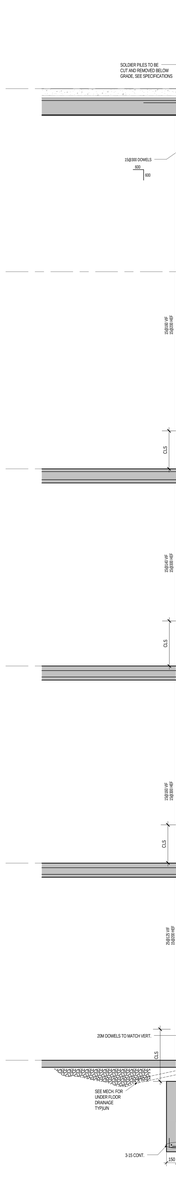
Scale	As indicated
Drawn by	DC
Checked by	BBS
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S4-02

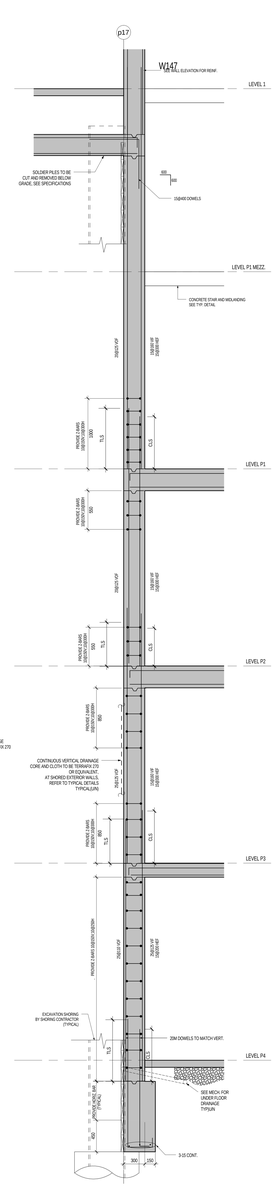
Plot Date: 6/11/2015 12:52:12 PM

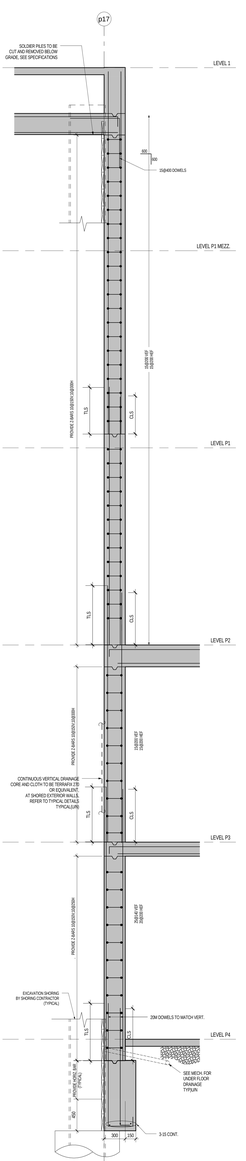


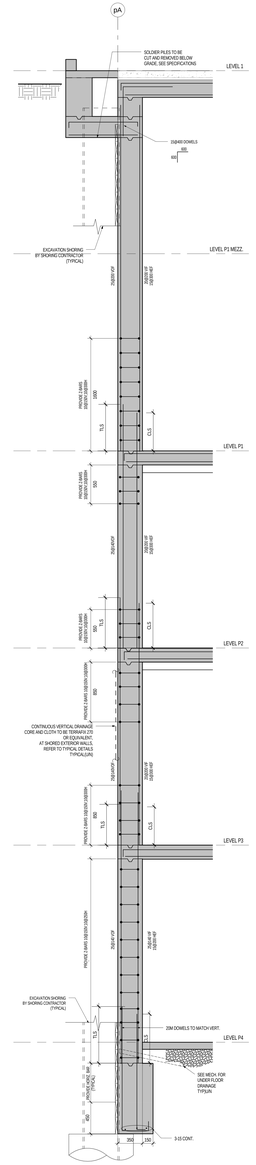












1. REFER TO TYPICAL DETAILS T-C56A AND TC56B FOR TLS (TENSION LAP SPLICE) AND CLS (COMPRESSION LAP SPLICE) LENGTHS.
2. OVERALL LENGTH OF REINFORCEMENT SHOULD ACCOUNT FOR VARIATIONS IN TOP OF FOOTING ELEVATION WHILE STILL MEETING THE MINIMUM LAP SPLICES INDICATED. VARIATIONS IN FOOTING ELEVATION DURING CONSTRUCTION WILL REQUIRE MODIFICATION TO REINFORCEMENT TO ACCOMMODATE ACCORDINGLY.

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No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

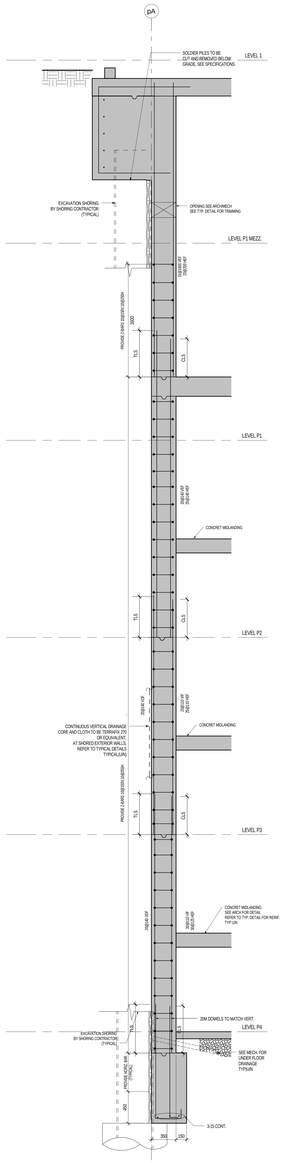
AQUAVISTA
AT BAYSIDE TORONTO

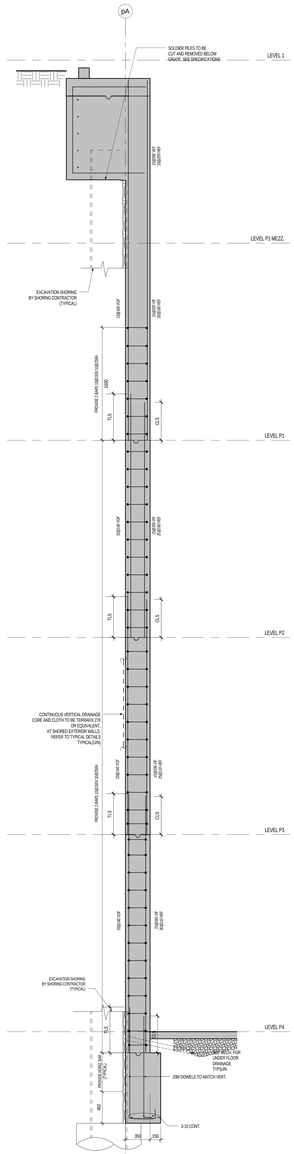
Drawing Title
FOUNDATION SECTIONS

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	DC
Checked by	Checker
Project No.	2014003
Date	JUNE 12,2015
Drawing No.	S4-03

Plot Date: 6/11/2015 12:52:16 PM





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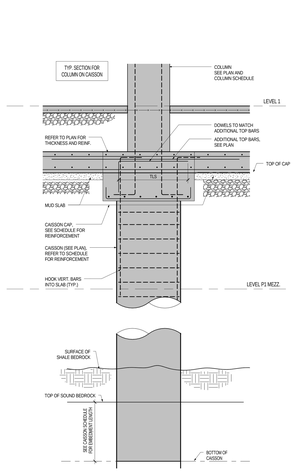
2. OVERALL LENGTH OF REINFORCEMENT SHOULD ACCOUNT FOR VARIATIONS IN TOP OF FOOTING ELEVATION WHILE STILL MEETING THE MINIMUM LAP SPLICES INDICATED. VARIATIONS IN FOOTING ELEVATION DURING CONSTRUCTION WILL REQUIRE MODIFICATION TO REINFORCEMENT TO ACCOMMODATE ACCORDINGLY.



SF17 SECTION
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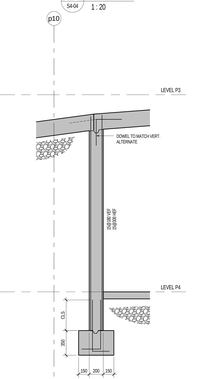

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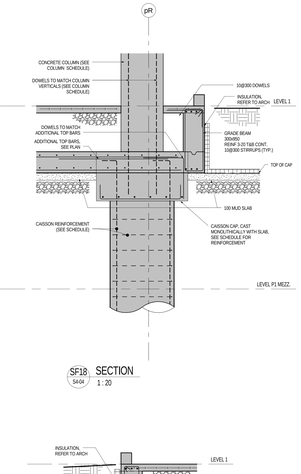

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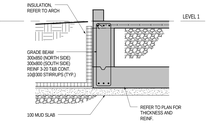

SF19 SECTION
S4-04 1 : 20

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No.	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR MT REVIEW	May 01,2015

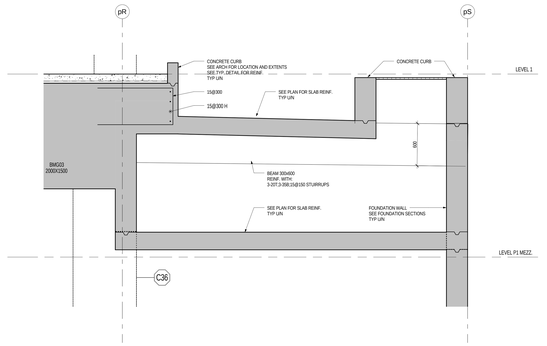
AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
FOUNDATION SECTIONS

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale	As indicated
Drawn by	Author
Checked by	Checker
Project No.	2014/0031
Date	JUNE 12,2015
Drawing No.	S4-04

Plot Date: 6/11/2015 12:52:19 PM



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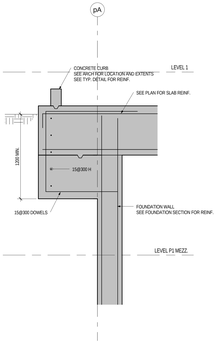

SG01 SECTION
S5-01 1 : 20

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No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015

AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
PODIUM FLOOR SECTIONS

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Wharf
Toronto, ON

Scale: 1 : 20
Drawn by: DC
Checked by: BK/BBS
Project No.: 20140031
Date: JUNE 12,2015
Drawing No.: S5-01

Plot Date: 6/12/2015 12:08:44 PM

GENERAL NOTES T-A01

1.1. GENERAL
1.1. DESIGN AND CONSTRUCTION IS TO CONFORM TO THE REQUIREMENTS OF THE 2012 ONTARIO BUILDING CODE, THE ONTARIO HIGHWAY BRIDGE DESIGN CODE, THE CANADIAN HIGHWAY BRIDGE DESIGN CODE, AND TO THE SPECIFICATION, ALL CODES, MANUALS, STANDARDS AND SPECIFICATIONS REFERRED TO SHALL BE THE LATEST EDITIONS INCLUDING ALL REVISIONS AND ADDENDA. ALL DIMENSIONS, OTHER THAN PURELY STRUCTURAL DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE CHECKED AGAINST THE ARCHITECTURAL DRAWINGS AND ANY INCONSISTENCIES REPORTED TO THE ARCHITECT BEFORE PROCEEDING WITH THE WORK. DO NOT SCALE DRAWINGS, USE FIGURED DIMENSIONS.
1.2. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS AND SIZES OF OPENINGS, TRENCHES, PITS, SUMPS, EQUIPMENT, SLEEVES, DEPRESSIONS, GROOVES AND CHAMFERS NOT INDICATED ON THE STRUCTURAL DRAWINGS. UNLESS SPECIFICALLY NOTED OTHERWISE, THE ABOVE ITEMS WHERE SHOWN ON THE STRUCTURAL DRAWINGS ARE INDICATED ONLY APPROXIMATELY AS TO SIZE AND LOCATION.
1.3. UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS, NO OPENINGS HAS BEEN MADE IN THE DESIGN FOR CONDITIONS OCCURRING DURING CONSTRUCTION. THE CONTRACTOR IS TO PROVIDE ALL NECESSARY BRACING AND SHORING REQUIRED FOR STRESSES AND INSTABILITY OCCURRING WHICH MAY CAUSE DURING CONSTRUCTION. THE CONTRACTOR SHALL ACCEPT FULL RESPONSIBILITY FOR ALL SUCH BRACING AND SHORING, AS SO BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE ALL NECESSARY BRACING, SHORING, AND TEMPORARY PLING OR OTHER TEMPORARY SUPPORTS TO SAFEGUARD ALL EXISTING OR ADJACENT STRUCTURES AFFECTED BY THIS WORK. CONSTRUCTION PROCESS, SHOP DRAWINGS FOR CONTRACTOR'S REVIEW.
2. SHOP DRAWINGS, PLACING DRAWINGS AND BAR LISTS:
2.1. SHOP DRAWINGS SHALL BE PREPARED FROM THE STRUCTURAL DRAWINGS. SUBMIT COPIES OF SHOP DRAWINGS AS DIRECTED FOR REVIEW BY THE STRUCTURAL CONSULTANT SHOP DRAWINGS TO SHOW COMPLETE INFORMATION FOR THE FABRICATION AND ERECTION OF THE STRUCTURAL COMPONENTS.
2.2. REVIEW OF SHOP DRAWINGS BY THE STRUCTURAL CONSULTANT IS ONLY TO ASSESS THAT THE SUBMITTED SHOP DRAWINGS REFLECT THE INTENT OF THE STRUCTURAL DESIGN.
2.3. REVIEW OF SHOP DRAWINGS BY THE STRUCTURAL CONSULTANT SHALL NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY FOR SHOWING THAT THE WORK IS COMPLETE, ACCURATE AND IN CONFORMITY WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS.
3. INSPECTION AND TESTING:
3.1. A SOILS CONSULTANT AND AN INDEPENDENT INSPECTION AND TESTING COMPANY ARE TO BE ENGAGED TO CARRY OUT THE FOLLOWING INSPECTIONS:
3.1.1. BEARING SOIL - REFER TO NOTES ON STRUCTURAL DRAWINGS AND TO THE SOIL REPORT.
3.1.2. PILE LENDER SLAB-ON-GRADE - CONFIRM THAT FILL MATERIAL USED IS SATISFACTORY AND THAT THE REQUIRED DEGREE OF COMPACTION HAS BEEN ATTAINED.
3.1.3. CAST-IN-PLACE AND PRECAST CONCRETE - ROUTINE INSPECTION OF MATERIALS, INCLUDING SLUMP, AIR CONTENT AND COMPRESSIVE STRENGTH TESTS. REINFORCING ROD TESTS WHEN REQUIRED, OR DIRECTED BY THE CONSULTANT OR CSA STANDARD A23.1.
3.2. THE PROJECT SUPERINTENDENT IS TO ADVISE THE STRUCTURAL CONSULTANT AT A MINIMUM OF 24 HOURS IN ADVANCE OF A CONCRETE POUR FOR A REVIEW OF PREPARATIONS.
3.3. STRUCTURAL STEEL - ROUTINE SHOP AND FIELD INSPECTION SHALL BE CARRIED OUT IN ACCORDANCE WITH CSA STANDARD S16.
3.4. SOILS, DECK - SEE STEEL DECK NOTES.
3.5. MASONRY - WHEN REQUIRED OR DIRECTED, CONCRETE BLOCKS SHALL BE TESTED IN ACCORDANCE WITH ASTM C140 BRICKS IN ACCORDANCE WITH CAN/CSA-A82 AND MORTAR AND/OR GROUT IN ACCORDANCE WITH CSA A179.
3.6. ALL INSPECTION AND TESTING SERVICES ARE TO BE PERFORMED BY COMPANIES CERTIFIED BY THE CANADIAN STANDARDS ASSOCIATION AND FOR WELDING, INSPECTORS ARE TO BE CERTIFIED BY THE CANADIAN WELDING BUREAU.
4. FOUNDATIONS:
4.1. REFER TO NOTES UNDER FOUNDATION PLANS. ALL EXTERIOR FOOTINGS OR OTHER FOOTINGS EXPOSED TO FREEZING SHALL BE PLACED AT LEAST 1.2m BELOW FINISHED GRADE UNLESS OTHERWISE NOTED. FOOTINGS FORMED IN TRENCHES SHALL HAVE VERTICAL SIDES. UNLESS FOOTINGS ARE POURED AGAINST UNDISTURBED SOIL, CONSTRUCTION SHALL BE PROTECTED BY A MINIMUM OF 100mm (4") OF EARTH OR ITS EQUIVALENT SUFFICIENT TO PREVENT FREEZING.
4.2. THE LINE OF SLOPE BETWEEN ADJACENT EXCAVATIONS FOR FOOTINGS OR ALONG STEPPED FOOTINGS SHALL NOT BE STEEPER THAN 1 VERTICAL TO 2 HORIZONTAL UNLESS OTHERWISE NOTED.
4.3. CAP SUITABLE ROCK EXCAVATION SHALL BE PERFORMED BY METHODS AND TECHNIQUES BASED UPON INFORMATION AVAILABLE AT THE TIME OF PREPARATION OF THE STRUCTURAL DRAWINGS.
4.4. IF ACTUAL JOB SITE OR SOIL CONDITIONS VARY FROM THOSE ASSUMED, THEN WRITTEN DIRECTIONS MUST BE OBTAINED FROM THE STRUCTURAL CONSULTANT BEFORE PROCEEDING WITH THE WORK.
4.5. ALL FOOTINGS SHALL BE PLACED ON UNDISTURBED NATURAL SOIL OR ON COMPACTED FILL APPROVED BY THE SOILS CONSULTANT. ALL LOOSE OR SOFT MATERIAL SHALL BE EXCAVATED TO THE SATISFACTION OF THE SOILS CONSULTANT AND THE FOOTINGS LOWERED TO SUIT.
5. BACKFILLING AND COMPACTION:
5.1. SLABS-ON-GRADE - AND ALL STRUCTURAL ELEMENTS FRAMING INTO WALLS WHICH RETAIN EARTH MUST BE BACKFILLED AS FOLLOWS:
5.1.1. FULL HEIGHT WALLS - WITH GRANULAR BATH SOILS, UNLESS ADEQUATELY HELD BY TEMPORARY BRACING, AND COMPACT EACH SIDE OF WALL SIMULTANEOUSLY.
5.1.2. UNDER SLAB-ON-GRADE, REMOVE SOFT SPOTS, ORGANIC AND FOREIGN MATTER IN THE SUB-GRADE. WHERE SUB-GRADE CONSISTS OF COMPACTED FILL, REFER TO SPECIFIC NOTES ON THE DRAWINGS.
5.1.3. ALL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATTER AND SHALL BE PLACED IN LAYERS AS CAREFULLY AS POSSIBLE UNLESS SPECIFICALLY NOTED OTHERWISE, BACKFILL SHALL BE CARRIED OUT A MINIMUM OF 200mm (8") INCH LIFTS OF LOOSE. FILL EACH COMPACTED TO A MINIMUM OF 95% STANDARD PROCTOR MAXIMUM DRY DENSITY.
5.2. UNLESS OTHERWISE NOTED, PROVIDE IMMEDIATELY UNDER SLABS-ON-GRADE A MINIMUM OF 200mm (8") OF GRANULAR FILL MATERIAL. GRANULAR MATERIAL, COMPACTED TO ACHIEVE A MINIMUM OF 98% STANDARD PROCTOR MAXIMUM DENSITY.

STANDARD ABBREVIATIONS T-A11

@	at	H (HOR)	Horizontal	PSL	Parallel Strand Lumber
A, B (TA,B,)	Anchor Bolt	HEF	Horizontal Each Face	PT	Pressure Treated
ABV	Above	HP	Asphalt Impregnated Fibre Board	R	Reaction
AFB	Asphalt Impregnated Fibre Board	HP	Horizontal Inside Face	RAD	Radius
ALT	Alternate	HOF	Horizontal Outside Face	REF	Reference
ARCH	Architectural	HSL	Horizontal Stability Slotted	REINF	Reinforced
ASL	Unbalanced Snow Loading	HSS	Connector	REQ'D	Required
B (BOT)	Bottom		Hollow Structural Section	REV	Revision/Revised
BEW	Bottom Each Way			RF	Factored Vertical Reaction
BF	Bottom Face	IF	Inside Face	RW	Reinforced Web
BLL	Bottom Lower Layer	INT	Interior	S	Standard Beam
BM	Beam	INV	Invert	SDD	Slotted Drain Footing
BML	Bottom Middle Layer			SHD	Superimposed Dead Load
BNT	Base Nominal Thickness	JT	Joint	SOL	Solid
BP	Baseplate	kg	Kilogram	SECT	Section
BSMT	Basement	kN-m	Kilo Newton Metres	SL	Slab
BUL	Bottom Upper Layer	kN/m²	Kilo Newton per Square Metre	SQ	Square
C	Standard Channel	kN/m	Kilo Newton per Linear Metre	SOG	Slab on Grade
CA	Column Above	kPa	Kilo Pascals	SPEC	Specifications
CANT	Cantilever			ST	Steel
CC (sc)	Centre to Centre	L	Angle	STD	Standard
CJ	Control Joint	LL	Live Load	STR	Straight
CL	Centreline	LLL	Long Lay	STRUCT	Structural
CLG	Compression Lap	LLL	Lower Lower / Lower Layer	T	Top
COL	Column	LLH	Long Leg Horizontal	TEMP	Temperature - Factored Tension
COMB	Combustible	LLV	Long Leg Vertical	TF	Top Flange
CONC	Concrete	LSG	Long Span Steel Joists	TJ	Top Joint
CONST	Construction	LVL	Laminated Veneer Lumber	TLL	Top Lower Layer
CONST JT (CJT)	Construction Joint			TNF	Factored Tensile Force
CONT (CONTIN)	Continuous		Moment		
CW	Complete Web	m	Metre	TML	Top Middle Layer
D.FIR	Douglas Fir	MAX	Maximum	TOC, TD	Top of Deck
DET	Detail	MECH	Mechanical	TOS, TS	Top of Slab
DIAG	Diagonal	MEZZ	Mezzanine	TOST	Top of Steel
DIM	Dimension	MIN	Minimum	TP	Tension per Square Foot
DIMEN	Dimension	MISC	Miscellaneous	TW	Top Web
DJ	Double Joist	ML	Middle Layer	TUL	Top Upper Layer
DL	Dead Load	MLL	Middle Lower Layer	TYPICAL	Typical
DO	Ditto	MM	Millimetre	TLS	Class B Tension Splice
DOWN	Downspout	MUL	Middle Upper Layer	U	Upper Layer
DWL	Dowel	MPa	Mega Pascals	UN	Unless Noted Otherwise
DT	Double Tee	MUL	Middle Upper Layer	UNO	Unless Noted Otherwise
E-W	East-West	N	Newton	UON	Underside of Deck
EA	Each	NAT	Natural Grade	US	Underside of Deck
EE	Each End	NF	Near Face	UTD	Underside of Deck
EF	Each Face	NIC	Not in Contract	V (VERT)	Vertical
ELECT	Electrical	NUM	Number	VIF	Verify on Site
ELEV (EL)	Elevation / Elevator	NTS	Not to Scale	VSF	Vertical Each Face
EN	Each Side			VSP	Vertical Inside Face
EO	Each Side	OWSJ	Open Web Steel Joist	VOF	Vertical Outside Face
EW	Each Way			VSS	Vertically Slotted Connection
EXIST	Existing	P	Pile	W	Wide Flange Beam
EXP. JT.	Expansion Joint	Pa	Pascal	WP	Wall Plate
EXT	Exterior	PC	Precast	WWF (WWM)	Welded Wire Fabric/ Mesh
FDN	Foundation	PL/F	Plate		
FF	Far Face		Pounds per Linear Foot		
FIN	Finished	PREL	Preliminary		
FL	Floor	PREFAB	Prefabricated		
FMC	Full Moment Connection	PSF	Pounds per Square Foot		
FT	First / Free	PSI	Pounds per Square Inch		
FTG	Footing				
GA	Gauge				
GALV	Galvanized				
GBN	Gobbin				

STRUCTURAL STEEL NOTES T-A03A

G.1.1 GENERAL
1.1. STRUCTURAL STEEL DESIGN DETAILS AND CONNECTIONS SHALL CONFORM TO CSA STANDARD S16 AND SHALL BE DESIGNED BY A LICENCED PROFESSIONAL ENGINEER EXPERIENCED IN THIS TYPE OF WORK.
1.2. REFER ALSO TO GENERAL NOTES; NOTES UNDER PLANS AND TO THE SPECIFICATION.
1.3. WELDING SHALL CONFORM TO CSA STANDARD W59 AND BE PERFORMED BY A FABRICATOR CERTIFIED TO CSA W47.1.
1.4. STRUCTURAL STEELWORK SHALL BE DESIGNED FOR A MINIMUM OF 50% OF THE BEAM SHEAR CAPACITY UNLESS OTHERWISE NOTED, AND IN NO CASE BE LESS THAN THE LOADS SHOWN ON OR IMPLIED BY THE DRAWINGS.
1.5. ALL STEEL OUTSIDE OF VAPOUR BARRIER SHALL BE HOT DIP GALVANIZED.
2. PRODUCTS
2.1. STRUCTURAL STEEL SHALL CONFORM TO CSA-G40.20/G40.21.
2.1.1. WF SHAPES, S SHAPES, CHANNELS, ANGLES, PLATES AND WOODS - GRADE 300 W
2.1.2. HSS SECTIONS - GRADE 350W (CLASS C,UR.)
2.1.3. W SHAPES - GRADE 350W
2.2. BOLTS, NUTS AND WASHERS FOR CONNECTIONS TO CONFORM TO ASTM A325 UNLESS NOTED OTHERWISE. USE HEAVY HEX NUTS AND WASHERS FOR BASE PLATES. BEARING AND WELD PLATES TO CONFORM TO ASTM A307 UNLESS NOTED.
2.3. SHEAR STUDS WHERE REQUIRED TO CONFORM TO ASTM A108. WELDING TO CONFORM TO CSA W59.
2.4. PRIMER PAINT TO CONFORM TO CAN/CGSB-1.40 OR CISC/CPMA 1 OR CISC/CPMA 2.
2.5. PRIMER PAINT TO BE COMPATIBLE WITH FIRE PROTECTIVE COATING OF 900g/m².
2.6. UNLESS OTHERWISE SPECIFIED.
3. EXECUTION
3.1. FABRICATION, HANDLING AND ERECTION TO CONFORM TO CAN / CSA - S16.
3.2. PROVIDE A MINIMUM OF 3.2 mm (1/7") DIAMETER BY 250 (10") LONG WELL ANCHORS FOR ALL BEAM BEARING PLATES. WELD ANCHORS TO BEARING PLATES. BEARING PLATES ARE TO BE WELDED TO BEAMS AND JOIST SHOES TO BEAM WELDS PLATES.
3.3. PROVIDE ADJUSTABLE ANCHORS TO ALL STEEL TO BE BUILT INTO, AFFIXED BY, OR FACED WITH MASONRY (REFER ALSO TO TYPICAL DETAILS IF SHOWN). SPACING TO BE:
FOR VERTICAL SPACING ..... 600 (24") MAX. CENTRES.
3.4. WHERE STEEL PROVIDES LATERAL BRACING ONLY TO MASONRY (I.E. DOES NOT SUPPORT MASONRY) ANCHORS SHALL PERMIT DIFFERENTIAL MOVEMENT BETWEEN STRUCTURAL MEMBERS AND MASONRY.
3.5. CLEAN, PREPARE SURFACES AND SHOP PRIME STRUCTURAL STEEL WITH ONE COAT OF PRIMER PAINT EXCEPT AT AREAS WHERE FIELD WELDING IS REQUIRED. SURFACES ARE TO BE ENCASED IN CONCRETE, OR TO RECEIVE SPRAY-APPLIED FIRE PROOFING, FIELD TOUCH-UP BOLTS, WELDS, BURNED OR SCRAPED SURFACES AFTER ERECTION.
3.6. PROVIDE ALL NECESSARY TEMPORARY BRACING TO KEEP STRUCTURE SAFE AND PLUMB. BRACING SHOWN ON STRUCTURAL DRAWINGS IS PERMANENT FOR FINISHED BUILDING ONLY.
3.7. DETAILING, FABRICATION AND ERECTION OF THE STEEL STRUCTURE SHALL CONFORM TO THE DETAILING, FABRICATION AND ERECTION OF THE STEEL STRUCTURE.
3.8. TOLERANCES: VARIATION FROM PLUMB AND LEVEL EXTERIOR COLUMNS, COLUMNS AT ELEVATOR SHAFTS, AND SPANDREL BEAMS INCLUDING ANGLES ..... 1:500 MM. 25 mm (1") MAX. OTHER PIECES ..... 1:500 (1/4" in 10'-0")
3.9. ERECTION TOLERANCE SHALL NOT EXCEED THOSE SHOWN IN CISC SHOP DRAWINGS SHALL BE MADE IN ANY STEEL MEMBER WITHOUT WRITTEN PERMISSION OF THE STRUCTURAL CONSULTANT.
4. QUALITY CONTROL
4.1. SEE GENERAL NOTES, NOTES UNDER PLANS, AND/OR SPECIFICATION FOR INSPECTION AND TESTING REQUIREMENTS.

CAST-IN-PLACE CONCRETE NOTES T-A02

1. GENERAL
1.1. PROVIDE ALL LABOUR, MATERIALS, TOOLS AND EQUIPMENT REQUIRED TO CARRY OUT THE WORK.
1.2. REFER TO GENERAL NOTES, NOTES UNDER PLANS AND TYPICAL DETAILS AND SPECIFICATION.
2. PRODUCTS
2.1. PORTLAND CEMENT, WATER AND AGGREGATES SHALL CONFORM TO CSA STANDARD A23.1.
2.2. PROVIDE AN APPROVED WATER REDUCING ADDITIVE IN ALL CONCRETE. PROVIDE AN APPROVED AIR ENTRAINING ADMIXTURE IN ALL CONCRETE EXPOSED TO THE WEATHER. DO NOT USE CALCIUM CHLORIDE THE ACTION OF OR USING SALT, ADMIXTURES SHALL CONFORM TO CSA STANDARD A23.1.
2.3. FORMWORK SHALL CONFORM TO CSA STANDARD A23.1, CSA STANDARD S269.3 AND FALSEWORK SHALL CONFORM TO CSA S269.1.
2.4. IF SO INSTRUCTED, THE DESIGN FOR THE FORMWORK SHALL BE SUBMITTED FOR REVIEW BEFORE CONSTRUCTION. FORMWORK DRAWINGS AND DESIGN SHALL BE SEALED BY A LICENSED PROFESSIONAL ENGINEER.
2.5. UNLESS OTHERWISE NOTED PROVIDE SLAB AND BEAM FORMS WITH AN UPWARD CAMBER OF 2 mm/1000 mm (1/4" PER 10'-0") OF SPAN AND ON ALL CANTILEVERED SLAB AND BEAM FORMS 3 mm/1000 mm (1/4" PER 8'-0") OF CANTILEVER LENGTH.
2.6. PROVIDE CHAMFERED ADJUSTABLE MASONRY ANCHOR SLOTS FOR ALL MASONRY FACING OR ABUTTING CONCRETE FACES.
2.7. PROVIDE ANCHOR INSTALL STANDARD ADJUSTABLE INSERTS AND ALL OTHER CAST IN INSERTS AS REQUIRED BY THE ARCHITECTURAL, STRUCTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND SPECIFICATION.
2.8. REINFORCING STEEL UNLESS SPECIFICALLY NOTED, SHALL BE DEFORMED BARS CONFORMING TO CSA STANDARD G30.18 GRADE 400 MPa.
2.9. WELDED WIRE FABRIC TO BE SUPPLIED IN FLAT SHEETS ONLY. UNLESS APPROVED OTHERWISE.
2.10. REINFORCING SHALL BE DETAILED, BENT, PLACED AND SUPPORTED TO CONFORM TO ACI DETAILING MANUAL AND THE MANUAL OF STANDARD PRACTICE FOR DETAILING OF THE REINFORCING STEEL INSTITUTE OF CANADA.
2.11. NON-SHRINK GROUT TO BE 1 PART PORTLAND CEMENT TO 1 1/2 PARTS SAND TO 2 PARTS OF 6 mm PEA GRAVEL WITH ONLY SUFFICIENT WATER TO DAMPEN MIXTURE, COMPRESSIVE STRENGTH 50MPA AT 28 DAYS.
2.12. NON-SHRINK GROUT TO BE AN APPROVED PRE-MIXED PROPRIETARY PRODUCT.
2.13. PROVIDE APPROVED EXTRUDED PVC WATERSTOPS AT ALL CONSTRUCTION JOINTS IN CONCRETE WALLS AND FOOTINGS AND FOR CONNECTIONS BETWEEN PRE-CAST AND PRE-MELDED CORNERS AND INTERSECTIONS. SEE ALSO TYPICAL DETAILS.
2.14. CONCRETE MIX DESIGN SHALL BE BASED ON THE EXPOSURE CLASS AS SET TO CONFORM TO ASTM STANDARD C309. GENERALLY ALL CONCRETE SURFACES ARE TO BE SEALED UNLESS NOTED OTHERWISE. COMPOUNDS ARE TO BE COMPATIBLE WITH APPLIED FINISHES.
2.15. SHEAR REINFORCEMENT AT SLAB CONNECTION AS SHOWN ON DRAWINGS AND DETAILS. SHALL BE STUDRAILS® AS MANUFACTURED BY DECON®. THE COMPLETE AND FINISHED STUDRAILS® SHALL BE SUPPLIED BY A CERTIFIED STUDRAILS® PLANT. THE STUDRAILS® SHALL BE MANUFACTURED IN A CERTIFIED STUDRAILS® SHALL CONFORM TO THE LATEST EDITION OF ICC-ES AC310.
3. EXECUTION
3.1. MINIMUM COMPRESSIVE STRENGTH FOR CONCRETE @ 28 DAYS SHALL BE AS NOTED ON THE DRAWINGS (28MPa MINIMUM).
3.2. LOCATION OF POINT OF DISCHARGE SHALL BE CONSISTENT AT 80 mm ±30mm (3" ±1 1/8") UNLESS NOTED OTHERWISE GREATER SLUMPS ARE NOT ALLOWED.
3.3. CONCRETE MIXING, TRANSPORTATION, HANDLING AND PLACING SHALL CONFORM TO CSA STANDARD A23.1.
3.4. CONSTRUCTION JOINTS FOR WALLS ARE BASED UPON VERTICAL JOINTS AT A MAXIMUM SPACING OF 10000 mm (32'-0"). UNLESS CONTROL JOINTS ARE PROVIDED AS PER DETAIL 17 C16. TOTAL LENGTH OF POUR BETWEEN CONSTRUCTION JOINTS SHALL NOT EXCEED 30m.
3.5. CONSTRUCTION JOINTS FOR WALLS, SLABS, AND BEAMS NOT SHOWN ON THE DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL CONSULTANT BEFORE CONSTRUCTION. GENERALLY JOINTS IN SLABS SHALL BE AT RIGHT ANGLES TO THE SPANS, AT MID-SPAN IF POSSIBLE AND BE CLEAR OF SUPPORTS AND FIXING.
3.6. INSERTS, PRIME OUTS, SLEEVES, BRACKETS, CONDUITS AND FASTENING DEVICES, SHALL BE INSTALLED AS REQUIRED BY THE DRAWINGS AND SPECIFICATIONS IN A MANNER THAT WILL NOT IMPAIR THE STRUCTURAL STRENGTH OF THE SYSTEM. BE SO INSTALLED THAT THEY SHALL NOT REQUIRE THE CUTTING, BENDING, OR DISPLACEMENT OF THE REINFORCING STEEL FROM THE POSITIONS SHOWN ON THE DRAWINGS.
3.7. ELECTRICAL SCHEDULE 40 PVC CONDUITS PASS THROUGH CONCRETE SLABS SHALL BE OF OUTSIDE DIAMETER LESS THAN 1/3 OF THE SLAB THICKNESS AND SHALL BE PLACED BETWEEN TOP AND BOTTOM REINFORCEMENT. CONDUITS LARGER THAN 3 DIAMETERS ON CENTRE UNLESS APPROVED. HAVE A MINIMUM CONCRETE COVER OF 25 mm (1") AND UNLESS SPECIFICALLY PERMITTED OTHERWISE, SHALL NOT RUN HORIZONTALLY IN A CONCRETE WALL.
3.8. OPENINGS AND OPENINGS FOR PIPES, DUCTS AND CONDUITS THROUGH CONCRETE IS PLACED SHALL BE APPROVED BY THE STRUCTURAL CONSULTANT BEFORE PROCEEDING.
3.9. FINISH OF CONCRETE SURFACES SHALL BE AS SPECIFIED ON THE DRAWINGS FOR REQUIRED FINISH TO EXPOSED CONCRETE ALL HONEYCOMBING SHALL BE CUT OUT TO SOUND CONCRETE. FLOOR FINISHES SHALL BE AS REQUIRED BY THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS AND SHALL CONFORM TO CSA STANDARD A23.1 CONCRETE FINISHES AND FINISHING METHODS.
3.10. TOLERANCES FOR FLOOR SLOTS CONSTRUCTION, FORMWORK, PRESTRESSING STEEL, CAST-IN-HARDWARE AND FOR FLOOR AND ROOF FINISHES SHALL BE AS SPECIFIED IN CSA STANDARD A23.1.
3.11. MINIMUM REINFORCING FOR ANY CONCRETE WALL TO BE AS SHOWN ON TYPICAL DETAILS FOR CONCRETE WALLS.
3.12. MINIMUM REINFORCING FOR ANY SUSPENDED SLAB SHALL BE TEMPERATURE BARS BOTTOM EACH WAY PLUS ADDITIONAL BARS OVERLS 6-M30M (1-0" x 2-0") TOP AROUND PERIMETER. REFER TO TYPICAL DETAIL OF ONE WAY SLABS.
4. QUALITY CONTROL
4.1. FOR INSPECTION AND TESTING, SEE GENERAL NOTES AND/OR SPECIFICATION.

SHORING NOTES T-A13

1.0 DESIGN
1.1. REFER TO 2007 SOILS REPORT #WHA-0001976-023 DATED NOVEMBER 29, 2013 BY DESIGN PARAMETERS.
1.2. TIE-BACK DESIGN BASED ON AN ANCHORAGE CONSISTENT W/ N-1 BAND THE CANADIAN FOUNDATION REINFORCEMENT MANUAL 3rd EDITION.
1.3. DESIGN COMPLIES WITH THE LATEST EDITIONS OF THE ONTARIO BUILDING CODE AND THE ONTARIO HIGHWAY BRIDGE DESIGN CODE.
2.0 MATERIALS
2.1. STRUCTURAL STEEL SHALL BE NEW OR SOUND USED MATERIAL CONFORMING TO C.S.A. G40.21, GRADE 300.
2.2. ALTERNATIVE SECTIONS OR GRADES OF EQUIVALENT STRENGTH, MAY BE SUBSTITUTED SUBJECT TO APPROVAL OF ENGINEER
2.3 CONCRETE STRENGTHS TO BE AS FOLLOWS:
a) FOOTINGS ..... 20 MPa
b) CANTILEVERED PILE TOES ..... 20 MPa
c) BACKFILL TO GRADE ..... 4 MPa
d) FREE ZONES ..... SEE CONCRETE SLAB
2.4 TIMBER LAGGING TO BE 75 mm MIN. (TRUE THICKNESS) HARDWOOD UNLESS NOTED OTHERWISE.
2.5 ALL STEEL SHALL CONFORM TO C.S.A. W59 AND BE PERFORMED BY QUALIFIED WELDERS.
3.0 PROCEDURES
3.1 DRILL HOLES TO SIZE AND DEPTHS INDICATED. INSTALL PILES PLUMB AND TO LINE.
3.2 FOR SPECIFIED CONCRETE TOES AND BACKS, SEE DETAILS.
3.3 EXCAVATE IN 1200mm MAX. LIFTS INSTALLING LAGGING AS EXCAVATION PROCEEDS. CUT SOIL FACES NEATLY AND TRIM BACK ANY LOOSE OR OVERHANGING MATERIAL. MEDGE AT PILES AS REQUIRED. FILL VOIDS BEHIND LAGGING WITH GRANULAR FILL RAMMED IN PLACE.
3.4 WHEN EXCAVATION REACHES 300mm MAX. BELOW TIE-BACK ELEVATIONS NOTED, DRILL AND INSTALL TIE-BACKS. LOCK-IN LOADS BEFORE EXCAVATING BELOW.
3.5 PROVIDE 2 TEST TIE-BACKS. TEST TO BE IN ACCORDANCE WITH THE LATEST EDITION OF THE CANADIAN FOUNDATION REINFORCEMENT MANUAL. TEST LOAD TO BE 1.5 TIMES DESIGN LOAD AND HOLD FOR 10 MINUTES.
3.6 PROOF TEST REMAINING TIE-BACKS TO 1.3 TIMES DESIGN LOAD AND HOLD LOAD FOR 10 MINUTES. IF NO APPRECIABLE CREEP OCCURS, REDUCE LOAD TO 1 TIMES DESIGN LOAD AND LOCK-IN.
4.0 GENERAL CONTRACTOR
4.1 GENERAL CONTRACTOR TO PROVIDE THE FOLLOWING:
a) ADEQUATE SPACE FOR ERECTION OF SHORING & STRUCT. ENGG. PRIOR TO PILE INSTALLATION.
b) LAYOUT STAKED FOR ALL PILES.
c) ADEQUATE PROTECTION TO SHORING SYSTEM AGAINST THE EFFECTS OF WEATHER, WATER OR FREEZING.
d) MONITORING OF ALL PILES FOR ANY LATERAL MOVEMENTS. REPORT ANY MOVEMENT GREATER THAN 10mm IMMEDIATELY TO ENGINEER.
e) LOCATE AND IDENTIFY ALL BELOW AND ABOVE GROUND SERVICES OR UTILITIES THAT MAY INTERFERE WITH PILE INSTALLATION. REMOVE OR RELOCATE AS NECESSARY.
f) DEWATER SITE AS NECESSARY.

LINTEL NOTES T-A06

UNLESS OTHERWISE SHOWN OR NOTED ON THE STRUCTURAL DRAWINGS, PROVIDE LINTELS OVER ALL OPENINGS IN MASONRY WALLS AS FOLLOWS:
1. FOR OPENINGS UP TO 1200 mm (4') CLEAR:
1.1. ONE ANGLE 90 x 90 x 6.1 (3 1/2" x 3 1/2") FOR EACH 100mm (4") OF WALL THICKNESS OR PORTION THEREOF.
2. FOR OPENINGS FROM 1200mm (4') CLEAR TO 1800mm (6'-0") CLEAR:
2.1. ONE ANGLE 125 x 90 x 8 LONG LEG VERTICAL (5"x 3 1/2" x 5/16") FOR EACH 100mm (4") OF WALL THICKNESS OR PORTION THEREOF.
2.2. 200mm (8") DEEP MASONRY LINTEL BLOCK REINFORCED WITH 1-15M BOTTOM FOR EACH 100mm (4") OF WALL THICKNESS OR PORTION THEREOF.
3. ALL LINTELS TO BEAR 200mm (8") MINIMUM AT EACH END ON SOLID MASONRY, UNLESS SHOWN OTHERWISE.
4. ALL ANGLE LINTELS ARE TO BE BOLTED OR WELDED TOGETHER, PRIOR TO ASSEMBLY, AT MAXIMUM 900mm (36") CENTRES.
5. MASONRY LINTEL BLOCKS MAY ONLY BE USED IN LOAD-BEARING WALLS WITH PERMISSION AND MUST BE FILLED WITH CONCRETE. MORTAR IS NOT ACCEPTABLE AND WILL BE REJECTED.
6. STEEL LINTELS ARE TO BE SUPPLIED BY STEEL CONTRACTOR BUT PLACED BY GENERAL CONTRACTOR OR MASONRY CONTRACTOR.
7. STEEL CONTRACTOR TO SUPPLY ALL NECESSARY DIRECTIONS REQUIRED FOR PLACING STEEL LINTELS.
8. WHILE EVERY EFFORT HAS BEEN MADE TO SHOW ON THE STRUCTURAL DRAWINGS EACH AND EVERY LINTEL OVER DOORS, MECHANICAL AND ELECTRICAL SERVICES, STOREFRONTS AND POCKETS ETC., THROUGH MASONRY WALLS, IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO CO-ORDINATE AND SUPPLY ALL LINTELS REQUIRED. ANY LINTELS REQUIRED BUT NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL CONSULTANT BEFORE CONSTRUCTION.

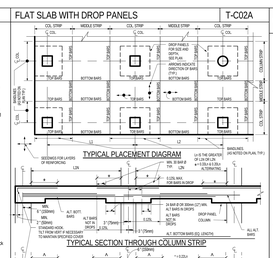
COLD WEATHER CONCRETING T-A14

FOR FORECASTED AIR TEMPERATURES AT OR BELOW 5°C

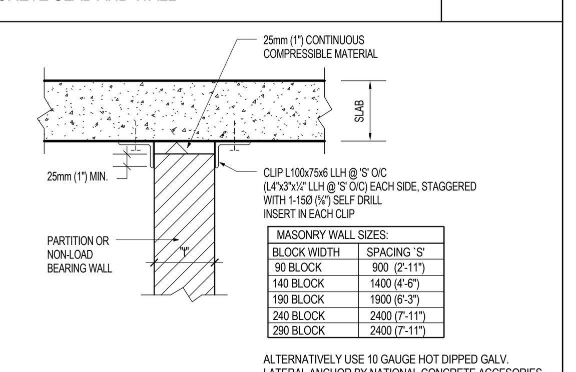
1. ALL MATERIALS AND EQUIPMENT NEEDED FOR HEATED ENCLOSURE AND CONCRETE CURING PROTECTION SHALL BE ON THE JOB SITE PRIOR TO THE START OF CONCRETE PLACEMENT IS STARTED.
2. AIR TEMPERATURE INSIDE ENCLOSURE IS TO BE HEATED TO A MINIMUM OF 5°C. CONCRETE SHALL NOT BE PLACED ON ANY SURFACE BELOW 5°C.
3. FORMS AND/OR STEEL DECK MUST BE FREE OF SNOW AND ICE PRIOR TO PLACING CONCRETE. THE USE OF ANY DE-ICING CHEMICAL IS NOT PERMITTED ON THE FORMWORK OR STEEL DECK.
4. THE TEMPERATURE OF CONCRETE AS PLACED SHALL BE MAINTAINED BETWEEN 10°C AND 25°C. TEMPERATURE OF CONCRETE AT THE TIME OF POURING IS TO BE 10°C. FOR SLABS LESS THAN 1000mm THICK THE MINIMUM CONCRETE TEMPERATURE AT TIME OF POURING IS TO BE 5°C.
5. CONCRETE TEMPERATURE SHALL BE KEPT AT A MINIMUM OF 10°C FOR AT LEAST 3 DAYS OR UNTIL THE CONCRETE REACHES 40% OF THE SPECIFIED STRENGTH. ADDITIONAL CURING MAY BE REQUIRED IN WHICH CASE THE CONCRETE SHALL BE KEPT ABOVE A MINIMUM TEMPERATURE OF 10°C FOR AT LEAST 7 DAYS OR UNTIL THE CONCRETE REACHES 70% OF THE SPECIFIED STRENGTH.
6. IF THE FORECASTED AIR TEMPERATURE IS TO FALL BELOW 0°C, THE TOP OF SLAB SHALL BE PROTECTED FROM FREEZING. TOP OF SLAB PROTECTION SHALL BE PLACED AS TO NOT AFFECT THE FINISH OF SLAB.
7. IF THE FORECASTED AIR TEMPERATURE IS TO FALL BELOW 0°C WITH REASONABLY HIGH WINDS THE CONCRETE SHALL BE PROTECTED FROM WIND. THE MINIMUM TEMPERATURE OF THE CONCRETE AND ITS PROTECTION MUST REMAIN IN PLACE UNTIL TOP OF SLAB PROTECTION HAS BEEN INSTALLED AS PER NOTE #6.

* SEE ALSO CSA A23.1-09/A23.2-09 CLAUSES 7.4.1.5 AND 7.4.2 FOR ADDITIONAL INFORMATION WITH RESPECT TO COLD WEATHER CONCRETING AND CURING.

FLAT SLAB WITH DROP PANELS T-C02A

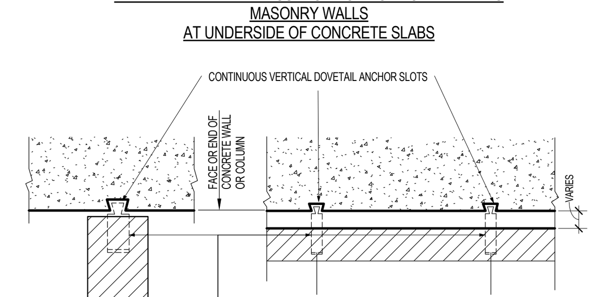


DETAILS OF NON LOAD-BEARING MASONRY WALLS AT CONCRETE SLAB AND WALL — T-C29



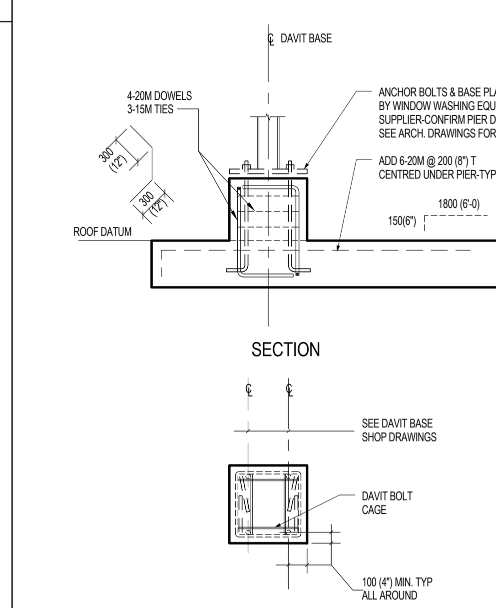
MASONRY WALL SIZES:	
BLOCK WIDTH	SPACING `S'
90 BLOCK	900 (2'-11")
140 BLOCK	1400 (4'-6")
190 BLOCK	1900 (6'-3")
240 BLOCK	2400 (7'-11")
290 BLOCK	2400 (7'-11")

ALTERNATIVE LATERAL SUPPORT OF NON LOAD-BEARING MASONRY WALLS AT UNDERSIDE OF CONCRETE SLABS



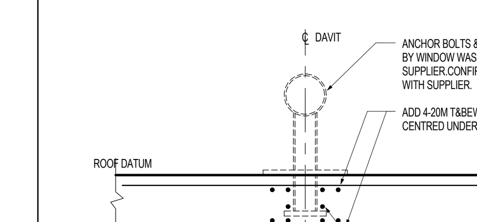
MASONRY WALL BUTTING CONCRETE — MASONRY FACING CONCRETE

TYPICAL WINDOW WASHING DAVIT BASE DETAIL — T-C30

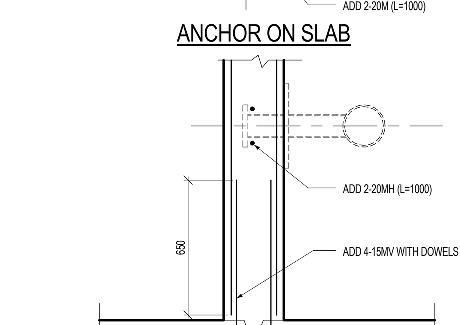


TYPICAL WINDOW WASHING DAVIT BASE DETAIL

TYPICAL ADDITIONAL REINFORCING AT WINDOW WASHING DAVITS — T-C32



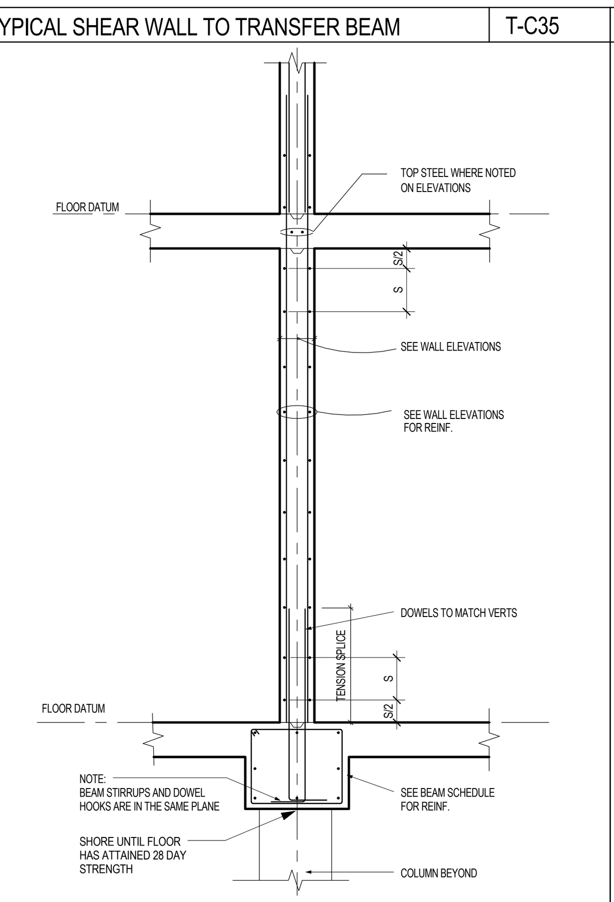
ANCHOR ON SLAB



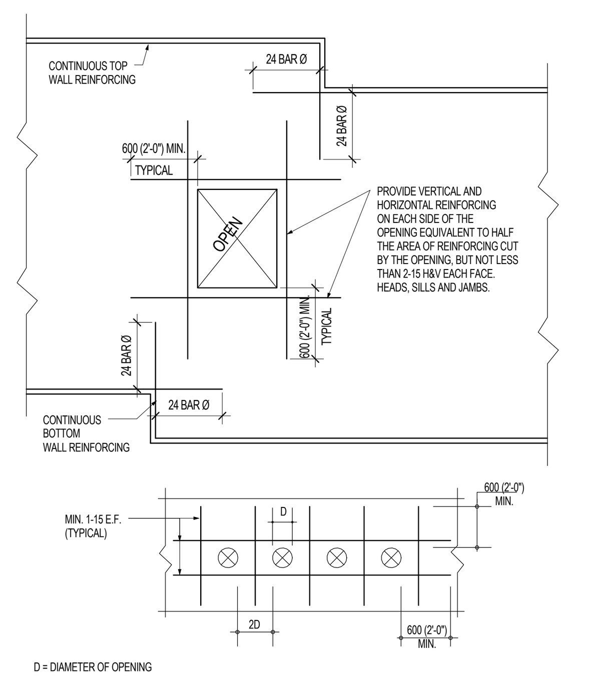
ANCHOR IN PENTHOUSE WALLS

TYPICAL ADDITIONAL REINFORCING AT WINDOW WASHING DAVITS

TYPICAL SHEAR WALL TO TRANSFER BEAM — T-C35



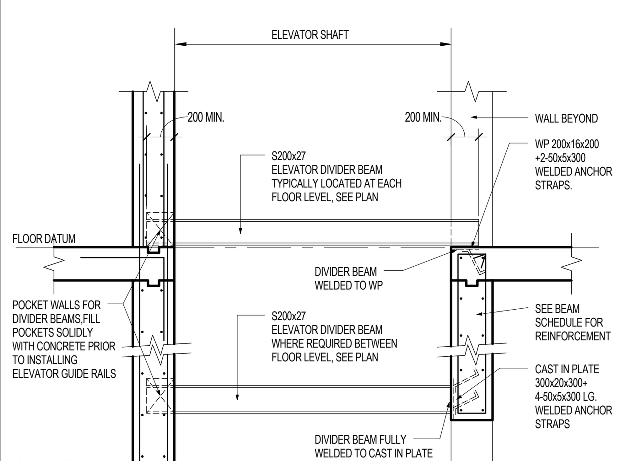
TYPICAL DETAIL OF OPENINGS IN CONCRETE WALL — T-C36



TYPICAL DETAIL OF ADDITIONAL REINFORCEMENT AT WALL OPENINGS

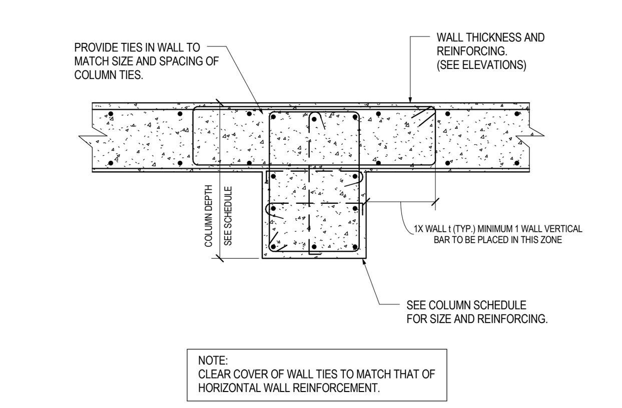
(TYPICAL UNLESS OTHERWISE SHOWN ON DRAWINGS)

TYPICAL ELEVATOR DIVIDER BEAM DETAIL — T-C37



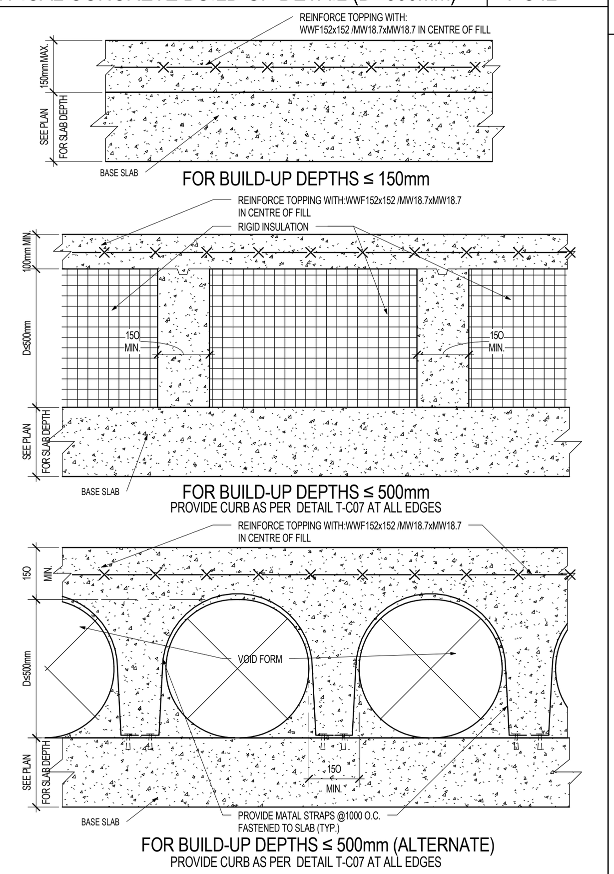
ELEVATOR DIVIDER BEAM ANCHORAGE DETAILS

COLUMN CAST INTEGRALLY WITH SHEAR WALLS — T-C41

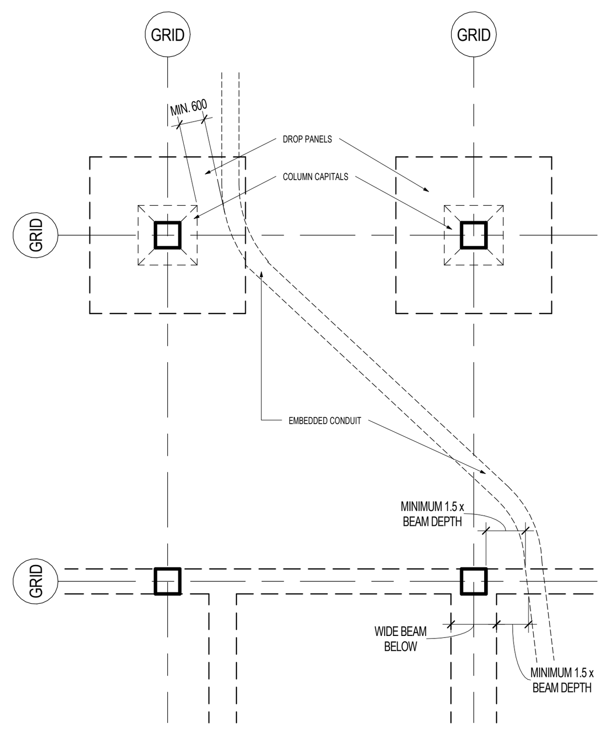


NOTE:
CLEAR COVER OF WALL TIES TO MATCH THAT OF HORIZONTAL WALL REINFORCEMENT.

TYPICAL CONCRETE BUILD-UP DETAIL (D≤500mm) — T-C42

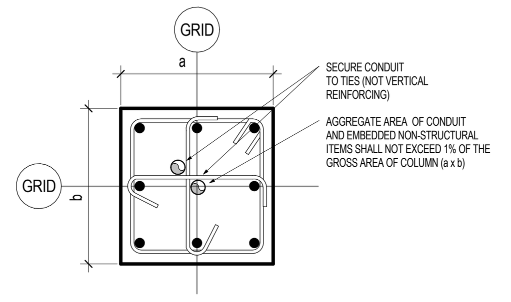


EMBEDDED CONDUITS IN SLAB — T-C43

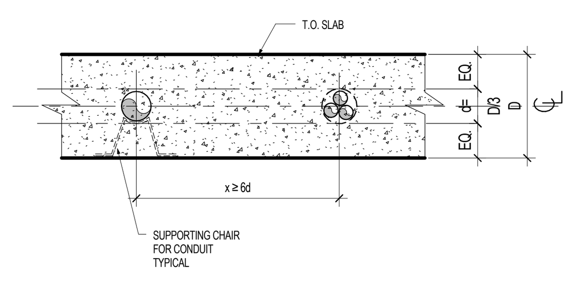


PART PLAN

EMBEDDED CONDUITS AND PIPES — T-C44



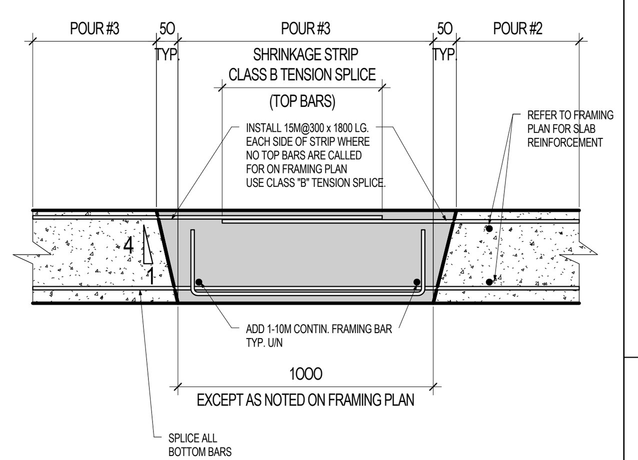
PLAN
CONDUITS EMBEDDED IN COLUMN



SECTION
CONDUITS EMBEDDED IN SLAB

*EXCEPT AS NOTED ABOVE, REFER TO SECTION 6.7.5. OF A23.1-09 FOR ADDITIONAL REQUIREMENTS FOR CONDUITS IN CONCRETE.

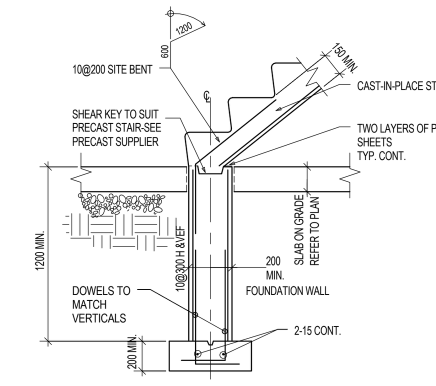
SHRINKAGE STRIP DETAIL — T-C45



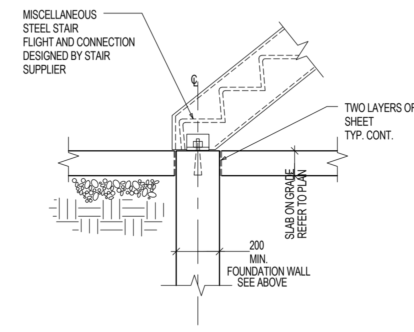
NOTES:

1. KEEP EDGE OF FLOORS AND ADJACENT BAYS SHORED UNTIL STRIP IS POURED AND CONCRETE HAS ATTAINED SPECIFIED STRENGTH.
2. POUR SHRINKAGE STRIP AT CONSULTANT SPECIFIED BAYS AFTER THE ADJOINING SEGMENTS OF SLABS HAVE BEEN CONSTRUCTED OR AS OTHERWISE NOTED IN SPECIFICATION.
3. PROVIDE VERTICAL EXPANSION JOINTS IN WALLS THAT INTERSECT SEPARATION STRIP.

SUPPORT FOR EXTERIOR STAIRS AT GRADE — T-C50

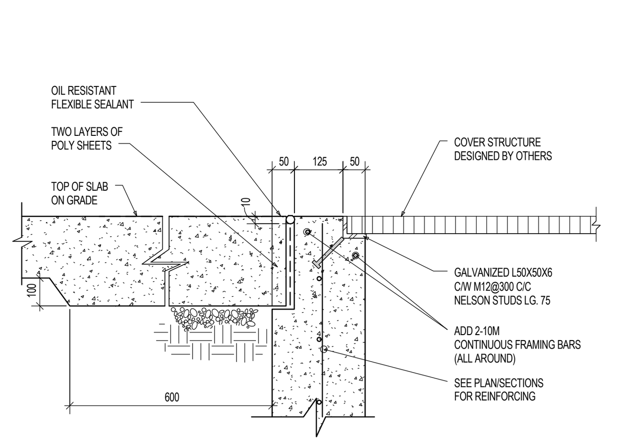


SUPPORT FOR STAIRS AT GRADE
FOR CAST-IN-PLACE/PRECAST STAIRS



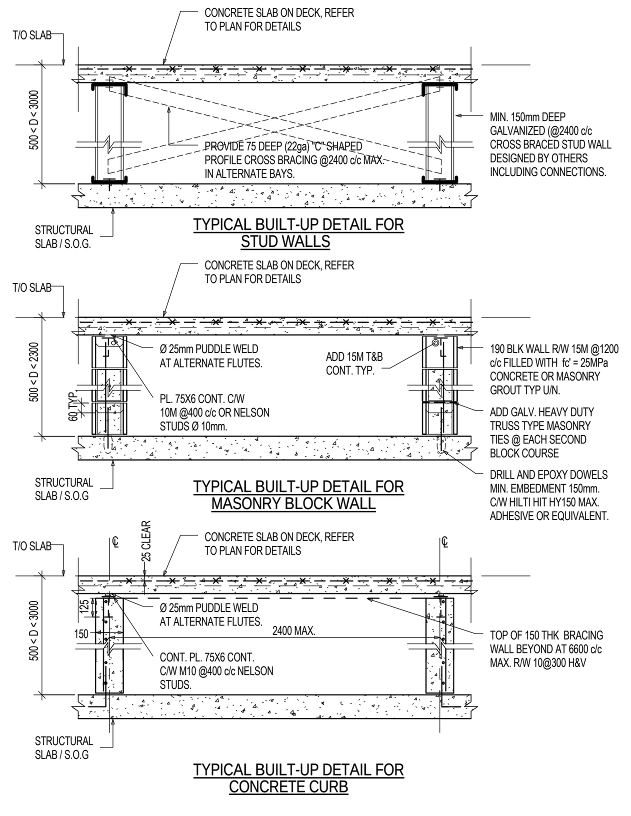
SUPPORT FOR STAIRS AT GRADE
FOR STEEL STAIRS

TYPICAL SUMP PIT DETAIL — T-C51

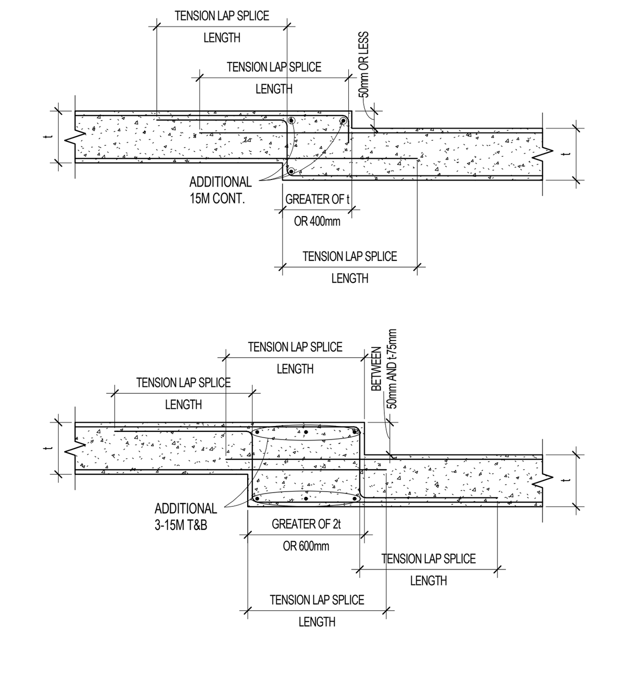


TYPICAL SUMP PIT DETAIL

BUILT-UP FLOOR DETAILS - CONCRETE ON DECK (FILL DEPTH (D) > 500mm) — T-C52



MIDSPAN SLAB DEPRESSION REINFORCING DETAILS — T-C53

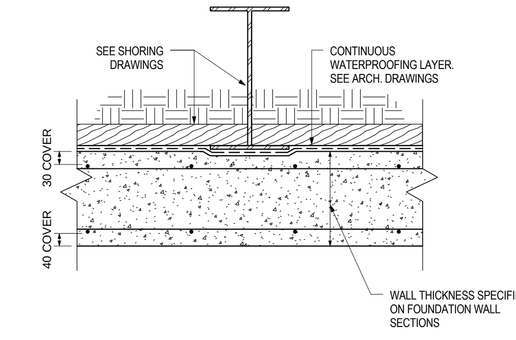


MIDSPAN SLAB DEPRESSION REINFORCING DETAILS

NOTE: FOR SLAB DEPRESSIONS GREATER THAN "t-75mm" SEE SECTIONS.

NON-LOAD BEARING BLOCK WALL LINTELS — T-M01

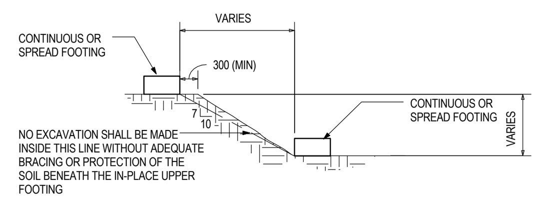
TYPICAL FOUNDATION WALL AGAINST SHORING — T-F02



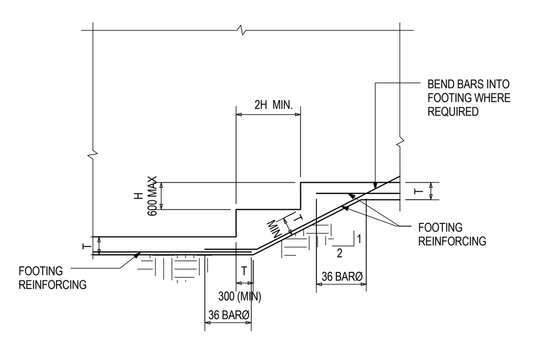
TYPICAL DETAIL OF FOUNDATION WALL AGAINST SHORING

SEE ALSO 1T-C06 FOR CONSTRUCTION JOINT DETAIL AND SPACING

TYPICAL STEPPED FOOTING DETAILS — T-F09

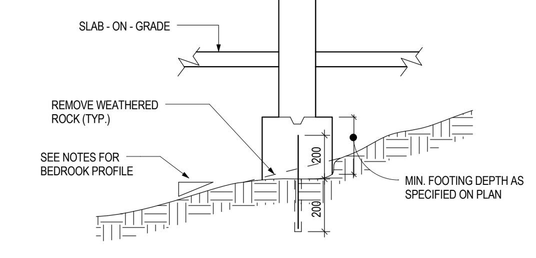


TYPICAL DETAIL OF SLOPE BETWEEN FOOTING AND ADJACENT CONSTRUCTION

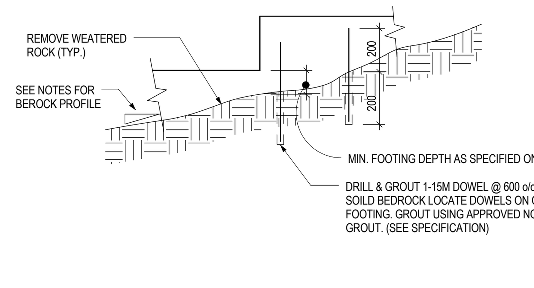


TYPICAL DETAIL OF STEPPED WALL FOOTING

TYPICAL FOOTING ON BEDROCK — T-F10



TYPICAL SECTION THROUGH WALL FOOTINGS



TYPICAL ELEVATION OF FOOTING FOR FOUNDATION WALLS

NOTES:
FOR BEDROCK PROFILE UP TO 1 IN 4 - NO DOWELS REQUIRED
FOR BEDROCK PROFILE FROM 1 IN 4 TO 1 IN 2 - PROVIDE DOWELS
FOR BEDROCK PROFILE GREATER THAN 1 IN 2 - CUT ROCK TO MAX. 1 IN 2.

COMPRESSION-TENSION DEVELOPMENT AND LAP LENGTHS Fy = 400 MPa — T-C56A

NOTES
1. STANDARD ABBREVIATIONS ON PLANS AND SCHEDULES SHOULD BE AS FOLLOWS
CLS - COMPRESSION LAP SPLICE
CDL - COMPRESSION DEVELOPMENT LENGTH
HEL - HOOK EMBEDMENT LENGTH

COMPRESSION LAP SPLICE AND DEVELOPMENT LENGTHS (Fy = 400 MPa)

CLS: COMPRESSION LAP SPLICE LENGTH

UNCOATED BLACK BAR							
10M	15M	20M	25M	30M	35M	45M	55M
330	470	570	740	880	1050	1280	1650

CDL: COMPRESSION DEVELOPMENT LENGTH

fc'	UNCOATED BLACK BAR							
	10M	15M	20M	25M	30M	35M	45M	55M
20MPa	250	340	420	540	640	770	940	1210
25MPa	220	310	380	480	570	690	840	1080
30MPa	200	280	340	440	530	630	770	990
35MPa	200	280	340	440	530	630	770	990
40MPa	200	280	340	440	530	630	770	990
> 40 MPa	SEE MINUMUM VALUES IN ROW ABOVE							

NOTES:
1. IF BUNDLED BARS ARE USED THE VAULES IN THE TABLES MUST BE INCREASED:
a. MULTIPLY BY 1.1 (TWO BAR BUNDLES)
b. MULTIPLY BY 1.2 (THREE BAR BUNDLES)
c. MULTIPLY BY 1.33 (FOUR BAR BUNDLES)

HEL: STANDARD HOOK TENSION EMBEDMENT LENGTHS

fc'	UNCOATED BLACK BAR					
	10M	15M	20M	25M	30M	35M
20MPa	260	360	440	570	670	800
25MPa	230	320	390	510	600	720
30MPa	210	300	360	470	550	660
35MPa	200	280	330	430	510	610
40MPa	180	260	310	400	480	570
45MPa	170	240	300	380	450	540
50MPa	160	230	280	360	430	510
55MPa	160	220	270	340	410	490

NOTES:
1. FOR EPOXY COATED BARS THE VALUES IN THE TABLES MUST BE INCREASED:
a. MULTIPLY BY 1.2 (WHEN CLEAR COVER GREATER THAN 3 X BAR DIAMETER AND CLEAR SPACING GREATER THAN 6 X BAR DIAMETER)
b. MULTIPLY BY 1.5 (WHEN COVER OR SPACING ARE LESS THAN ABOVE)
2. VALUES PROVIDED ARE BASED ON NORMAL WEIGHT CONCRETE AND MUST BE INCREASED FOR LIGHTWEIGHT CONCRETES:
a. MULTIPLY BY 1.2 (FOR SEMI-LOW DENSITY CONCRETE)
b. MULTIPLY BY 1.3 (FOR LOW-DENSITY CONCRETE)

TENSION DEVELOPMENT AND LAP LENGTHS Fy = 400 MPa — T-C56B

NOTES
1. STANDARD ABBREVIATIONS ON PLANS AND SCHEDULES SHOULD BE AS FOLLOW
TLS - TENSION LAP SPLICE
TDL - TENSION DEVELOPMENT LENGTH

TENSION LAP SPLICE AND DEVELOPMENT LENGTHS (Fy = 400 MPa)

TLS: TENSION LAP SPLICE LENGTH (CLASS B)

fc'	UNCOATED BLACK BAR											
	10M		15M		20M		25M		30M		35M	
	Top	Bottom	Top	Bottom	Top	Bottom	Top	Bottom	Top	Bottom	Top	Bottom
20MPa	620	480	880	670	1070	820	1720	1320	2040	1560	2430	1870
25MPa	550	430	780	600	950	740	1530	1180	1820	1400	2170	1680
30MPa	510	390	720	550	870	670	1400	1080	1660	1280	1980	1530
35MPa	470	360	660	510	810	620	1300	1000	1540	1190	1840	1420
40MPa	440	340	620	480	760	580	1210	940	1440	1110	1720	1330
45MPa	410	320	590	450	710	550	1140	880	1360	1050	1620	1250
50MPa	390	300	560	430	680	520	1090	840	1290	990	1540	1190
55MPa	380	300	530	410	640	500	1030	800	1230	950	1470	1130
60MPa	360	300	510	390	620	480	990	770	1180	910	1400	1080
65MPa	350	300	490	380	590	460	950	740	1130	870	1350	1040

TDL: TENSION DEVELOPMENT LENGTH

fc'	UNCOATED BLACK BAR											
	10M		15M		20M		25M		30M		35M	
	Top	Bottom	Top	Bottom	Top	Bottom	Top	Bottom	Top	Bottom	Top	Bottom
20MPa	480	360	670	520	820	630	1320	1020	1560	1210	1870	1440
25MPa	430	330	600	470	740	570	1180	910	1400	1080	1680	1290
30MPa	390	300	550	430	670	520	1080	830	1280	990	1530	1180
35MPa	360	300	510	390	620	480	1000	770	1190	910	1420	1090
40MPa	340	300	480	370	580	450	940	720	1110	860	1330	1020
45MPa	320	300	450	350	550	420	880	680	1050	810	1250	960
50MPa	300	300	430	330	520	400	840	650	990	770	1190	910
55MPa	300	300	410	320	500	380	800	620	950	730	1130	870
60MPa	300	300	390	300	480	370	770	590	910	700	1080	830
65MPa	300	300	380	300	460	360	740	570	870	680	1040	800

NOTES:
1. FOR EPOXY COATED BARS THE VALUES IN THE TABLES MUST BE INCRESED:
a. MULTIPLY BY 1.2 (WHEN CLEAR COVER GREATER THAN 3 X BAR DIAMETER AND CLEAR SPACING GREATER THAN 6 X BAR DIAMETER)
b. MULTIPLY BY 1.5 (WHEN COVER OR SPACING ARE LESS THAN ABOVE)
2. VALUES PROVIDED ARE BASED ON NORMAL WEIGHT CONCRETE AND MUST BE INCREASED FOR LIGHTWEIGHT CONCRETES:
a. MULTIPLY BY 1.2 (FOR SEMI-LOW DENSITY CONCRETE)
b. MULTIPLY BY 1.3 (FOR LOW-DENSITY CONCRETE)
3. IF BUNDLED BARS ARE USED THE VAULES IN THE TABLES MUST BE INCREASED:
a. MULTIPLY BY 1.1 (TWO BAR BUNDLES)
b. MULTIPLY BY 1.2 (THREE BAR BUNDLES)
c. MULTIPLY BY 1.33 (FOUR BAR BUNDLES)

TENSION AND COMPRESSION DEVELOPMENT AND LAP LENGTHS Fy = 500 MPa — T-C57A

NOTES
1. STANDARD ABBREVIATIONS ON PLANS AND SCHEDULES SHOULD BE AS FOLLOWS
CLS - COMPRESSION LAP SPLICE
CDL - COMPRESSION DEVELOPMENT LENGTH
HEL - HOOK EMBEDMENT LENGTH

COMPRESSION LAP SPLICE AND DEVELOPMENT LENGTHS (Fy = 500 MPa)

CLS: COMPRESSION LAP SPLICE LENGTH

UNCOATED BLACK BAR							
10M	15M	20M	25M	30M	35M	45M	55M
490	680	830	1080	1280	1520	1840	2400

CDL: COMPRESSION DEVELOPMENT LENGTH

fc'	UNCOATED BLACK BAR							
	10M	15M	20M	25M	30M	35M	45M	55M
20MPa	310	430	530	680	810	960	1180	1520
25MPa	280	390	470	610	720	860	1050	1360
30MPa	250	360	430	560	660	790	970	1250
35MPa	250	360	430	560	660	790	970	1250
40MPa	250	360	430	560	660	790	970	1250
> 40 MPa	SEE MINUMUM VALUES IN ROW ABOVE							

NOTES:
1. IF BUNDLED BARS ARE USED THE VAULES IN THE TABLES MUST BE INCREASED:
a. MULTIPLY BY 1.1 (TWO BAR BUNDLES)
b. MULTIPLY BY 1.2 (THREE BAR BUNDLES)
c. MULTIPLY BY 1.33 (FOUR BAR BUNDLES)

HEL: STANDARD HOOK TENSION EMBEDMENT LENGTHS

fc'	UNCOATED BLACK BAR					
	10M	15M	20M	25M	30M	35M
20MPa	320	450	550	710	840	1000
25MPa	290	400	490	630	750	900
30MPa	260	370	450	580	690	820
35MPa	240	340	420	540	640	760
40MPa	230	320	390	500	600	710
45MPa	220	300	370	470	560	670
50MPa	200	290	350	450	530	640
55MPa	200	270	330	430	510	610

NOTES:
1. FOR EPOXY COATED BARS THE VALUES IN THE TABLES MUST BE INCREASED:
a. MULTIPLY BY 1.2 (WHEN CLEAR COVER GREATER THAN 3 X BAR DIAMETER AND CLEAR SPACING GREATER THAN 6 X BAR DIAMETER)
b. MULTIPLY BY 1.5 (WHEN COVER OR SPACING ARE LESS THAN ABOVE)
2. VALUES PROVIDED ARE BASED ON NORMAL WEIGHT CONCRETE AND MUST BE INCREASED FOR LIGHTWEIGHT CONCRETES:
a. MULTIPLY BY 1.2 (FOR SEMI-LOW DENSITY CONCRETE)
b. MULTIPLY BY 1.3 (FOR LOW-DENSITY CONCRETE)

TENSION AND COMPRESSION DEVELOPMENT AND LAP LENGTHS Fy = 500 MPa — T-C57B

NOTES
1. STANDARD ABBREVIATIONS ON PLANS AND SCHEDULES SHOULD BE AS FOLLOW
TLS - TENSION LAP SPLICE
TDL - TENSION DEVELOPMENT LENGTH

TENSION LAP SPLICE AND DEVELOPMENT LENGTHS (Fy = 500 MPa)

TLS: TENSION LAP SPLICE LENGTH (CLASS B)

fc'	UNCOATED BLACK BAR											
	10M		15M		20M		25M		30M		35M	
	Top	Bottom	Top	Bottom	Top	Bottom	Top	Bottom	Top	Bottom	Top	Bottom
20MPa	600	770	840	1090	1030	1330	1650	2150	1960	2550	2340	3040
25MPa	530	690	750	980	920	1190	1480	1920	1750	2280	2090	2720
30MPa	490	630	690	890	840	1090	1350	1750	1600	2080	1910	2480
35MPa	450	590	640	830	780	1010	1250	1620	1480	1930	1770	2300
40MPa	420	550	600	770	730	940	1170	1520	1390	1800	1660	2150
45MPa	400	520	560	730	690	890	1100	1430	1310	1700	1560	2030
50MPa	380	490	530	690	650	840	1050	1360	1240	1610	1480	1920
55MPa	360	470	510	660	620	800	1000	1300	1180	1540	1410	1840
60MPa	350	450	490	630	590	770	960	1240	1130	1470	1350	1760
65MPa	330	430	470	600	570	740	920	1190	1090	1420	1300	1690

TDL: TENSION DEVELOPMENT LENGTH

fc'	UNCOATED BLACK BAR											
	10M		15M		20M		25M		30M		35M	
	Top	Bottom	Top	Bottom	Top	Bottom	Top	Bottom	Top	Bottom	Top	Bottom
20MPa	460	600	650	840	790	1030	1270	1650	1510	1960	1800	2340
25MPa	410	530	580	750	710	920	1140	1480	1350	1750	1610	2090
30MPa	380	490	530	690	650	840	1040	1350	1230	1600	1470	1910
35MPa	350	450	490	640	600	780	960	1250	1140	1480	1360	1770
40MPa	330	420	460	600	560	730	900	1170	1070	1390	1280	1660
45MPa	310	400	430	560	530	690	850	1100	1010	1310	1200	1560
50MPa	290	380	410	530	500	650	810	1050	960	1240	1140	1480
55MPa	280	360	390	510	480	620	770	1000	910	1180	1090	1410
60MPa	270	350	380	490	460	590	740	960	870	1130	1040	1350
65MPa	260	330	360	470	440	570	710	920	840	1090	1000	1300

NOTES:
1. FOR EPOXY COATED BARS THE VALUES IN THE TABLES MUST BE INCRESED:
a. MULTIPLY BY 1.2 (WHEN CLEAR COVER GREATER THAN 3 X BAR DIAMETER AND CLEAR SPACING GREATER THAN 6 X BAR DIAMETER)
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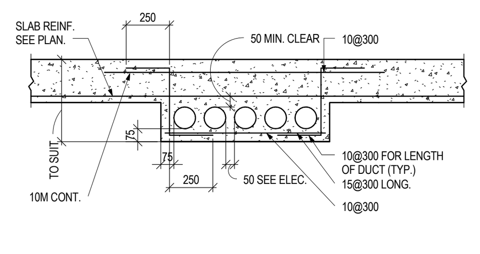
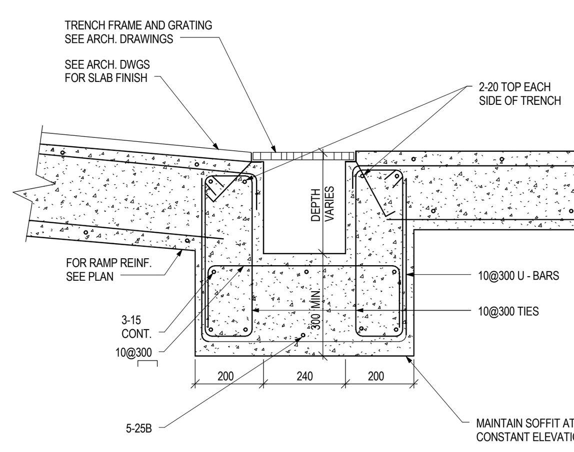


TYPICAL CONCRETE ENCASED ELECTRICAL DUCT BANKPOURED MONOLITHICALLY WITH SLAB



D-C03 TYPICAL TRENCH IN FRAMED RAMP SLAB

* ALTERNATIVES WHERE EXPOSED FACE OF SINGLE ANGLE IS NOT ACCEPTABLE:-
1. REINFORCED MASONRY LINTEL BLOCKS (SEE LINTEL NOTES)
2. DOUBLE ANGLES (SEE BELOW) (CUT BLOCKS TO SUIT)

NON-LOAD BEARING BLOCK WALL LINTELS

	T. NOMINAL \ CLEAR SPAN	300 TO 550 (12" TO 22")	550 TO 1220 (22" TO 4'-0")	1220 TO 1830 (4'-0" TO 6'-0")	1830 TO 2440 (6'-0" TO 8'-0")	2440 TO 3080 (8'-0" TO 10'-0")	3080 TO 3660 (10'-0" TO 12'-0")
	100	75x8 PL.	* 1L-90x90x6	* 1L-125x90x8 (LLV)	* 1L-125x90x8 (LLV)	N/A	N/A
	(4")	3x 5/16 PL.	* 1L-3½x3½x¼	* 1L-5x3½x 5/16 (LLV)	* 1L-5x3½x 5/16 (LLV)	N/A	N/A
	150	125x8 PL.	* 1L-125x90x6 (LLH)	* 1L-125x125x8	* 1L-125x125x8	* 1L-125x125x8	N/A
	(6")	5x 5/16 PL.	* 1L-5x3½x¼ (LLH)	* 1L-5x5x 5/16	* 1L-5x5x 5/16	* 1L-5x5x 5/16	N/A
	200	175x8 PL.	2Ls-90x90x6	2Ls-125x90x8 (LLV)	2Ls-125x90x8 (LLV)	W200x27 + 175x6 PL. BOTTOM	W200x27 + 175x6 PL. BOTTOM
	(8")	7x 5/16 PL.	2Ls-3½x3½x¼	2Ls-5x3½x 5/16 (LLV)	2Ls-5x3½x 5/16 (LLV)	W18x18 + 7x¼ PL. BOTTOM	W18x18 + 7x¼ PL. BOTTOM
	250	225x8 PL.	2Ls-100x90x6 (LLH)	2Ls-100x100x8	2Ls-150x100x8 (LLV)	W200x27 + 225x6 PL. BOTTOM	W200x27 + 225x6 PL. BOTTOM
	(10")	9x 5/16 PL.	2Ls-4x3½x¼ (LLH)	2Ls-4x4x 5/16	2Ls-6x4x 5/16 (LLV)	W18x18 + 9x¼ PL. BOTTOM	W18x18 + 9x¼ PL. BOTTOM
	300	275x8 PL.	3Ls-90x90x6	3Ls-125x90x8 (LLV)	N/A	N/A	N/A
	(12")	11x 5/16 PL.	3Ls-3½x3½x¼	3Ls-5x3½x 5/16 (LLV)	N/A	N/A	N/A
ALT. 2. *	100	N/A	2Ls-45x45x5	2Ls-55x35x6 (LLV)	N/A	N/A	N/A
ALT. 2. *	(4")	N/A	2Ls-1¾x1¾x 3/16	2Ls-2¼x1½x¼ (LLV)	N/A	N/A	N/A
ALT. 2. *	150	N/A	2Ls-65x65x6	2Ls-90x65x6 (LLV)	2Ls-90x65x6 (LLV)	N/A	N/A
ALT. 2. *	(6")	N/A	2Ls-2½x2½x¼	2Ls-3½x2½x¼ (LLV)	2Ls-3½x2½x¼ (LLV)	N/A	N/A

Contractor Must Check And Verify All Dimensions On The Job.
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No	Issued For	Date
2	BUILDING PERMIT SUBMISSION	JUNE 12,2015
1	ISSUED FOR WT REVIEW	May 01,2015

AQUAVISTA
AT BAYSIDE TORONTO

Drawing Title
TYPICAL DETAILS

Project
Hines TRIDEL
AQUAVISTA
Block 3 - R3 & R4
Merchant's Whart
Toronto, ON

Scale: 1 : 1
Drawn by: Author
Checked by: BBS
Project No.: 20140031
Date: JUNE 12,2015
Drawing No.: S8-02